

Northeast Metropolitan Regional Vocational High School

100 Hemlock Road, Wakefield, MA 01880
DRA PROJECT NO. 20202



• Structural

VOLUME 2 of 4

BID SET

Issue Date: August 28th, 2023

NORTHEAST METROPOLITAN REGIONAL VOCATIONAL H.S. BUILDING COMMITTEE

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DISTRICT COMMUNITIES

Chelsea, Malden, Melrose, North Reading, Reading, Revere, Saugus, Stoneham,
Wakefield, Winchester, Winthrop & Woburn

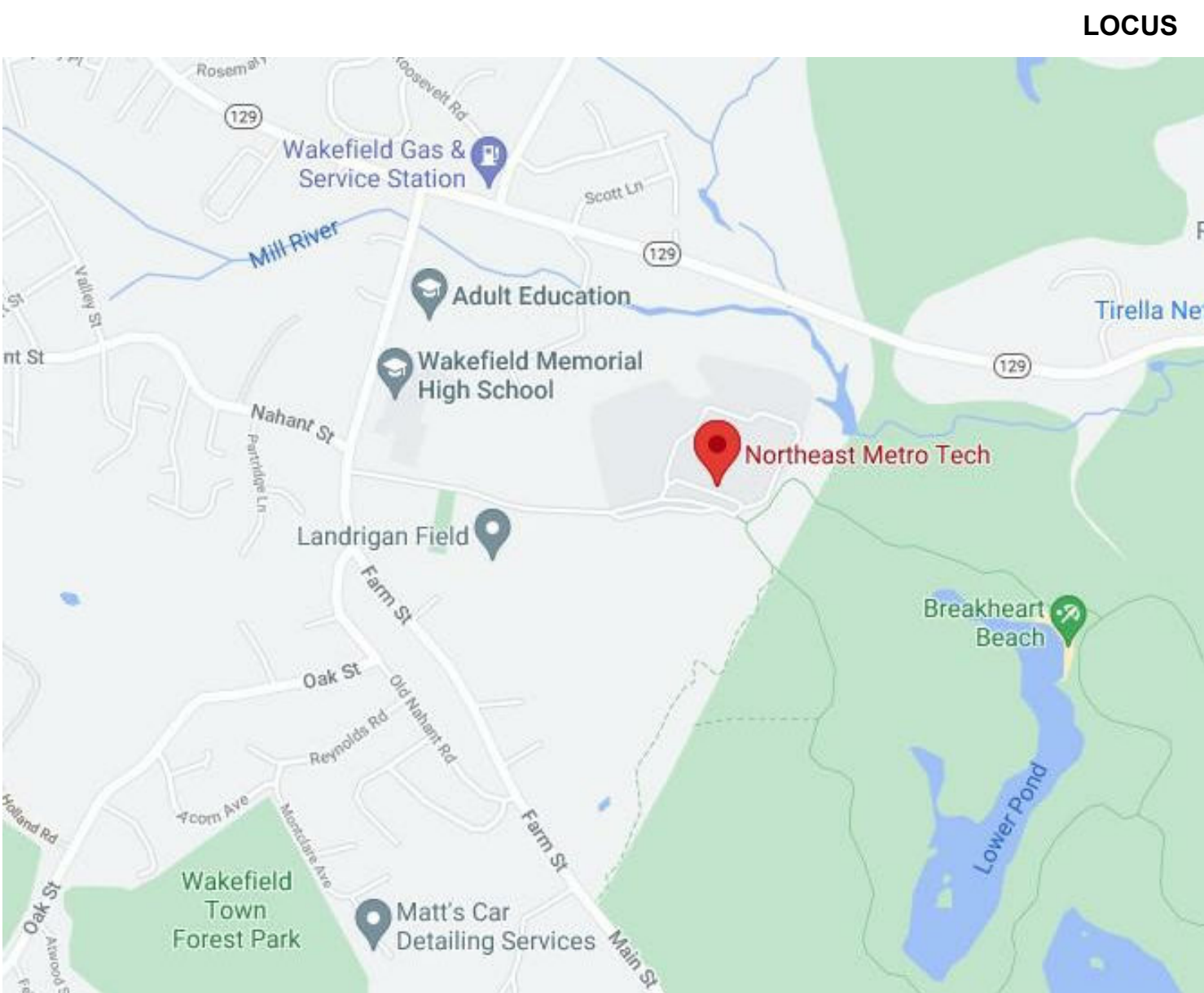
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VOLUME 2 DRAWINGS ARE A PART OF BID PACKAGE,
EARLY STRUCTURAL BID PACKAGE, DATED 2/24/2023.
THESE DRAWINGS ALSO INCLUDE STRUCTURAL
ADDENDUM NO. 1 (DATED 4/14/2023), ADDENDUM NO.2
(DATED 4/14/2023), PROPOSAL REQUEST PR-1 (DATED
5/24/23), PROPOSAL REQUEST PR-2 (DATED 6/30/23)
AND PROPOSAL REQUEST PR-3 (DATED 8/23/23).



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DRA

VOLUME 1 OF 4

CIVIL	
NUMBER	DRAWING NAME
EX-1	TOPOGRAPHIC SURVEY
EX-2	TOPOGRAPHIC SURVEY
EX-3	TOPOGRAPHIC SURVEY
EX-4	TOPOGRAPHIC SURVEY
EX-5	TOPOGRAPHIC SURVEY
EX-6	TOPOGRAPHIC SURVEY
EX-7	TOPOGRAPHIC SURVEY
EX-8	TOPOGRAPHIC SURVEY
EX-9	TOPOGRAPHIC SURVEY
EX-10	TOPOGRAPHIC SURVEY
EX-11	TOPOGRAPHIC SURVEY
EX-12	TOPOGRAPHIC SURVEY
EX-13	TOPOGRAPHIC SURVEY
EX-14	TOPOGRAPHIC SURVEY

C-000	CIVIL NOTES, LEGENDS, AND ABBREVIATIONS
C-100	KEY PLAN
C-200	UTILITY DEMOLITION PLAN I
C-201	UTILITY DEMOLITION PLAN II
C-202	UTILITY DEMOLITION PLAN III
C-203	UTILITY DEMOLITION PLAN IV
C-204	UTILITY DEMOLITION PLAN V
C-205	UTILITY DEMOLITION PLAN VI
C-300	SEDIMENTATION AND EROSION CONTROL PLAN I
C-301	SEDIMENTATION AND EROSION CONTROL PLAN II
C-302	SEDIMENTATION AND EROSION CONTROL PLAN III
C-303	SEDIMENTATION AND EROSION CONTROL PLAN IV
C-304	SEDIMENTATION AND EROSION CONTROL PLAN V
C-305	SEDIMENTATION AND EROSION CONTROL PLAN VI
C-400	DRAINAGE PLAN I
C-401	DRAINAGE PLAN II
C-402	DRAINAGE PLAN III
C-403	DRAINAGE PLAN IV
C-404	DRAINAGE PLAN V
C-405	DRAINAGE PLAN VI
C-406	DRAINAGE TABLE
C-500	UTILITY SITE PLAN I
C-501	UTILITY SITE PLAN II
C-502	UTILITY SITE PLAN III
C-503	UTILITY SITE PLAN IV
C-504	UTILITY SITE PLAN V
C-505	UTILITY SITE PLAN VI
C-506	UTILITY X-SECTION
C-600	DRIVEWAY LAYOUT AND GRADING PLAN I
C-601	DRIVEWAY LAYOUT AND GRADING PLAN II
C-602	DRIVEWAY LAYOUT AND GRADING PLAN III
C-603	DRIVEWAY LAYOUT AND GRADING PLAN IV
C-604	DRIVEWAY PROFILE
C-605	DRIVEWAY CROSS-SECTIONS
C-700	CIVIL SITE DETAILS I
C-701	CIVIL SITE DETAILS II
C-702	CIVIL SITE DETAILS III
C-703	CIVIL SITE DETAILS IV
C-704	CIVIL SITE DETAILS V
C-705	CIVIL SITE DETAILS VI
ADD ALT-1	ALTERNATE NO.1 LOCKER BUILDING UTILITY PLAN
ADD ALT-3	ALTERNATE NO.3 MAINTENANCE BUILDING
C800	WALL PLAN, ELEVATION & SECTION

LANDSCAPE

NUMBER	DRAWING NAME
L000	OVERALL SITE PLAN
L101	DEMO AND SITE PREPARATION PLAN – 1
L102	DEMO AND SITE PREPARATION PLAN – 2
L103	DEMO AND SITE PREPARATION PLAN – 3
L104	DEMO AND SITE PREPARATION PLAN – 4
L105	DEMO AND SITE PREPARATION PLAN – 5
L201	LAYOUT PLAN – 1
L202	LAYOUT PLAN – 2
L203	LAYOUT PLAN – 3
L204	LAYOUT PLAN – 4
L205	LAYOUT PLAN – 5
L301	MATERIALS PLAN – 1
L302	MATERIALS PLAN – 2
L303	MATERIALS PLAN – 3
L304	MATERIALS PLAN – 4
L305	MATERIALS PLAN – 5
L401	GRADING PLAN – 1
L402	GRADING PLAN – 2
L403	GRADING PLAN – 3
L404	GRADING PLAN – 4
L405	GRADING PLAN – 5
L501	PLANTING PLAN – 1
L502	PLANTING PLAN – 2
L503	PLANTING PLAN – 3
L504	PLANTING PLAN – 4
L505	PLANTING PLAN – 5
L601	LANDSCAPE DETAILS - 1
L602	LANDSCAPE DETAILS - 2
L603	LANDSCAPE DETAILS - 3
L604	LANDSCAPE DETAILS - 4
L605	LANDSCAPE DETAILS - 5
L606	LANDSCAPE DETAILS - 6
L607	BOARDWALK HANDRAIL AND GUARDRAIL PLAN – 1
L701	BOARDWALK HANDRAIL AND GUARDRAIL PLAN – 2
L702	BOARDWALK HANDRAIL AND GUARDRAIL PLAN – 3
L703	1703 – PLAZA ENTRANCE JOINTS PLAN
L801	ALTERNATE NO. 1 LOCKER BUILDING
L901	SIGNAGE & PAVEMENT MARKINGS PLAN

IRRIGATION

NUMBER	DRAWING NAME
I101	IRRIGATION PLAN
I201	IRRIGATION DETAILS
L202	IRRIGATION DETAILS
A1-0-1	OVERALL LOWER LEVEL & FIRST FLOOR PLANS
A1-0-2	OVERALL LOWER LEVEL AND FIRST FLOOR MEZZ. PLANS
A1-0-3	OVERALL SECOND FLOOR PLAN
A1-0-4	OVERALL THIRD FLOOR PLAN
A1-0-5	OVERALL FOURTH FLOOR PLAN
A1-0-6	OVERALL ROOF PLAN
A1-0-7	LOWER LEVEL BUILDING LAYOUT & EDGE OF SLAB PLAN
A1-0-8	FIRST FLOOR BUILDING LAYOUT & EDGE OF SLAB PLAN
A1-0-9	SECOND & THIRD FLOOR EDGE OF SLAB PLANS
A1-0-10	FOURTH FLOOR EDGE OF SLAB PLAN
A1-1-0C	LOWER LEVEL FLOOR PLAN - AREA C
A1-1-0D	LOWER LEVEL FLOOR PLAN - AREA D
A1-1-0MC	LOWER LEVEL MEZZANINE PLAN - AREA C
A1-1-0MD	LOWER LEVEL MEZZANINE PLAN - AREA D
A1-1-1A	FIRST FLOOR PLAN - AREA A
A1-1-1B	FIRST FLOOR PLAN - AREA B
A1-1-1C	FIRST FLOOR PLAN - AREA C
A1-1-1D	FIRST FLOOR PLAN - AREA D
A1-1-1MA	FIRST FLOOR MEZZANINE PLAN - AREA A
A1-1-1MB	FIRST FLOOR MEZZANINE PLAN - AREA B
A1-1-2A	SECOND FLOOR PLAN - AREA A
A1-1-2B	SECOND FLOOR PLAN - AREA B
A1-1-2C	SECOND FLOOR PLAN - AREA C
A1-1-2D	SECOND FLOOR PLAN - AREA D
A1-1-3A	THIRD FLOOR PLAN - AREA A
A1-1-3B	THIRD FLOOR PLAN - AREA B
A1-1-3C	THIRD FLOOR PLAN - AREA C
A1-1-3D	THIRD FLOOR PLAN - AREA D
A1-1-4A	FOURTH FLOOR PLAN - AREA A
A1-1-4B	FOURTH FLOOR PLAN - AREA B
A1-1-4C	FOURTH FLOOR PLAN - AREA C
A1-1-4D	FOURTH FLOOR PLAN - AREA D
A1-2-1A	ROOF PLAN - AREA A
A1-2-1B	ROOF PLAN - AREA B
A1-2-1C	ROOF PLAN - AREA C
A1-2-1D	ROOF PLAN - AREA D
A2-0-1	OVERALL BUILDING ELEVATIONS
A2-1-1	BUILDING ELEVATIONS
A2-1-2	BUILDING ELEVATIONS
A2-1-3	BUILDING ELEVATIONS
A2-1-4	BUILDING ELEVATIONS
A2-1-5	BUILDING ELEVATIONS
A2-1-6	BUILDING ELEVATIONS
A2-1-7	BUILDING ELEVATIONS
A2-1-8	BUILDING ELEVATIONS
A2-2-1	INTERIOR ELEVATIONS
A2-2-2	INTERIOR ELEVATIONS
A2-2-3	INTERIOR ELEVATIONS
A2-2-4	INTERIOR ELEVATIONS
A2-2-5	INTERIOR ELEVATIONS
A2-2-6	INTERIOR ELEVATIONS
A2-2-7	INTERIOR ELEVATIONS
A2-2-8	INTERIOR ELEVATIONS
A2-2-9	INTERIOR ELEVATIONS
A2-2-10	INTERIOR ELEVATIONS
A2-2-11	INTERIOR ELEVATIONS
A2-2-12	INTERIOR ELEVATIONS
A2-2-13	INTERIOR ELEVATIONS
A2-2-14	INTERIOR ELEVATIONS
A2-2-15	INTERIOR ELEVATIONS
A2-2-16	INTERIOR ELEVATIONS
A3-1-1	BUILDING SECTIONS
A3-1-2	BUILDING SECTIONS
A3-1-3	BUILDING SECTIONS
A3-1-4	BUILDING SECTIONS
A3-1-5	BUILDING SECTIONS
A3-1-6	BUILDING SECTIONS

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STRUCTURAL	
NUMBER	DRAWING NAME
S0-0-1	GENERAL NOTES
S0-0-2	TYPICAL DETAILS
S0-0-3	TYPICAL DETAILS
S0-0-4	TYPICAL DETAILS
S0-0-5	TYPICAL DETAILS
S0-0-6	TYPICAL DETAILS
S0-0-7	TYPICAL DETAILS
S0-0-8	TYPICAL DETAILS
S0-0-9	TYPICAL DETAILS
S1-1-0C	LOWER LEVEL FOUNDATION PLAN - AREA C
S1-1-0D	LOWER LEVEL FOUNDATION PLAN - AREA D
S1-1-0MC	MEZZANINE FLOOR FRAMING PLAN - AREA C
S1-1-0MD	MEZZANINE FLOOR FRAMING PLAN - AREA D
S1-1-1A	FIRST FLOOR FOUNDATION PLAN - AREA A
S1-1-1B	FIRST FLOOR FOUNDATION PLAN - AREA B
S1-1-1C	FIRST FLOOR FOUNDATION PLAN - AREA C
S1-1-1D	FIRST FLOOR FOUNDATION PLAN - AREA D
S1-1-1MA	MEZZANINE FLOOR FRAMING - AREA A
S1-1-1MB	MEZZANINE FLOOR FRAMING - AREA B
S1-1-2A	SECOND FLOOR FRAMING PLAN - AREA A
S1-1-2B	SECOND FLOOR FRAMING PLAN - AREA B
S1-1-2C	SECOND FLOOR FRAMING PLAN - AREA C
S1-1-2D	SECOND FLOOR FRAMING PLAN - AREA D
S1-1-3A	THIRD FLOOR FRAMING PLAN - AREA A
S1-1-3B	THIRD FLOOR FRAMING PLAN - AREA B
S1-1-3C	THIRD FLOOR FRAMING PLAN - AREA C
S1-1-3D	THIRD FLOOR FRAMING PLAN - AREA D
S1-1-4A	FOURTH FLOOR FRAMING PLAN - AREA A
S1-1-4B	FOURTH FLOOR FRAMING PLAN - AREA B
S1-1-4C	FOURTH FLOOR FRAMING PLAN - AREA C
S1-1-4D	FOURTH FLOOR FRAMING PLAN - AREA D
S1-1-5A	ROOF FRAMING PLAN - AREA A
S1-1-5B	ROOF FRAMING PLAN - AREA B
S1-1-5C	ROOF FRAMING PLAN - AREA C
S1-1-5D	ROOF FRAMING PLAN - AREA D
S1-1-6	JOIST DRAINAGE FRAMING PARTS
S2-0-1	SECTIONS
S2-0-2	SECTIONS
S2-0-3	SECTIONS
S2-0-4	SECTIONS
S2-0-5	SECTIONS
S3-0-1	SECTIONS
S3-0-2	SECTIONS
S3-0-3	SECTIONS
S3-0-4	SECTIONS
S3-0-5	SECTIONS
S3-0-6	SECTIONS
S3-0-7	SECTIONS
S3-0-8	SECTIONS
S3-0-9	SECTIONS
S4-0-1	BRACE FRAME ELEVATIONS - AREA A
S4-0-2	BRACE FRAME ELEVATION - AREAS A + B
S4-0-3	BRACE FRAME ELEVATION - AREA B
S4-0-4	BRACE FRAME ELEVATION C + D
S4-0-5	SHEAR WALLS
S4-0-6	SHEAR WALLS
S4-0-7	SHEAR WALLS
S4-0-8	BRACED FRAME DETAILS
S4-0-9	BRACED FRAME DETAILS
S5-0-1	JOIST LOADING DIAGRAMS
S5-0-2	JOIST LOADING DIAGRAMS
S5-0-3	JOIST LOADING DIAGRAMS
SC-1-1	CONCESSION BUILDING PLANS
SC-2-1	CONCESSION BUILDING SECTIONS
SL-1-1	LOCKER ROOM BUILDING SECTIONS
SL-3-1	LOCKER ROOM BUILDING SHEAR WALLS
SM-1-1	MAINTENANCE BUILDING FOUNDATION PLANS
SS-1-1	SITE BUILDING FOUNDATION PLANS

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CODE ANALYSIS

NUMBER	DRAWING NAME
CD1-1	CODE ANALYSIS LOWER LEVEL
CD1-2	CODE ANALYSIS FIRST FLOOR
CD1-3	CODE ANALYSIS FIRST FLOOR MEZZ
CD1-4	CODE ANALYSIS SECOND FLOOR
CD1-5	CODE ANALYSIS THIRD FLOOR
CD1-6	CODE ANALYSIS FOURTH FLOOR

ARCHITECTURAL

NUMBER	DRAWING NAME
A0-2-1	SYMBOLS & ABBREVIATIONS
A1-0-0	PARTITION TYPES
A1-0-1	OVERALL LOWER LEVEL & FIRST FLOOR PLANS
A1-0-2	OVERALL LOWER LEVEL AND FIRST FLOOR MEZZ. PLANS
A1-0-3	OVERALL SECOND FLOOR PLAN
A1-0-4	OVERALL THIRD FLOOR PLAN
A1-0-5	OVERALL FOURTH FLOOR PLAN
A1-0-6	OVERALL ROOF PLAN
A1-0-7	LOWER LEVEL BUILDING LAYOUT & EDGE OF SLAB PLAN
A1-0-8	FIRST FLOOR BUILDING LAYOUT & EDGE OF SLAB PLAN
A1-0-9	SECOND & THIRD FLOOR EDGE OF SLAB PLANS
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A1-1-0D	LOWER LEVEL FLOOR PLAN - AREA D
A1-1-0MC	LOWER LEVEL MEZZANINE PLAN - AREA C
A1-1-0MD	LOWER LEVEL MEZZANINE PLAN - AREA D
A1-1-1A	FIRST FLOOR PLAN - AREA A
A1-1-1B	FIRST FLOOR PLAN - AREA B
A1-1-1C	FIRST FLOOR PLAN - AREA C
A1-1-1D	FIRST FLOOR PLAN - AREA D
A1-1-1MA	FIRST FLOOR MEZZANINE PLAN - AREA A
A1-1-1MB	FIRST FLOOR MEZZANINE PLAN - AREA B
A1-1-2A	SECOND FLOOR PLAN - AREA A
A1-1-2B	SECOND FLOOR PLAN - AREA B
A1-1-2C	SECOND FLOOR PLAN - AREA C
A1-1-2D	SECOND FLOOR PLAN - AREA D
A1-1-3A	THIRD FLOOR PLAN - AREA A
A1-1-3B	THIRD FLOOR PLAN - AREA B
A1-1-3C	THIRD FLOOR PLAN - AREA C
A1-1-3D	THIRD FLOOR PLAN - AREA D
A1-1-4A	FOURTH FLOOR PLAN - AREA A
A1-1-4B	FOURTH FLOOR PLAN - AREA B
A1-1-4C	FOURTH FLOOR PLAN - AREA C
A1-1-4D	FOURTH FLOOR PLAN - AREA D
A1-2-1A	ROOF PLAN - AREA A
A1-2-1B	ROOF PLAN - AREA B
A1-2-1C	ROOF PLAN - AREA C
A1-2-1D	ROOF PLAN - AREA D
A2-0-1	OVERALL BUILDING ELEVATIONS
A2-1-1	BUILDING ELEVATIONS
A2-1-2	BUILDING ELEVATIONS
A2-1-3	BUILDING ELEVATIONS
A2-1-4	BUILDING ELEVATIONS
A2-1-5	BUILDING ELEVATIONS
A2-1-6	BUILDING ELEVATIONS
A2-1-7	BUILDING ELEVATIONS
A2-1-8	BUILDING ELEVATIONS
A2-2-1	INTERIOR ELEVATIONS
A2-2-2	INTERIOR ELEVATIONS
A2-2-3	INTERIOR ELEVATIONS
A2-2-4	INTERIOR ELEVATIONS
A2-2-5	INTERIOR ELEVATIONS
A2-2-6	INTERIOR ELEVATIONS
A2-2-7	INTERIOR ELEVATIONS
A2-2-8	INTERIOR ELEVATIONS
A2-2-9	INTERIOR ELEVATIONS
A2-2-10	INTERIOR ELEVATIONS
A2-2-11	INTERIOR ELEVATIONS
A2-2-12	INTERIOR ELEVATIONS
A2-2-13	INTERIOR ELEVATIONS
A2-2-14	INTERIOR ELEVATIONS
A2-2-15	INTERIOR ELEVATIONS
A2-2-16	INTERIOR ELEVATIONS
A3-1-1	BUILDING SECTIONS
A3-1-2	BUILDING SECTIONS
A3-1-3	BUILDING SECTIONS
A3-1-4	BUILDING SECTIONS
A3-1-5	BUILDING SECTIONS
A3-1-6	BUILDING SECTIONS

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A3-2-2	WALL SECTIONS
A3-2-3	WALL SECTIONS
A3-2-4	WALL SECTIONS
A3-2-5	WALL SECTIONS
A3-2-6	WALL SECTIONS
A3-2-7	WALL SECTIONS
A3-2-8	WALL SECTIONS
A3-2-9	WALL SECTIONS
A3-2-10	WALL SECTIONS
A3-2-11	WALL SECTIONS
A3-2-12	WALL SECTIONS
A3-2-13	WALL SECTIONS
A3-2-14	WALL SECTIONS
A3-2-15	WALL SECTIONS
A3-2-16	WALL SECTIONS
A3-2-17	WALL SECTIONS
A3-2-18	WALL SECTIONS
A3-2-19	WALL SECTIONS
A3-2-20	WALL SECTIONS
A3-2-21	WALL SECTIONS
A3-2-22	WALL SECTIONS
A3-2-23	WALL SECTIONS
A3-2-24	WALL SECTIONS
A3-2-25	WALL SECTIONS
A3-2-26	WALL SECTIONS
A3-2-27	WALL SECTIONS
A3-2-28	WALL SECTIONS
A3-2-29	WALL SECTIONS
A3-2-30	WALL SECTIONS
A3-3-1	VERTICAL DETAILS
A3-3-2	VERTICAL DETAILS
A3-3-3	VERTICAL DETAILS
A3-3-4	VERTICAL DETAILS
A3-3-5	VERTICAL DETAILS
A4-1-1	TOILET AND LOCKER ROOM PLANS
A4-1-2	TOILET PLANS
A4-1-3	TOILET PLANS
A4-1-4	TOILET AND LOCKER ROOM ELEVATIONS
A4-1-5	TOILET AND LOCKER ROOM ELEVATIONS
A4-1-6	TOILET ELEVATIONS
A4-1-7	TOILET ELEVATIONS
A4-1-8	EXTERIOR PLAN DETAILS
A5-1-2	EXTERIOR PLAN DETAILS
A5-1-3	EXTERIOR PLAN DETAILS
A5-1-4	EXTERIOR PLAN DETAILS
A5-2-1	INTERIOR PLAN DETAILS
A5-2-2	INTERIOR PLAN DETAILS
A5-2-3	INTERIOR PLAN DETAILS
A5-3-1	ROOF DETAILS
A5-3-2	ROOF DETAILS
A5-3-3	ROOF DETAILS
A6-2-1	DOOR SCHEDULE
A6-2-2	DOOR SCHEDULE
A6-2-3	DOOR SCHEDULE, FRAME & BORROWED LIGHT TYPES
A6-2-4	DOOR AND BORROWED LITE DETAILS
A6-2-5	DOOR AND BORROWED LITE DETAILS
A6-3-1	WINDOW, TRANSLUCENT WALL PANEL AND LOUVER TYPES
A6-3-2	CURTAIN WALL TYPES
A6-3-3	CURTAIN WALL TYPES
A6-3-4	CURTAIN WALL TYPES
A6-3-5	INTERIOR CURTAIN WALL TYPES
A6-3-6	STOREFRONT AND INTERIOR STOREFRONT TYPES
A6-3-7	INTERIOR STOREFRONT TYPES
A6-3-8	WINDOW AND TRANSLUCENT WALL PANEL DETAILS
A6-3-9	LOUVER DETAILS
A6-3-10	CURTAIN WALL DETAILS
A6-3-11	CURTAIN WALL DETAILS
A6-3-12	CURTAIN WALL DETAILS
A6-3-13	CURTAIN WALL DETAILS
A6-3-14	INTERIOR CURTAIN WALL DETAILS
A6-3-15	INTERIOR CURTAIN WALL DETAILS
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A6-3-17	STOREFRONT & INTERIOR STOREFRONT DETAILS
A6-3-18	INTERIOR STOREFRONT DETAILS
A6-3-19	INTERIOR STOREFRONT DETAILS
A7-1-1	STAIR PLANS
A7-1-2	STAIR SECTIONS
A7-1-3	STAIR PLANS & SECTIONS
A7-1-4	STAIR PLANS
A7-1-5	STAIR SECTIONS
A7-1-6	STAIR PLANS & SECTIONS
A7-1-7	STAIR PLANS & SECTIONS
A7-1-8	STAIR PLANS & SECTIONS
A7-1-9	ELEVATOR PLANS & SECTIONS
A7-1-10	STAIR DETAILS
A7-1-11	STAIR DETAILS - ELEVATOR DETAILS, SPECIALTY DETAILS
A7-1-12	LOADING RAMP PLAN, SECTION & DETAILS
A8-1-0C	LOWER LEVEL REFLECTED CEILING PLAN - AREA C
A8-1-0D	LOWER LEVEL REFLECTED CEILING PLAN - AREA D
A8-1-0MC	LOWER LEVEL MEZZANINE REFLECTED CEILING PLAN - AREA C
A8-1-0MD	LOWER LEVEL MEZZANINE REFLECTED CEILING PLAN - AREA D
A8-1-1A	FIRST FLOOR REFLECTED CEILING PLAN - AREA A
A8-1-1B	FIRST FLOOR REFLECTED CEILING PLAN - AREA B
A8-1-1C	FIRST FLOOR REFLECTED CEILING PLAN - AREA C
A8-1-1D	FIRST FLOOR REFLECTED CEILING PLAN - AREA D
A8-1-1MA	FIRST FLOOR MEZZANINE REFLECTED CEILING PLAN - AREA A
A8-1-1MB	FIRST FLOOR MEZZANINE REFLECTED CEILING PLAN - AREA B
A8-1-2A	SECOND FLOOR REFLECTED CEILING PLAN - AREA A
A8-1-2B	SECOND FLOOR REFLECTED CEILING PLAN - AREA B
A8-1-2C	SECOND FLOOR REFLECTED CEILING PLAN - AREA C
A8-1-2D	SECOND FLOOR REFLECTED CEILING PLAN - AREA D
A8-1-3A	THIRD FLOOR REFLECTED CEILING PLAN - AREA A
A8-1-3B	THIRD FLOOR REFLECTED CEILING PLAN - AREA B
A8-1-3C	THIRD FLOOR REFLECTED CEILING PLAN - AREA C
A8-1-3D	THIRD FLOOR REFLECTED CEILING PLAN - AREA D
A8-1-4A	FOURTH FLOOR REFLECTED CEILING PLAN - AREA A
A8-1-4B	FOURTH FLOOR REFLECTED CEILING PLAN -

DRAWING

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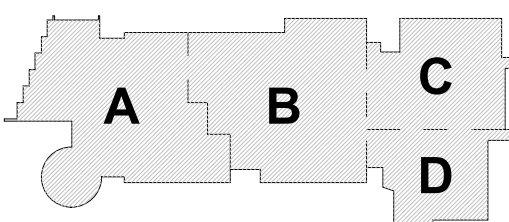


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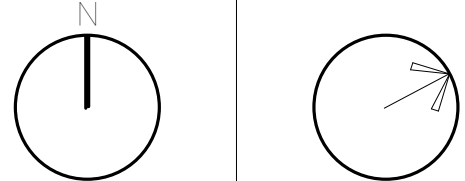
BID SET

August 28th, 2023



KEY PLAN

PROJECT NORTH MAGNETIC NORTH



GENERAL NOTES

Scale: As Indicated
Job No.: 20202
Drawn By: EDG
Date: August 28th, 2023
S0-0-1

GENERAL NOTES

- G1. THE INTENT OF THE STRUCTURAL DRAWINGS IS TO SHOW THE MAIN STRUCTURAL FEATURES AND DESIGN FOR THE INTENDED PROJECT. ARCHITECTURAL DETAILS AND OTHER COMPONENTS THAT MAY BE NECESSARY TO CONSTRUCT THE PROJECT ARE SHOWN INCIDENTALLY ONLY AND NOT COMPLETELY. THEREFORE, ALL CONTRACT DRAWINGS AND SPECIFICATIONS MUST BE USED IN CONJUNCTION WITH THE STRUCTURAL DRAWINGS DURING ALL PHASES OF CONSTRUCTION. DISCREPANCIES BETWEEN STRUCTURAL DRAWINGS AND ARCHITECTURAL DRAWINGS, IF NOT CLARIFIED IN THE ADDENDA AT THE REQUEST OF THE CONTRACTOR, SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT DURING CONSTRUCTION FOR CLARIFICATIONS. THE CONTRACTOR SHALL TAKE THIS INTO CONSIDERATION IN HIS BID.
- G2. THE CONTRACTOR SHALL INFORM THE ARCHITECT OF ALL DISCREPANCIES BETWEEN DRAWINGS OF DIFFERENT TRADES PRIOR TO INITIATION OF ANY WORK.
- G3. THE DESIGN IS IN ACCORDANCE WITH THE NINTH EDITION OF THE MASSACHUSETTS STATE BUILDING CODE.
- G4. THE CONTRACTOR SHALL BE COMPLETELY RESPONSIBLE FOR THE SAFETY OF ADJACENT STRUCTURES, PROPERTY, AND THE PUBLIC. THE CONTRACTOR SHALL SHIELD, BRACE, AND PROTECT THE EXISTING BUILDING AS REQUIRED FOR CONSTRUCTION OF NEW WORK.
- G5. REFER TO ARCHITECTURAL, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS FOR VERIFICATION OF LOCATIONS AND DIMENSIONS OF ALL CHASES, SLOTS, INSERTS, CURBS, OPENINGS, SLEEVES, ANCHOR BOLTS, FLOOR PITCHES, ANGLE FRAMES, AND ALL OTHER PROJECT REQUIREMENTS NOT SHOWN ON STRUCTURAL DRAWINGS. WORK NOT INDICATED ON A PART OF THE DRAWINGS BUT REASONABLY IMPLIED TO BE SIMILAR TO THAT SHOWN AT CORRESPONDING LOCATIONS SHALL BE CONSIDERED.
- G6. DETAILS SHOWN AS TYPICAL ARE APPLICABLE TO ALL SIMILAR CONDITIONS.
- G7. ALL CONTRACTORS ARE REQUIRED TO EXAMINE THE DRAWINGS AND SPECIFICATIONS CAREFULLY, VISIT THE SITE, AND FULLY INFORM THEMSELVES AS TO ALL EXISTING CONDITIONS AND LIMITATIONS, PRIOR TO SUBMITTING THE PROPOSAL. FAILURE TO VISIT THE SITE AND FAMILIARIZE THEMSELVES WITH THE EXISTING CONDITIONS AND LIMITATIONS WILL, IN NO WAY RELIEVE THE SUCCESSFUL BIDDER FROM FURNISHING ANY MATERIALS OR PERFORMING ANY WORK IN ACCORDANCE WITH DRAWINGS AND SPECIFICATIONS WITHOUT ADDITIONAL COST TO THE OWNER.
- G8. THE CONTRACTOR IS RESPONSIBLE FOR ALL CONSTRUCTION MEANS AND METHODS INCLUDING BUT NOT LIMITED TO TEMPORARY SHORING AND BRACING OF NEW AND EXISTING CONSTRUCTION TO MAINTAIN STRUCTURAL STABILITY FOR ALL CONDITIONS OF STATIC, DYNAMIC, GRAVITY, AND WIND LOADS DURING DEMOLITION PROCEDURES, REPAIR PROCEDURES, AND NEW CONSTRUCTION PROCEDURES THROUGHOUT THE DURATION OF THE CONSTRUCTION CONTRACT. THE CONTRACTOR IS RESPONSIBLE FOR ALL ASPECTS OF TEMPORARY SHORING INCLUDING ENGINEERING. A PROFESSIONAL ENGINEER REGISTERED IN THE COMMONWEALTH OF MASSACHUSETTS, THE CONTRACTOR IS RESPONSIBLE FOR ALL CONSTRUCTION-RELATED SAFETY MEASURES.
- G9. ANY ILLUSTRATION OR DESCRIPTION OF CONSTRUCTION SEQUENCING, TEMPORARY SHORING SEQUENCE, OR TEMPORARY SHORING SYSTEM, AS SHOWN ANYWHERE IN THE CONTRACT DOCUMENTS (DRAWINGS AND/OR SPECIFICATIONS) IS PROVIDED TO THE CONTRACTOR ONLY FOR ILLUSTRATION OF A POSSIBLE METHOD OR SEQUENCE OF ACCOMPLISHING THE WORK. TO DEMONSTRATE FEASIBILITY IN PRINCIPLE ONLY, UNLESS OTHERWISE NOTED. SUCH ILLUSTRATIONS OR DESCRIPTIONS OF CONSTRUCTION SEQUENCING OR TEMPORARY SHORING IN THE CONTRACT DOCUMENTS ARE DEMED SOLELY FOR CONSIDERATION OF VERY SPECIAL ATTENTION REQUIRED BY THE CONTRACTOR. SUCH ILLUSTRATIONS OR DESCRIPTIONS OF CONSTRUCTION SEQUENCING OR TEMPORARY SHORING ARE NOT TO BE INTERPRETED IN ANY WAY AS LIMITING THE WORK REQUIRING SEQUENCING OR TEMPORARY SHORING TO ONLY THOSE ASPECTS ILLUSTRATED OR DESCRIBED. AS PART OF THE BASE CONTRACT WORK, THE CONTRACTOR SHALL IDENTIFY, PLAN, FOR, ENGINEER AND DETAIL, AND PROVIDE ALL CONSTRUCTION SEQUENCING AND TEMPORARY SHORING AS NECESSARY TO SAFELY AND SUCCESSFULLY EXECUTE ALL THE WORK ENCOMETERED FOR THIS PROJECT.

FOUNDATION NOTES

- F1. THE FOUNDATION DESIGN IS BASED ON THE RECOMMENDATIONS PROVIDED IN THE GEOTECHNICAL ENGINEERING REPORT PREPARED BY LAHAF GEOTECHNICAL CONSULTANTS, INC. AUGUST 12, 2020. REFER TO BORING LOGS AND TEST PIT DATA IN THE SPECIFICATIONS FOR ADDITIONAL INFORMATION.
- F2. FOOTINGS SHALL BEAR TYPICALLY ON STRUCTURAL FILL PLACED DIRECTLY OVER THE NATURAL SAND AND GRAVEL OR ON ROCK HAVING A MINIMUM BEARING CAPACITY OF 2 TONS PER SQUARE FOOT. REFER TO EARTHWORK SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.
- F3. ALL BACKFILL UNDER STRUCTURAL SLABS, MATS, AND FOOTINGS WILL BE ENGINEERED STRUCTURAL FILL COMPACTED IN SPECIFIED LIFTS TO 95 PERCENT OF MAXIMUM DRY DENSITY, UNLESS OTHERWISE INDICATED OR SPECIFIED. REFER TO EARTHWORK SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.
- F4. PROVIDE SHEETING, BRACING, AND UNDERPINNING AS REQUIRED TO PRESERVE ADJACENT STRUCTURES.
- F5. FOUNDATIONS AND SLABS SHALL NOT BE POURED IN WATER OR ON FROZEN GROUND.
- F6. VERIFY LOCATIONS AND REQUIREMENTS FOR INSERTS, SLEEVES, CONDUITS, EMBEDMENTS, AND PENETRATIONS WITH RESPECTIVE TRADES BEFORE PLACING CONCRETE.
- F7. BLASTING SHALL BE COMPLETED BEFORE ANY CONCRETE IS PLACED.
- F8. DOWELS FROM FOUNDATIONS INTO PIERS, PIPE CAPS, COLUMNS, BUTTRESSES, OR WALLS SHALL BE THE SAME SIZE AND NUMBER AS REINFORCEMENT IN PIERS, COLUMNS, BUTTRESSES, OR WALLS ABOVE, EXCEPT AS OTHERWISE SHOWN.
- F9. CONTRACTOR SHALL PROVIDE CONTINUOUS DRAINAGE BY MECHANICAL METHODS TO CONTROL SURFACE AND UNDERGROUND WATER AS REQUIRED UNDER CONSTRUCTION. REFER TO EARTHWORK SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.
- F10. CONTRACTOR SHALL ENSURE THAT GROUND WATER LEVELS UNDER ADJACENT STRUCTURES AND PROPERTIES ARE NOT ALTERED. REFER TO EARTHWORK SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.
- F11. ALL FOUNDATIONS UNITS (PIERS AND FOOTINGS) SHALL BE CENTERED UNDER SUPPORT MEMBERS, UNLESS NOTED OTHERWISE ON PLANS.
- F12. COORDINATE UNDER FLOOR AND PERIMETER DRAIN REQUIREMENTS WITH ARCHITECTURAL, CIVIL, AND PLUMBING DRAWINGS.
- F13. THE OWNER AND ENGINEER ASSUME NO RESPONSIBILITY FOR THE VALIDITY OF THE SUBSURFACE CONDITIONS DESCRIBED ON THE DRAWINGS, SPECIFICATIONS, BORING LOGS, OR TEST PITS. THIS DATA IS INCLUDED ONLY TO ASSIST THE CONTRACTOR DURING BIDDING AND SUBSEQUENT CONSTRUCTION AND REPRESENT CONDITIONS ONLY OF THOSE SPECIFIED LOCATIONS AT THE PARTICULAR TIME THEY WERE MADE.
- F14. CONTRACTOR SHALL INFORM THE ARCHITECT AND RELOCATE ANY EXISTING UTILITY LINES AS REQUIRED THAT MAY INTERFERE WITH NEW FOUNDATIONS. CONTRACTOR SHALL REMOVE ANY EXISTING UTILITY LINES THAT ARE BEING ABANDONED IN THE VICINITY OF THE NEW FOUNDATION AND BACKFILL THE AREA WITH COMPACTED STRUCTURAL FILL.
- F15. SEE THE PROJECT SPECIFICATIONS FOR ADDITIONAL INFORMATION AND OTHER SPECIFIC FOUNDATION CONSTRUCTION REQUIREMENTS.

REINFORCED CONCRETE NOTES

- R1. ALL CONCRETE WORK SHALL CONFORM TO ACI-318-14 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE AND TO THE NINTH EDITION OF THE MASSACHUSETTS STATE BUILDING CODE. IN CASE OF CONFLICT, THE MASSACHUSETTS STATE BUILDING CODE SHALL GOVERN.
- R2. ALL CONCRETE SHALL BE CONTROLLED, MIXED, AND PLACED UNDER THE SUPERVISION OF AN APPROVED CONCRETE TESTING AGENCY, PROVIDED BY OWNER.
- R3. ALL CONCRETE EXPOSED TO THE WEATHER SHALL BE NORMAL WEIGHT CONCRETE WITH A MINIMUM 5,000 PSI 28 DAY COMPRESSION STRENGTH AND CONTAIN AN AIR ENTRAINMENT ADMIXTURE.
- R4. UNLESS OTHERWISE NOTED, ALL CONCRETE SHALL BE NORMAL WEIGHT AND HAVE A MINIMUM COMPRESSIVE STRENGTH OF 4,500 POUNDS PER SQUARE INCH AT THE END OF 28 DAYS. CONCRETE SLABS ON GRADE AND SUPPORTED CONCRETE SLABS SHALL BE NORMAL WEIGHT AND HAVE A MINIMUM COMPRESSIVE STRENGTH OF 4,500 POUNDS PER SQUARE INCH AT THE END OF 28 DAYS. CONCRETE SLABS ON STEEL DECK SHALL BE NORMAL WEIGHT CONCRETE AS INDICATED ON PLANS WITH A MINIMUM COMPRESSIVE STRENGTH OF 4,500 PSI AT THE END OF 28 DAYS.
- R5. CONCRETE QUALITY IN ACCORDANCE WITH THE REQUIREMENTS OF THESE DRAWINGS AND SPECIFICATIONS IS ESSENTIAL. TO THE STRUCTURAL PERFORMANCE OF THIS BUILDING. CONCRETE THAT IS NOT IN ACCORDANCE WITH THE DRAWINGS AND SPECIFICATIONS WILL NOT BE ACCEPTED.
- R6. CONCRETE SHALL REACH THE 40 PERCENT OF ITS 28 DAY COMPRESSIVE STRENGTH (6) BEFORE FORMS OR SHORES FOR WALLS MAY BE REMOVED, NO FORMS CAN BE REMOVED UNLESS CONCRETE HAS ATTAINED SUFFICIENT STRENGTH TO SUPPORT ITS OWN WEIGHT.
- R7. CONSTRUCTION JOINT LOCATIONS, OTHER THAN SHOWN ON THE DRAWINGS, ARE PERMITTED SUBJECT TO PRIOR APPROVAL OF THE ENGINEER. EXPANSION JOINT AND CONTROL JOINT LOCATIONS ARE MANDATORY AS SHOWN.
- R8. REINFORCING BARS SHALL CONFORM TO ASTM A615 WITH 60,000 POUNDS PER SQUARE INCH YIELD STRENGTH, AS INDICATED AND SHALL HAVE THE FOLLOWING CONCRETE COVER, UNLESS NOTED OTHERWISE ON THE DRAWINGS.
A. SURFACES PLACED IN CONTACT WITH THE GROUND - 3"
B. FORMED SURFACE EXPOSED TO GROUND - 2"
C. INSIDE FACE OF FORMED WALL - 1 1/2"
D. WALL PIERES - 1 1/2"
E. SLAB REINFORCING - 3/4" TOP & BOTTOM
- R9. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185, LAP TWO SQUARES AT ALL SPLICES AND TIE AT 3 FOOT CENTERS.
- R10. ALL LAP REINFORCING TO DEVELOP FULL TENSION CAPACITY OF THE SMALLER BAR REINFORCEMENT UNLESS OTHERWISE NOTED ON THE DRAWINGS.
- R11. PROVIDE BAR SUPPORTS, SPACES, AND ACCESSORIES RECOMMENDED IN THE LATEST ADDITION OF THE ACI DETAILING MANUAL. PUBLICATION SHALL. ALL REINFORCEMENT DETAILING LAP SPACES AND EMBEDMENTS SHALL CONFORM TO THIS MANUAL. ALL ACCESSORIES, SUCH AS SLAB BOLSTERS AND BEAM AND SLAB CHAIRS IN CONTACT WITH EXPOSED SURFACES, SHALL BE ZINC COATED OR PLASTIC TYPE.
- R12. PIPES OR CONDUITS SHALL NOT BE PLACED IN SLABS ON GRADE OR ELEVATED SLABS.
- R13. CONCRETE WALLS SHALL BE CAST IN PANELS NOT EXCEEDING 60 FEET IN LENGTH.

REINFORCED CONCRETE NOTES (CONT)

- R14. DETAILING OF REINFORCEMENT SHALL BE ACCORDING TO THE LATEST EDITION OF ACI 318 "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES".
- R15. SET SECRETLY AND TIE ALL REINFORCEMENT BEFORE PLACING CONCRETE.
- R16. ALL REINFORCING SHALL BE CONTINUOUS THROUGH CONSTRUCTION JOINTS.
- R17. FOR CONCRETE FILL AND TOPPING, PLACE CONSTRUCTION AND EXPANSION JOINTS AT THE SAME LOCATION AS THE CONSTRUCTION AND EXPANSION JOINTS IN THE SUPPORTING CONCRETE.
- R18. BUILD ALL CONCRETE MAT FOUNDATIONS, BASE SLABS, WALLS, AND FLOORS TO MINIMIZE THE EFFECTS OF SHRINKAGE BY CASTING ALTERNATE SECTIONS. ADJACENT SECTIONS MAY BE CAST WHEN PREVIOUSLY PLACED SECTION HAS CURED FOR 48 HOURS AFTER ITS INITIAL SET. CURING REQUIREMENTS ARE SPECIFIED IN SECTION 03300, CAST-IN-PLACE CONCRETE OF THE SPECIFICATIONS.
- R19. EXPOSED EDGES OF CONCRETE ELEMENTS, SUCH AS PILASTERS, CURBS, AND EQUIPMENT PADS, WILL HAVE A 1/4 CHAMFER.
- R20. ALL KEYS SHALL BE 2"x4" (NOMINAL) UNLESS SHOWN OTHERWISE ON DRAWINGS.
- R21. NOT ALL OPENINGS THROUGH CONCRETE SLABS AND WALLS ARE SHOWN ON STRUCTURAL DRAWINGS. OPENINGS INDICATED, OR ANY ADDITIONAL OPENINGS OR INSERTS REQUIRED, SHALL BE VERIFIED WITH RESPECTIVE TRADES BEFORE POURING OF CONCRETE.
- R22. USE NON-SHRINK, NON-METALLIC GROUT WHERE INDICATED. SEE SECTION 03300, CAST-IN-PLACE CONCRETE OF THE SPECIFICATIONS FOR ALL THE REQUIREMENTS.
- R23. SEE ARCHITECTURAL DRAWINGS FOR FINISHES, DEPRESSIONS, REGLETS, NOTCHES, AND OTHER ARCHITECTURAL FEATURES.
- R24. PROVIDE SEALANT JOINTS FOR ALL EXPOSED TO VIEW CONSTRUCTION JOINTS, CONTROL JOINTS, AND SHEAR KEYS.
- R25. SET ANCHOR BOLTS AND EMBEDDED PLATES REQUIRED FOR CONNECTION OF WORK FURNISHED BY OTHER TRADES FOR INSTALLATION AS PART OF THEIR SCOPE OF WORK.
- R26. PROVIDE A MINIMUM OF 6" AT 12" EACH WAY, EACH FACE FOR ALL WALLS, FOOTINGS, PITS, OR PADS, UNLESS NOTED OTHERWISE.
- R27. PROVIDE CONCRETE PADS FOR MECHANICAL EQUIPMENT ACCORDING TO THE REQUIREMENTS OF THE MANUFACTURER AND IN ACCORDANCE WITH THE TYPICAL DETAILS ALWAYS PROVIDE A MINIMUM REINFORCEMENT FOR PADS, UNLESS NOTED OTHERWISE. COORDINATE LOCATIONS WITH MECHANICAL WORK.
- R28. PROVIDE CONDENSATE PITS AND OTHER DEPRESSIONS OR CURBS AS REQUIRED FOR COMPLETION OF THE MECHANICAL WORK.
- R29. NO CONCRETE SHALL BE PLACED BEFORE REVIEW AND APPROVAL OF THE REINFORCING AND EMBEDDED ITEMS HAVE BEEN OBTAINED FROM THE ARCHITECT / ENGINEER.
- R30. WHEN REINFORCEMENT IS PLACED IN TWO OR MORE LAYERS, BARS IN THE UPPER LAYERS SHALL BE PLACED DIRECTLY ABOVE THOSE IN THE BOTTOM LAYER WITH A CLEAR DISTANCE BETWEEN THE LAYERS NOT LESS THAN ONE INCH.
- R31. FLOOR SLABS WILL BE AN INTEGRAL PART OF STRUCTURAL SLABS. SEPARATE CONCRETE FILL IS NOT PERMITTED. NO. 10 FIBER REINFORCING SHALL BE PLACED IN THE BOTTOM LAYER ON CENTER OR CLOSER TO SLOPED SURFACES, SHALL BEGIN AT THE LOWEST ELEVATION AND CONTINUE MONOLITHICALLY TOWARD THE HIGHER ELEVATIONS UNTIL THE INTENDED POUR IS COMPLETED.
- R32. ELEVATED SLABS ON DECK SHALL BE FINISHED LEVEL USING LASER MEASUREMENT AND SHALL MEET THE TOLERANCES FOR FLATNESS. SLAB THICKNESS INDICATED ON THE PLANS ARE A MINIMUM THICKNESSES. FOR QUANTITY OF CONCRETE, THE CONTRACTOR SHALL TAKE INTO ACCOUNT DEFLECTIONS OF SUPPORTING STRUCTURAL MEMBERS, FLOOR AND DECK FOR AN AVERAGE ADDITIONAL 3/4" OF CONCRETE FOR THE ENTIRE AREA OF THE SLAB. USE A MAXIMUM OF THE DESIGN SLAB THICKNESS PLUS 1 INCH MAX.
- R33. FIBER REINFORCEMENT SHALL BE ASTM C 1116 - TYPE III, MACRO-SYNTHETIC FIBERS (HIGH-VOLUME SYNTHETICS USED FOR REDUCTION OF PLASTIC AND DRYING SHRINKAGE CRACKING). PROVIDE IN SLABS-ON-GRADE, ONLY WHERE SHOWN ON THE DRAWINGS. CONTRACTOR SHALL FOLLOW ALL MANUFACTURERS REQUIREMENTS AND PROVIDE A DOSAGE AS RECOMMENDED BY THE MANUFACTURER FOR CONDITIONS SHOWN ON THE DRAWINGS. E.G. SLAB THICKNESS, CONCRETE MIX STRENGTH, SPACING, AND LOCATION OF CONSTRUCTION JOINTS AND SAW-CUT CONTROL JOINTS, ETC.

REINFORCED CONCRETE MASONRY

- M1. ALL REINFORCED CONCRETE MASONRY SHALL CONFORM TO ACI 300-13 "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES" AND ACI 308-13 "3" BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES" FOR THE DESIGN AND CONSTRUCTION OF LOAD-BEARING CONCRETE MASONRY AND TO THE NINTH EDITION OF THE MASSACHUSETTS STATE BUILDING CODE. IN CASE OF CONFLICT, THE MASSACHUSETTS STATE BUILDING CODE SHALL GOVERN.
- M2. CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90, GRADE N, TYPE I, NORMAL WEIGHT WITH AN AVERAGE MINIMUM COMPRESSIVE STRENGTH OF 2,000 POUNDS PER SQUARE INCH ON THE NET AREA.
- M3. MORTAR SHALL CONFORM TO ASTM C270, TYPE S, AND SHALL DEVELOP A COMPRESSIVE STRENGTH OF 1,800 POUNDS PER SQUARE INCH AT 28 DAYS.
- M4. GROUT SHALL CONFORM TO ASTM C478, FINE TYPE, AND SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 2,500 POUNDS PER SQUARE INCH.
- M5. ALL REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60 (YIELD), REINFORCING BARS BEING WELDED SHALL CONFORM TO A706, GRADE 60. DEFORMED HORIZONTAL TRUSS REINFORCING MATERIAL SHALL CONFORM TO ASTM A485 AND SHALL HAVE MINIMUM CROSS-SECTIONAL AREAS INDICATED ON PLANS OR IN SECTIONS.
- M6. PROVIDE HORIZONTAL AND VERTICAL REINFORCING AS NOTED ON THE DRAWINGS AND FILL ALL REINFORCED BLOCK CELLS WITH GROUT.
- M7. WALLS SHALL BE GROUTED USING LOW LIFT GROUT METHOD AND LIMITING THE GROUT LIFT HEIGHT TO 4'-0" AT LOCATION OF THE BOND BEAM.
- M8. REINFORCE CMU AT ALL ELEVATORS, STAIRS, AND MEANS OF EGRESS AND EXTERIOR WALLS WITH A MINIMUM #7 @ 48" ON CENTER VERTICALLY.
- M9. PROVIDE 48 DIAMETER LAPS FOR ALL REINFORCING UNLESS NOTED OTHERWISE.

STRUCTURAL STEEL NOTES

- S1. ALL STEEL WORK SHALL CONFORM TO THE AISC 360-10 SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION AND TO THE NINTH EDITION OF THE MASSACHUSETTS STATE BUILDING CODE. IN CASE OF CONFLICT, THE MASSACHUSETTS STATE BUILDING CODE SHALL GOVERN.
- S2. THE STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING:
A. ALL STRUCTURAL WIDE FLANGE SHAPES: ASTM A992 FY = 50ksi,
HAVING A MINIMUM YIELD STRENGTH OF 50 KSI UNLESS OTHERWISE NOTED.
B. BARS, PLATES, CHANNELS, AND CONNECTION ANGLES: ASTM A36, UNLESS NOTED OTHERWISE.
C. STRUCTURAL TUBES: ASTM A500, GRADE C, FY = 50ksi,
D. STRUCTURAL PIPES: ASTM A53, GRADE B OR ASTM A501,
E. ALL ANCHOR BOLTS SHALL CONFORM TO ASTM F1554.
- S3. ALL WELDING SHALL BE IN ACCORDANCE WITH THE AWS D1.1 STRUCTURAL WELDING CODE.
- S4. BOTTOM OF DECK ELEVATIONS REFER TO TOP ELEVATION OF SUPPORTING ELEMENT INCLUDING JOISTS, BEAMS, PLATES, TEES, ANGLES, ETC.
- S5. TOP-OF-STEEL ELEVATIONS INDICATED ON THE DRAWINGS REFER TO TOP OF TOP-BEAM FLANGE UNLESS NOTED OTHERWISE ON PLANS.
- S6. DESIGN AND DETAIL OF ALL CONNECTIONS SHALL BE ACCORDING TO AISC.
- S7. THE BEAM CONNECTION DESIGN SHOULD ACCOUNT FOR REACTIONS OF MEMBERS SUPPORTED BY THE BEAM NEAR SUPPORTS CONCENTRATED LOADS WITHIN ONE THIRD THE SPAN OF THE BEAM CLOSER TO THE SUPPORT AND/OR VERTICAL COMPONENTS OF FORCE IN DIAGONAL BRACING MEMBERS FRAMING INTO THE MEMBERS.
- S8. ALL CONNECTIONS SHALL BE BOLTED WITH ASTM A325 HIGH-STRENGTH BOLTS OR WELDED IN ACCORDANCE TO AWS AND WITH THE AISC MANUAL REQUIREMENTS UNLESS NOTED OTHERWISE.
- S9. UNLESS OTHERWISE NOTED IN PLAN, DETAIL, FLOOR MEMBER CONNECTIONS FOR THE FOLLOWING FACTORED (LRFD) VERTICAL REACTIONS:
SHAPE MINIMUM REACTIONS (KIPS) TO COLUMNS MINIMUM NUMBER OF ROWS
W8W10 20 28 2
W12 30 36 3
W14 38 45 3
W16 45 54 3
W18 54 69 4
W21 63 75 4
W24 86 108 5
W27 105 129 6
W30 129 159 6
W33 153 183 8
E. SLAB REINFORCING - 3/4" TOP & BOTTOM 180 220 9
W44 198 242 10
- S10. SHOP CONNECTIONS MAY BE BOLTED OR WELDED, UNLESS THE CONNECTION METHOD IS SPECIFICALLY INDICATED ON THE STRUCTURAL DRAWINGS.
- S11. THE CONTRACTOR SHALL SUPPLY ALL PLATES, CLIPS, SET ANGLES, CONNECTIONS, ETC. AS REQUIRED FOR COMPLETION OF THE STRUCTURE. EVEN IF SUCH ITEMS ARE NOT EXPLICITLY CALLED FOR ON THE ARCHITECTURAL OR STRUCTURAL DRAWINGS.
- S12. THE CONTRACTOR SHALL PROVIDE ALL EMBEDDED PLATES, SLEEVES, BOX-OUTS, CONDUITS, ETCETERAS, AS REQUIRED BY OTHER TRADES IN THE CONCRETE STRUCTURE.
- S13. CONTRACTOR SHALL PROVIDE ALL NECESSARY TEMPORARY GUYING AND BRACING REQUIRED TO ERECT AND HOLD NEW STRUCTURE FOR WIND AND CONSTRUCTION LOADS. THE STEEL FRAME IS A NON-SUPPORTING STEEL FRAME IN ACCORDANCE WITH SECTION 7.3.1 OF THE AISC CODE OF STANDARD PRACTICE. IT RELIES ON THE INTERACTION BETWEEN THE METAL ROOF DECK, THE CONCRETE COMPOSITE FLOOR, BRACED FRAMES, AND MASONRY SHEAR WALLS FOR STABILITY. THE CONTRACTOR IS TO PROVIDE TEMPORARY SUPPORTS UNTIL ALL ELEMENTS REQUIRED FOR STABILITY OF THE STEEL FRAME ARE COMPLETED.
- S14. PROVIDE ANCHOR BOLTS, SETTING PLATES, AND EMBEDDED PLATES TO BE BY OTHERS.
- S15. PROVIDE 1/4" THICK LEVELING PLATES FOR USE IN ALIGNING AND SETTING ANCHOR BOLTS AND BASE PLATES.
- S16. MOMENT CONNECTIONS BETWEEN BEAMS AND COLUMNS INDICATED ON THE DRAWINGS SHALL BE DESIGNED AND DETAILED FOR THE FULL MOMENT CAPACITY OF THE CONNECTING MEMBERS, UNLESS NOTED OTHERWISE.
- S17. ALL TUBE STEEL COLUMN CAP PLATES ARE TO BE 1/2" THICK MINIMUM EXCEPT MOMENT FRAMED COLUMNS ARE TO BE 3/4" THICK MINIMUM OR MATCH BEAM FLANGE, WHICH EVER IS GREATER, UNLESS OTHERWISE NOTED.
- S18. PROVIDE 1/4" THICK CLOSURE PLATES AT ALL OPEN ENDED TUBE STEEL MEMBERS.
- S19. STEEL FRAMING SHALL NOT BE CANTED UNLESS SPECIFICALLY NOTED AS "CANTED" ON THE STRUCTURAL DRAWINGS. WHERE THE DECK SLOPE ACROSS THE BEARING SURFACE IS GREATER THAN 1:24, PROVIDE STEEL SHIMS FOR ADEQUATE BEARING OF STEEL DECK.

STEEL JOIST NOTES

- SJ1. ALL STEEL JOISTS SHALL CONFORM TO SJI K-10 "STANDARD SPECIFICATION FOR OPEN WEB STEEL JOISTS K-SERIES", AND TO SJI L-10 "STANDARD SPECIFICATION FOR DEEP LONGSPAN STEEL JOISTS L-SERIES", AND TO THE NINTH EDITION OF THE MASSACHUSETTS STATE BUILDING CODE. IN CASE OF CONFLICT, THE MASSACHUSETTS STATE BUILDING CODE SHALL GOVERN.
- SJ2. ALL WELDING WILL BE IN ACCORDANCE WITH THE AWS D1.1 STRUCTURAL WELDING CODE.
- SJ3. TOP AND BOTTOM CHORD OF ALL JOISTS SHALL BE OF DOUBLE ANGLES.
- SJ4. STARTING POINTS FOR ALL JOIST SPACING ARE BASED ON A JOIST OCCURRING AT COLUMN CENTER LINES UNLESS OTHERWISE NOTED ON PLANS.
- SJ5. ALL CONTINUOUS HORIZONTAL BRIDGING SHALL CONSIST OF TWO MEMBERS WITH ONE MEMBER AT TOP CHORD OF JOIST AND THE OTHER AT THE BOTTOM CHORD OF JOIST. ATTACH BY WELDING AT POINT OF CONTACT WITH EACH JOIST.
- SJ6. ALL BOLTED DIAGONAL BRIDGING SHALL CONSIST OF 2 MEMBERS BOLTED TO TOP CHORD OF JOIST AND BOTTOM CHORD OR FLANGE OF ADJACENT MEMBER AND BOLTED TOGETHER AT INTERSECTION.
- SJ7. DESIGN JOISTS AND PROVIDE UPLIFT BRIDGING FOR UPLIFT WIND LOADS AS PER SPECIFICATIONS. PROVIDE BRIDGING AT FIRST BOTTOM CHORD PAIR POINT AT BOTH ENDS OF JOIST.

STEEL DECK

- SD1. ALL STEEL DECK WORK SHALL CONFORM TO THE REQUIREMENTS OF
NC10 - 10 STANDARD FOR NONCOMPOSITE STEEL FLOOR DECK.
RD10 - 10 STANDARD FOR STEEL FLOOR DECK.
SD10 - 2011 STANDARD FOR COMPOSITE STEEL FLOOR DECK SLABS.
SD1 - 2011 STANDARD FOR COMPOSITE STEEL FLOOR DECK.
OF THE STEEL DECK INSTITUTE AND THE NINTH EDITION OF THE MASSACHUSETTS STATE BUILDING CODE. IN CASE OF CONFLICT, THE MASSACHUSETTS STATE BUILDING CODE SHALL GOVERN.
- SD2. STEEL DECK UNITS SHALL BE FORMED FROM STEEL SHEETS CONFORMING TO ASTM A611, OR A653, WHERE GALVANIZED DECK IS INDICATED. SHEETS SHALL BE COATED WITH A ZINC COATING CONFORMING TO ASTM A653, G-60 COATING FOR FLOOR DECK AND FORM DECK AND G-90 COATING FOR ROOF DECK.
- SD3. STEEL DECKING SHALL BE FASTENED TO ALL SUPPORTING STEEL MEMBERS AS FOLLOWS AND AS SHOWN ON THE STRUCTURAL DRAWINGS.
3" DEEP DECK
FIELD: 5/8" DIAMETER ROUND SPOT WELDS TO STEEL SUPPORTS AT EACH RIB, 8" ON CENTER.
PERIMETER: 3/4" DIAMETER ROUND SPOT WELDS TO STEEL SUPPORTS AT 8" ON CENTER.
OPENINGS: 3/4" DIAMETER ROUND SPOT WELDS TO STEEL SUPPORTS AT 8" ON CENTER.
CORNERS: TWO EACH 3/4" DIAMETER ROUND SPOT WELDS TO STEEL SUPPORTS
SIDEPLATES: NO. 10 TEK SCREWS AT 1'-0" ON CENTER AT SIDEPLATES BETWEEN SUPPORTS.
2" AND 3" DEEP COMPOSITE DECK
FIELD: 5/8" DIAMETER ROUND SPOT WELDS TO STEEL SUPPORTS AT EACH RIB, 12" ON CENTER.
PERIMETER: 5/8" DIAMETER ROUND SPOT WELDS TO STEEL SUPPORTS AT 12" ON CENTER.
OPENINGS: 5/8" DIAMETER ROUND SPOT WELDS TO STEEL SUPPORTS AT 12" ON CENTER.
CORNERS: ONE EACH 5/8" DIAMETER ROUND SPOT WELDS TO STEEL SUPPORTS
SIDEPLATES: NO. 10 TEK SCREWS AT 1'-0" ON CENTER ON CENTER OR CLOSER TO SUPPORT WET CONCRETE AND CONSTRUCTION LOADS AT SIDEPLATES BETWEEN SUPPORTS.
- SD4. STEEL DECK SIZES, GAGES, AND MINIMUM PROPERTIES SHALL BE AS INDICATED ON DRAWINGS OR IN SPECIFICATIONS.
- SD5. PROVIDE MINIMUM 16 GAGE SCORED ANGLE OR GREATER WHERE REQUIRED AT PERIMETER OF BUILDING AND ALL OPENINGS IN CONCRETE SLAB ON STEEL DECK, UNLESS OTHERWISE NOTED.
- SD6. HANGING FROM OR ATTACHING TO METAL DECK IS PROHIBITED. ALL DUCTS, PIPES, CONDUITS SHALL BE SUPPORTED FROM STEEL FRAMING OR SUPPLEMENTAL STEEL FRAMING PROVIDED BY THE CONTRACTOR OR SUBCONTRACTOR.

DESIGN LOADS

- D1. SNOW LOADS
ROOF SNOW LOAD, Pg = 50 PSF
MINIMUM FLAT ROOF SNOW LOAD Pf = 30 PSF
FLAT ROOF SNOW LOAD PE = 30 PSF (PARTIALLY EXPOSED)
ALLOWANCE FOR DRIFTING PER MASSACHUSETTS STATE BUILDING CODE
- D2. FLOOR LIVE LOADS
MECHANICAL ROOMS 150 PSF MIN
PUBLIC GATHERING AREAS 100 PSF
STAIRS/LOBBY 100 PSF
STORAGE 125 PSF
TYPICAL CONCENTRATED LOAD 1,000 POUNDS
FUTURE PHOTOVOLTAIC PANELS 15 PSF
CLASSROOMS 40PSF + 15 PSF PARTITIONS
LIBRARY/READING ROOM 60PSF
CORRIDORS ABOVE FIRST FLOOR 80PSF
CONCENTRATED LOAD IN AUDITORIUM 3,000 LBS
STAGE LOADING PLATFORM 400 PSF
TOILETS 60 PSF
ELEVATOR MACHINE ROOM 250 PSF
- D3. WIND LOADS
AS PER THE NINTH EDITION OF THE MASSACHUSETTS STATE BUILDING CODE.
BASIC WIND SPEED 137 M.P.H.
EXPOSURE C
WIND LOADS IN ACCORDANCE WITH ASCE7-10
- D4. SEISMIC LOADS
AS PER THE NINTH EDITION OF THE MASSACHUSETTS STATE BUILDING CODE
SEISMIC DESIGN CATEGORY B
SOIL PROFILE TYPE SITE CLASS C
MAPPED SPECTRAL RESPONSE ACCELERATIONS Ss = 0.232
S1 = 0.072
- BASIC SEISMIC FORCE RESISTING SYSTEMS
STRUCTURAL STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE
RESPONSE MODIFICATION FACTOR R = 3.0
DEFLECTION AMPLIFICATION FACTOR Cd = 3.0
ANALYSIS PROCEDURE EQUIVALENT LATERAL FORCE
SEISMIC IMPORTANCE FACTOR, I = 1.25
- RISK CATEGORY 3
SPECTRAL RESPONSE COEFFICIENT Sds = 0.174
SD1
AREA A AND B (TOTAL COMBINED) CS = 0.073
SEISMIC RESPONSE COEFFICIENT CS = 0.073
DESIGN BASE SHEAR V = 320 KIPS
AREA C AND D
SEISMIC RESPONSE COEFFICIENT CS = 0.073
DESIGN BASE SHEAR V = 170 KIPS

EXPOSED STEEL NOTES

- SPECIAL CARE USED IN THE HANDLING AND FABRICATING OF EXPOSED STEEL INDICATED ON THE DRAWINGS AND AS FOLLOWS:
- LOCATE FIELD JOINTS AT CONCEALED LOCATIONS IF POSSIBLE.
 - FABRICATE WITH EXPOSED SURFACES SMOOTH, SQUARE AND FREE OF SURFACE BLEMISHES INCLUDING PITTING, RUST, SCALE AND ROUGHNESS.
 - GRIND SHEARED, PUNCHED AND FLAME-CUT EDGES TO REMOVE BURRS AND PROVIDE SMOOTH SURFACES AND EDGES.
 - FABRICATE STEEL WITH EXPOSED SURFACES FREE OF MILL MARKS, INCLUDING ROLLED TRADE NAMES AND STAMPED OR RAISED IDENTIFICATION.
 - FABRICATE STEEL WITH EXPOSED SURFACES FREE OF SEAMS TO MAXIMUM EXTENT POSSIBLE.
 - REMOVE BLEMISHES BY FILLING OR GRINDING OR BY WELDING AND GRINDING, BEFORE CLEANING, TREATING AND SHOP PRIMING.
 - FABRICATE WITH PIECE MARKS FULLY HIDDEN IN THE COMPLETED STRUCTURE OR MADE WITH MEDIA THAT PERMITS FULL REMOVAL AFTER ERECTION.
 - ALL EXPOSED TO VIEW STEEL, NOT INDICATED TO RECEIVE SPRAY APPLIED FIREPROOFING SHALL BE SHOP PRIMED. COORDINATE WITH INTUMESCENT AND SPRAY FIREPROOFING WHERE REQUIRED.

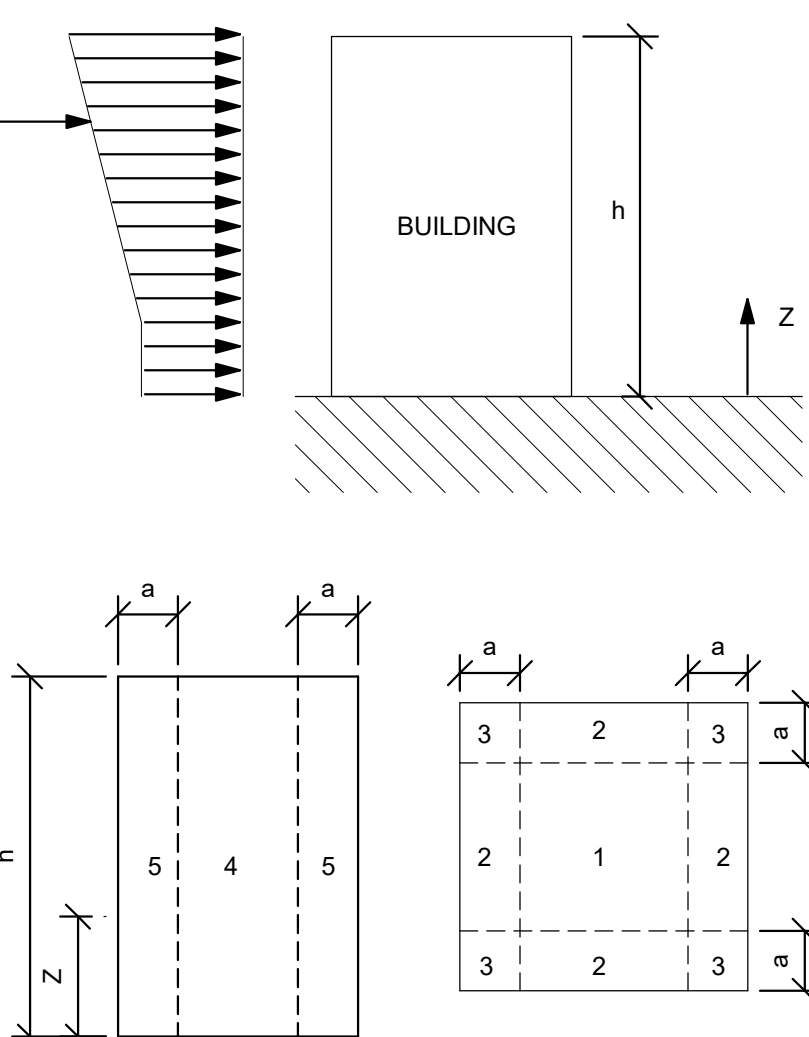
UNIT PRICES - PROVIDE UNIT PRICES AS PART OF THE BID FOR THE FOLLOWING:

- 1) STRUCTURAL STEEL BEAMS, ANGLES, PLATES, COLUMNS, ETC., THE UNIT PRICE SHALL INCLUDE WEIGHT OF THE STEEL, COST OF DETAILING, FABRICATION, DELIVERY AND INSTALLATION. PROVIDE THE UNIT PRICE FOR THE FOLLOWING, USING UNIT MEASUREMENT AS TON.
A. STRUCTURAL STEEL UNIT WEIGHT UP TO 15 LB/LF
B. STRUCTURAL STEEL UNIT WEIGHT 15 TO 30 LB/LF
C. STRUCTURAL STEEL UNIT WEIGHT 30 TO 60 LB/LF
D. STRUCTURAL STEEL UNIT WEIGHT 60 TO 120 LB/LF
- 2) COST OF REINFORCEMENT OF BEAMS IN THE SHOP FOR PENETRATIONS FOR DUCTS AND OTHER UTILITIES PER DETAIL 5 ON DRAWING 50-0-6 MINIMUM OF 10 LOCATIONS. IN ADDITION, PROVIDE UNIT COST OF EACH ADDITIONAL REINFORCEMENT DETAIL AND CREDIT FOR UNIT COST OF EACH DETAIL THAT IS REDUCED FROM ALLOWANCE OF MINIMUM 10 UNITS.
- 3) COST OF PROVIDING UNREINFORCED PENETRATIONS THROUGH STEEL BEAMS PER DETAIL 6 ON DRAWING 50-0-6 FOR DUCTS AND OTHER UTILITIES AT A MINIMUM OF 10 LOCATIONS. IN ADDITION, PROVIDE UNIT COST OF EACH ADDITIONAL REINFORCEMENT DETAIL AND CREDIT FOR UNIT COST OF EACH DETAIL THAT IS REDUCED FROM ALLOWANCE OF MINIMUM 10 PENETRATIONS.
- 4) FRAMES FOR OPENINGS THROUGH ROOF PER DETAIL 2B AND 3 ON DRAWING 50-0-8. IN ADDITION, PROVIDE A UNIT COST IF THESE FRAMES WERE TO BE INSTALLED AFTER ALL OF THE STRUCTURAL STEEL HAS BEEN ERECTED. IN THE BASE BID, ALLOW FOR THE COST OF THIS DETAIL FOR ALL OPENINGS SHOWN IN THE STRUCTURAL, ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS. PROVIDE A UNIT COST OR CREDIT FOR EACH DETAIL THAT IS REDUCED FROM THE OPENINGS SHOWN ON THE STRUCTURAL, ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS.
- 5) FRAMES FOR OPENINGS THROUGH FLOOR AND ROOF PER DETAIL 1 ON DRAWING 50-0-8. IN ADDITION, PROVIDE A UNIT COST IF THESE FRAMES WERE TO BE INSTALLED AFTER ALL OF THE STRUCTURAL STEEL HAS BEEN ERECTED. IN THE BASE BID, ALLOW FOR THE COST OF THIS DETAIL FOR ALL OPENINGS SHOWN IN THE STRUCTURAL, ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS. PROVIDE A UNIT COST OR CREDIT FOR EACH DETAIL THAT IS REDUCED FROM THE OPENINGS SHOWN ON THE STRUCTURAL, ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS.
- 6) COST OF ONE SQUARE FOOT OF VARIOUS TYPES OF DECK SPECIFIED FOR THE PROJECT. THE UNIT PRICE SHALL INCLUDE MATERIAL COST OF THE DECK AND ACCESSORIES, COST OF DETAILING, FABRICATION, DELIVERY AND INSTALLATION.

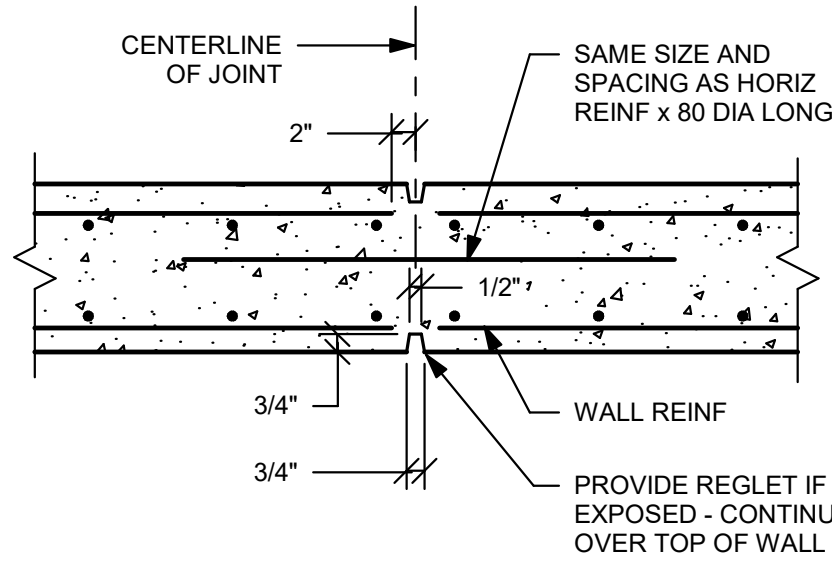
NOTES:

- FOR CONSTANTS K2T, Kd, Gc, Qc, EXPOSURE "IMPORTANCE FACTOR OR RISK CATEGORY" REFER TO STRUCTURAL DESIGN LOADS ON GENERAL NOTES.
- FOR DESIGN WIND PRESSURE EQUATIONS, REFER TO EQUATION (6-22)
- WIND LOAD ZONE REIGNS ARE DIAGRAMMATIC, ONLY FOR APPLICATION OF LOADS. REFER TO GEOMETRY - SPECIFIC DIAGRAMS IN ASCE 7.

DESIGN WIND PRESSURE - COMPONENTS & CLADDING AREAS A & B				DESIGN WIND PRESSURE - COMPONENTS & CLADDING AREAS A & B			
WALLS				WALLS			
EFFECTIVE WIND AREA				EFFECTIVE WIND AREA			
ZONE	10 SF	100 SF	500 SF	ZONE	10 SF	100 SF	500 SF
4 - FIELD	53 PSF	43 PSF	37 PSF	4 - FIELD	56 PSF	46 PSF	39 PSF
	-53 PSF	-47 PSF	-42 PSF		-56 PSF	-49 PSF	-45 PSF
5 - EDGE	53 PSF	43 PSF	37 PSF	5 - EDGE	56 PSF	46 PSF	39 PSF
	-56 PSF	-73 PSF	-68 PSF		-102 PSF	-78 PSF	-61 PSF
ROOFS				ROOFS			
ZONE	10 SF	50 SF	100 SF	ZONE	10 SF	50 SF	100 SF
1 - FIELD	NA	-77 PSF	NA	1 - FIELD	NA	-72 PSF	NA
	-77 PSF	NA	-63 PSF		-82 PSF	NA	-67 PSF
2 - EDGE	NA	-121 PSF	NA	2 - EDGE	NA	-113 PSF	NA
	-121 PSF	NA	-99 PSF		-128 PSF	NA	-105 PSF
3 - CORNER	NA	-164 PSF	NA	3 - CORNER	NA	-173 PSF	NA
	-164 PSF	NA	-134 PSF		-173 PSF	NA	-144 PSF



WALL ELEVATIONS		ROOF PLANS	
AREAS A & B	CORNER DIMENSIONS	a = 20 FT	
	HEIGHT	h = 62 FT	
	EXPOSURE COEFFICIENT	Kh = 1.13	
AREAS C & D	CORNER DIMENSIONS	a = 20 FT	
	HEIGHT	h = 82 FT	
	EXPOSURE COEFFICIENT	Kh = 1.21	

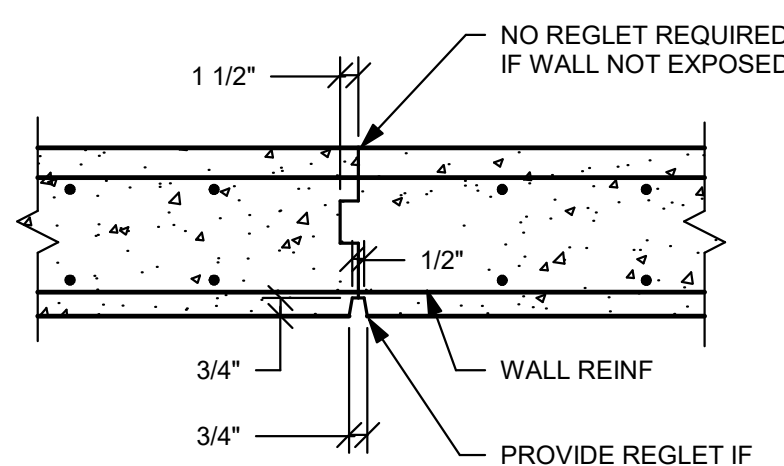


TYPICAL CONCRETE WALL CONTROL JOINT DETAIL

- NOTES:
- 1) SPACE AT 30" - 0" CENTER TO CENTER MAX.
 - 2) A CONSTRUCTION JOINT MAY BE SUBSTITUTED FOR A CONTROL JOINT, SEE CONSTRUCTION JOINT DETAIL.

1

NO SCALE

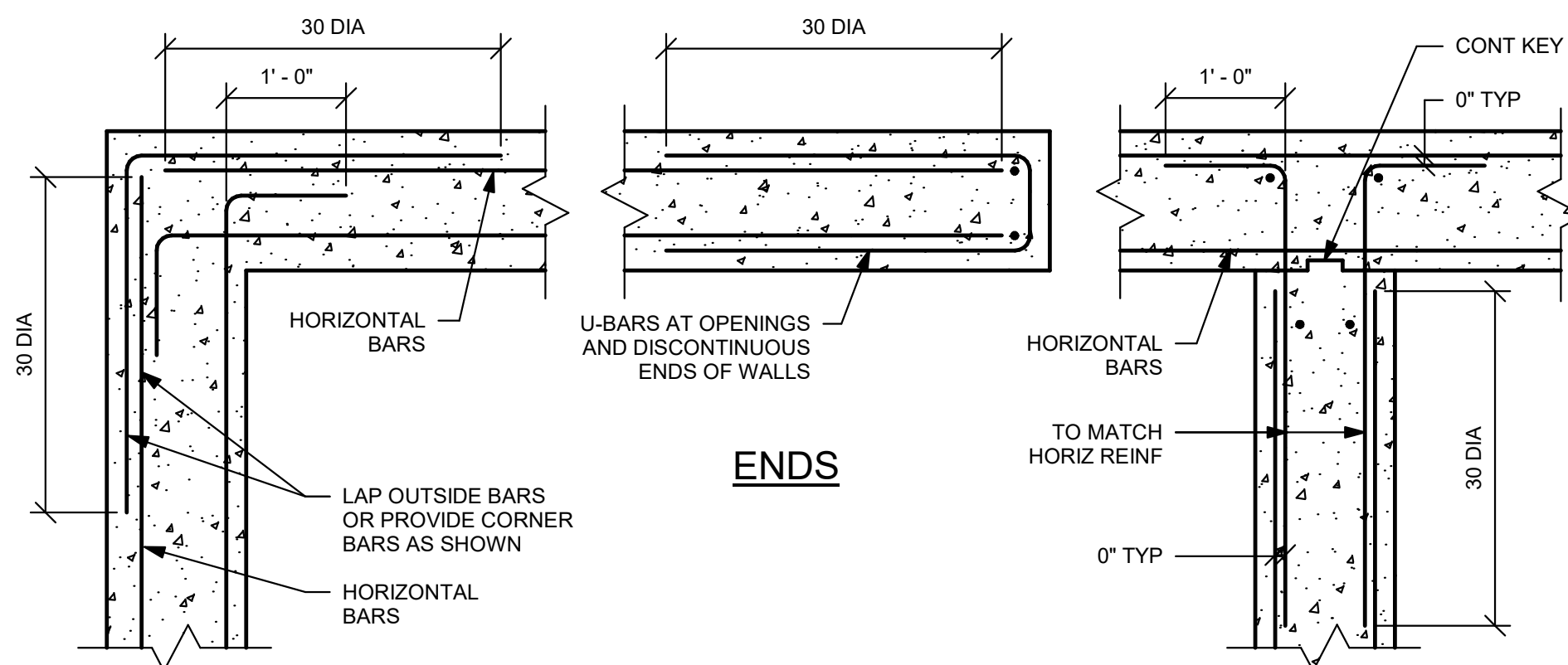


TYPICAL CONCRETE WALL CONSTRUCTION JOINT DETAIL

- NOTE:
- SPACE AT 60" - 0" CENTER TO CENTER MAX.

2

NO SCALE



CORNERS

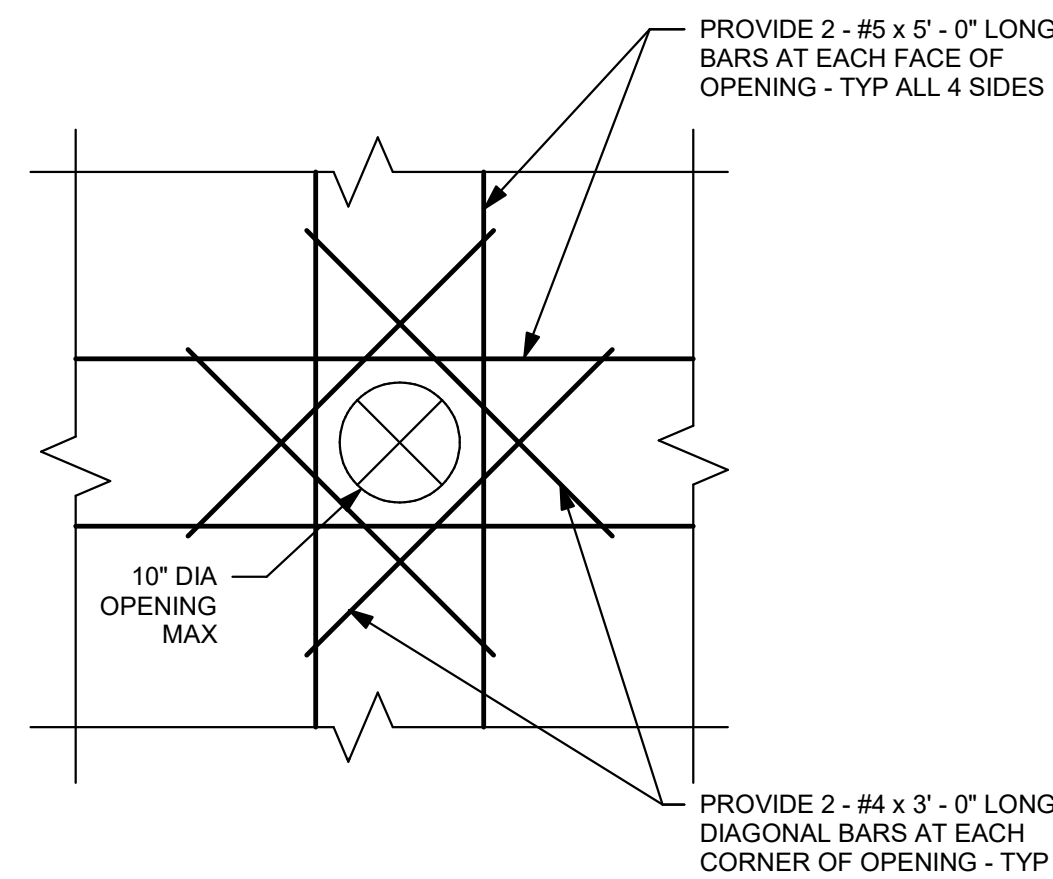
ENDS

INTERSECTIONS

TYPICAL PLAN OF HORIZONTAL REINFORCING OF CONCRETE WALLS DETAIL

3

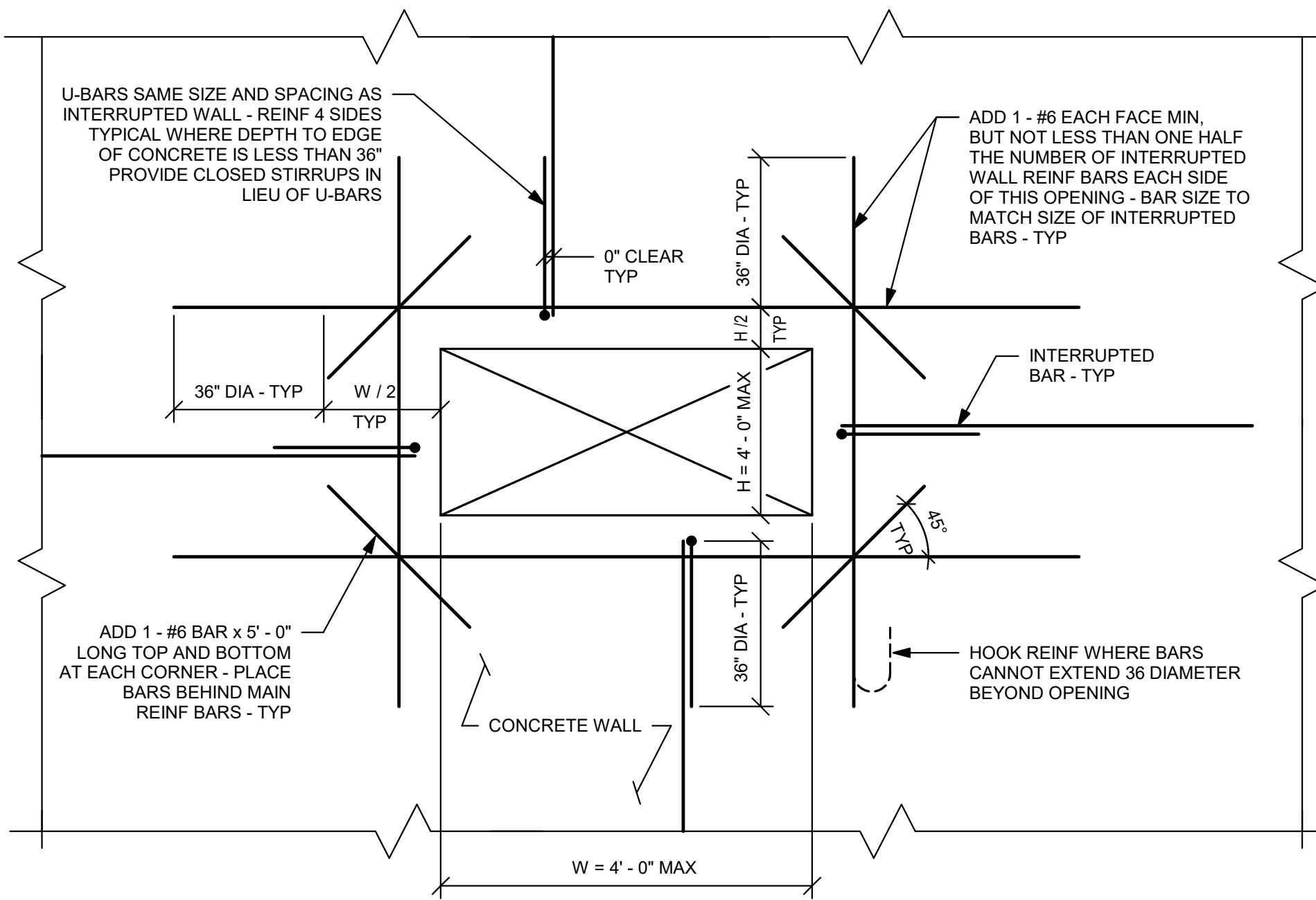
NO SCALE



- NOTE:
- THE SLEEVE SHALL NOT INTERRUPT OR CUT THROUGH THE VERTICAL REINFORCING STEEL AND SHALL NOT BE PLACED IN COLUMN PIERS OR PLASTERS.

4

NO SCALE



TYPICAL REINFORCEMENT AT OPENINGS IN REINFORCED CONCRETE WALLS DETAIL

- NOTES:
- 1) FOR QUANTITY, LOCATION, AND SIZES REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS.
 - 2) TREAT EACH CONDUIT AS A SEPARATE OPENING.
 - 3) PROVIDE A MINIMUM OF 12" CLEAR CONCRETE BETWEEN TOP OF PENETRATION AND TOP OF WALL.

5

NO SCALE

BAR SIZE	ℓd PER SPACING AND COVER CASE			
	CASE 1	CASE 2	CASE 1	CASE 2
	TOP BARS	OTHER BARS	TOP BARS	OTHER BARS
#3	19	15	28	22
#4	25	19	37	29
#5	31	24	47	36
#6	37	29	56	43
#7	44	37	67	51
#8	51	44	81	61
#9	61	51	100	75
#10	75	61	125	94
#11	94	75	156	117
#14	125	94	209	161
#18	209	161	299	229

TENSION DEVELOPMENT LENGTHS, ℓd (INCHES) FOR GRADE 60 UNCOATED BARS
fc = 4500 psi; NORMAL-WEIGHT CONCRETE
BASED ON ACI 12.2.2

BAR SIZE	LAP CLASS	LAP LENGTH PER SPACING AND COVER CASE			
		CASE 1	CASE 2	CASE 1	CASE 2
		TOP BARS	OTHER BARS	TOP BARS	OTHER BARS
#3	B	24	19	36	28
#4	B	32	25	48	37
#5	B	40	31	60	47
#6	B	48	37	72	56
#7	B	56	44	84	67
#8	B	64	51	96	75
#9	B	72	58	108	84
#10	B	80	66	120	93
#11	B	88	74	132	102
#14	B	112	92	168	128
#18	B	144	116	216	164

TENSION LAP SPLICE LENGTHS, ℓs (INCHES) FOR GRADE 60 UNCOATED BARS
fc = 4500 psi; NORMAL-WEIGHT CONCRETE
BASED ON ACI 12.2.2

BAR SIZE	ℓd PER SPACING AND COVER CASE			
	CASE 1	CASE 2	CASE 1	CASE 2
	TOP BARS	OTHER BARS	TOP BARS	OTHER BARS
#3	17	13	25	19
#4	22	17	33	26
#5	28	22	42	32
#6	33	26	50	38
#7	40	33	60	46
#8	46	39	72	55
#9	53	46	84	64
#10	60	53	96	73
#11	67	60	108	81
#14	84	72	144	108
#18	125	96	216	164

TENSION DEVELOPMENT LENGTHS, ℓd (INCHES) FOR GRADE 60 UNCOATED BARS
fc = 4500 psi; NORMAL-WEIGHT CONCRETE
BASED ON ACI 12.2.2

BAR SIZE	LAP CLASS	LAP LENGTH PER SPACING AND COVER CASE			
		CASE 1	CASE 2	CASE 1	CASE 2
		TOP BARS	OTHER BARS	TOP BARS	OTHER BARS
#3	B	22	17	33	25
#4	B	29	22	43	33
#5	B	36	28	54	42
#6	B	43	33	65	50
#7	B	50	40	77	60
#8	B	57	47	88	69
#9	B	64	54	99	77
#10	B	71	61	110	85
#11	B	78	68	121	93
#14	B	99	88	156	121
#18	B	132	116	208	161

TENSION LAP SPLICE LENGTHS, ℓs (INCHES) FOR GRADE 60 UNCOATED BARS
fc = 4500 psi; NORMAL-WEIGHT CONCRETE
BASED ON ACI 12.2.2

NOTES:

- 1) TOP BARS ARE DEFINED AS HORIZONTAL BARS WITH MORE THAN 12 INCHES OF CONCRETE CAST IN THE MEMBER BELOW THE REINFORCEMENT. WALL REINFORCEMENT IS CLASSIFIED AS OTHER BARS.
- 2) FOR LIGHTWEIGHT AGGREGATE CONCRETE MULTIPLY THE VALUES ABOVE BY 1.3.

ABBREVIATIONS:

- db DENOTES NOMINAL BAR DIAMETER
> DENOTES GREATER THAN
≥ DENOTES EQUAL TO OR GREATER THAN
< DENOTES LESS THAN
≤ DENOTES EQUAL TO OR LESS THAN

CASE 1 BEAMS AND COLUMNS: CONCRETE COVER ≥ 2 db, C-C, BAR SPACING ≥ 2 db, AND WITH STIRRUPS OR TIES THROUGHOUT (4" NOT LESS THAN THE CODE MINIMUM FOR GRADE 60 UNCOATED BARS)

CASE 2 BEAMS AND COLUMNS: CONCRETE COVER < 2 db AND C-C, BAR SPACING < 2 db, OR C-C, BAR SPACING < 3 db

4,500 PSI NORMAL-WEIGHT CONCRETE

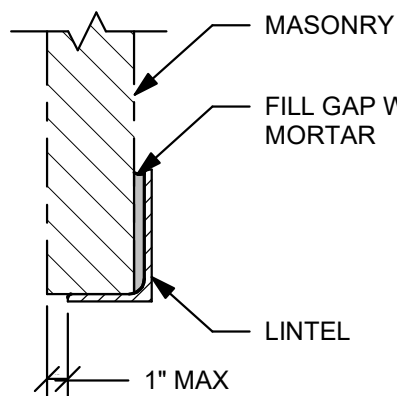
5,000 PSI NORMAL-WEIGHT CONCRETE

MINIMUM SPLICE AND EMBEDMENT LENGTH SCHEDULE

(UNLESS SHOWN OTHERWISE ON DRAWINGS)

NOTES:

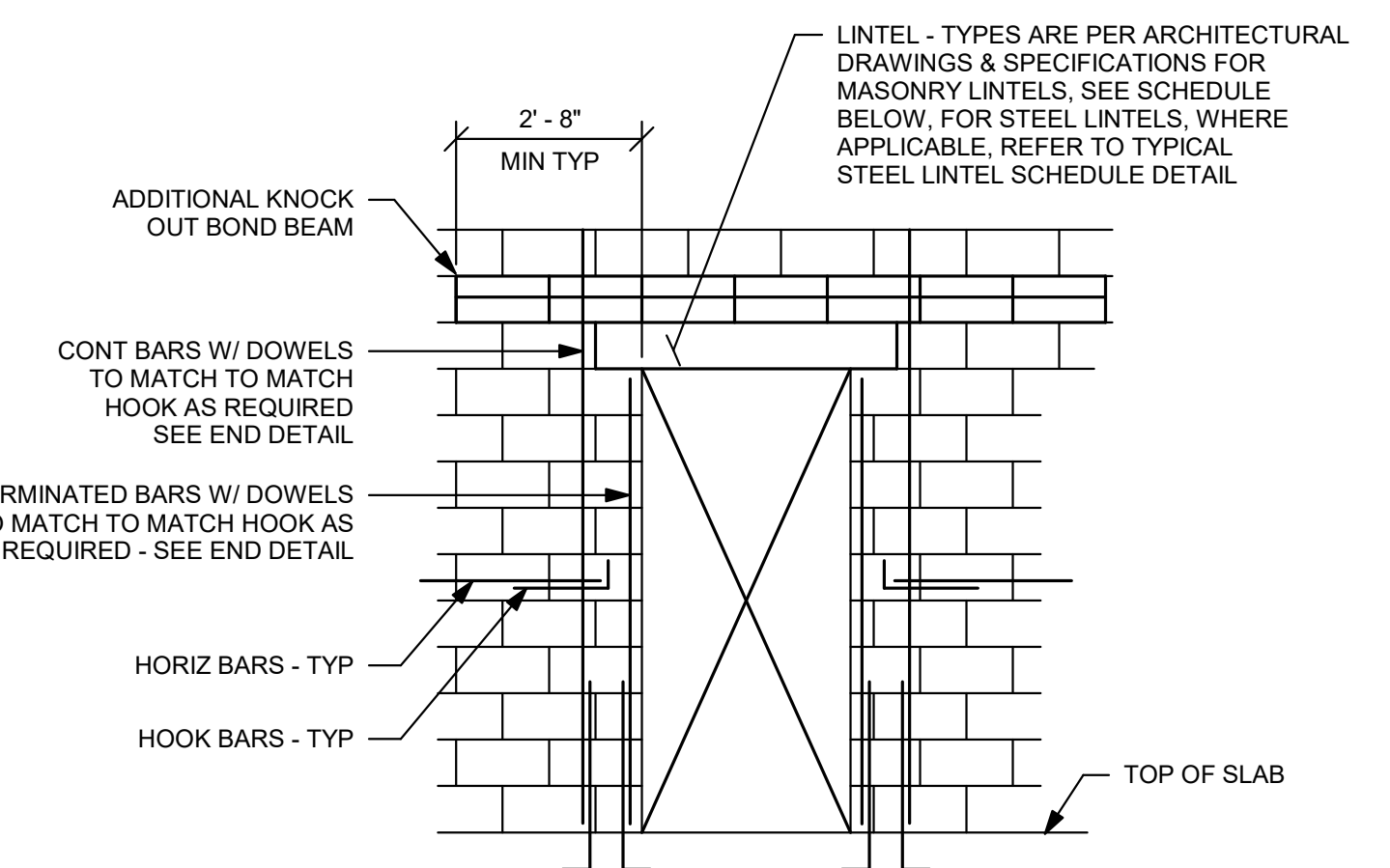
- 1) PROVIDE LINTELS OVER ALL MASONRY OPENINGS UNLESS OTHERWISE NOTED OR DETAILED.
- 2) PROVIDE ONE ANGLE FOR EACH 4" OF WALL THICKNESS. FOR 8" WALLS PROVIDE TEE, DOUBLE ANGLE OR BUILT-UP SECTION WITH PROPERTIES EQUAL TO OR GREATER THAN 1-1/2" TIMES ANGLE PROPERTIES FOR 4" WALL.
- 3) PROVIDE 8" OF BEARING EACH END OF ALL LINTELS.
- 4) SPAN LENGTH = CENTERLINE TO CENTERLINE OF BEARING.
- 5) ALL EXTERIOR LINTELS SHALL BE GALVANIZED. PROVIDE 1/4" THICK CLOSURE PLATE OVER AIR SPACE AT OPENINGS UNLESS NOTED OR DETAILED OTHERWISE ON ARCHITECTURAL DRAWINGS.
- 6) FOR CURVED LINTELS USE CHORD LENGTH IN CONJUNCTION WITH SCHEDULE ABOVE. PROVIDE HORIZONTAL ANGLES AT EACH END OF LINTEL FOR 8" OF BEARING. HORIZONTAL ANGLES SHALL MATCH LINTEL SIZE AND SHALL BE WELDED TO CURVED ANGLE WITH FULL PENETRATION WELD.
- 7) LOOSE LINTELS SHALL BE FURNISHED BY METAL FABRICATORS (SPECIFICATION 05000) AND INSTALLED BY UNIT MASONRY ASSEMBLIES (SPECIFICATION 04200).
- 8) LOOSE LINTELS ARE REQUIRED FOR ALL OPENINGS INCLUDING DOORS, WINDOWS, MECHANICAL DUCTS, PIPES ETC.
- 9) ALL THE LINTELS NOT ATTACHED TO STRUCTURAL STEEL ARE IN THE SCOPE OF THE METAL FABRICATION CONTRACTOR (SPECIFICATION 05000). SEE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS. ALL THE LINTELS WELDED TO STRUCTURAL STEEL ARE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR AND THE STRUCTURAL STEEL FABRICATOR.



TYPICAL SECTION SINGLE WYTHE MASONRY

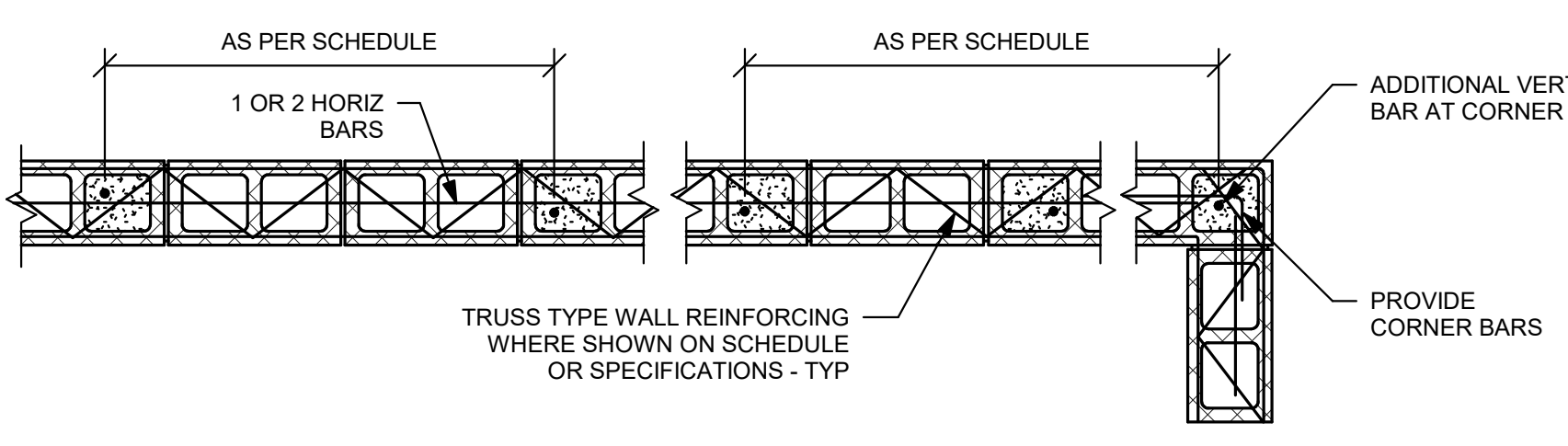
TYPICAL STEEL LINTEL SCHEDULE

LINTEL SCHEDULE	
MASONRY OPENING	LINTEL SIZE
UP TO 4' - 0"	L 4" x 3-1/2" x 5/16" (4" LEG VERTICAL)
4' - 7" TO 6' - 0"	L 5" x 3-1/2" x 5/16" (5" LEG VERTICAL)
6' - 1" TO 8' - 0"	L 6" x 3-1/2" x 3/8" (6" LEG VERTICAL)
8' - 1" TO 10' - 0"	L 7" x 4" x 3/8" (7" LEG VERTICAL)



TYPICAL DETAIL

CORNER DETAIL



BOND BEAM DETAIL

- NOTES:
- 1) SEE SCHEDULE FOR SPACING.
 - 2) PROVIDE REINFORCED BOND BEAM WITHIN 16" OF TOP OF WALL.
 - 3) PROVIDE REINFORCED BOND BEAM AT TOP AND BOTTOM OF ALL OPENINGS.

MASONRY LINTEL DETAIL AND SCHEDULE

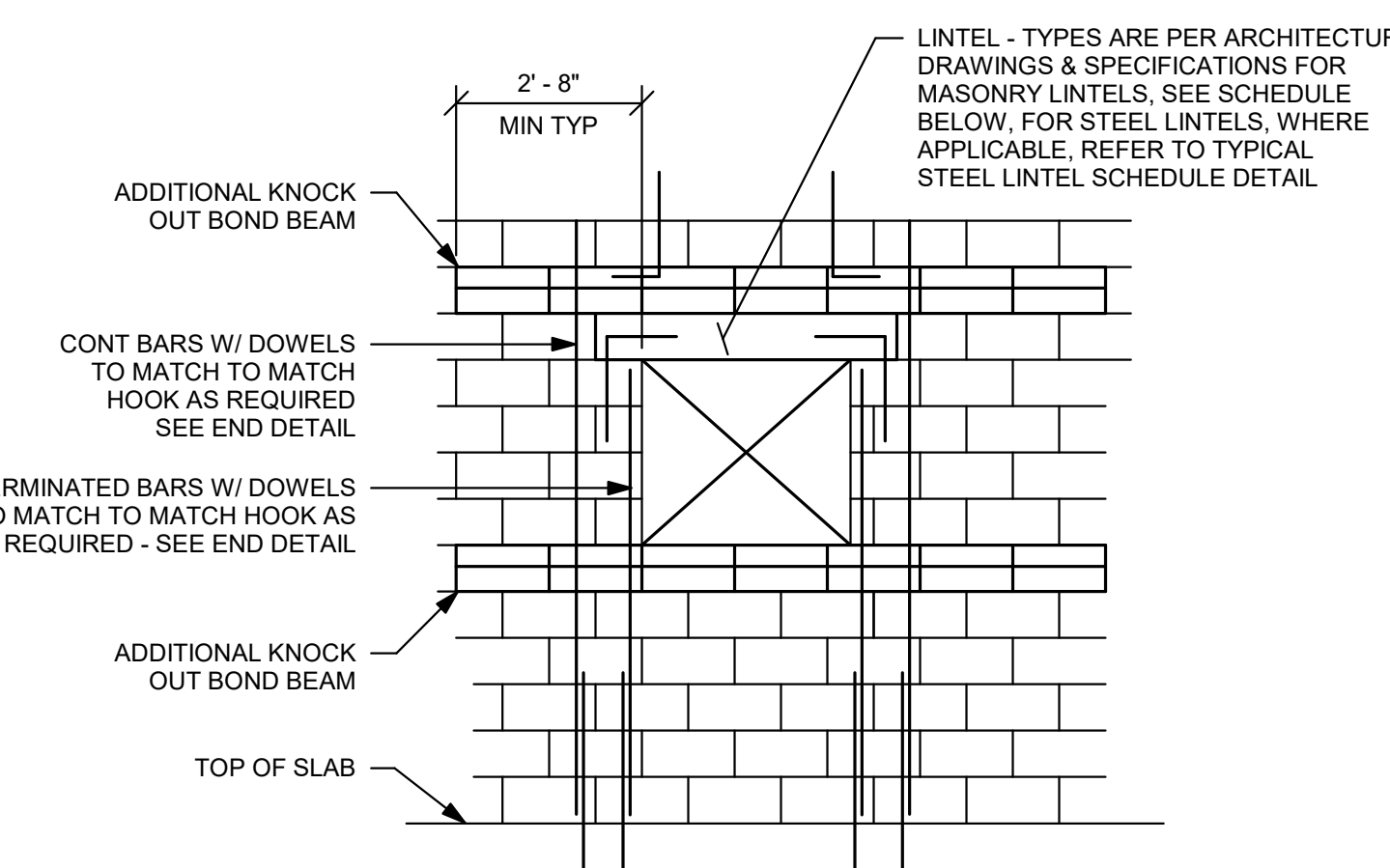
NOTE: NO CONSTRUCTION JOINTS OR CONTROL JOINTS ARE PERMITTED WITHIN 3' - 0" OF EDGE OF OPENING.

MASONRY LINTEL SCHEDULE		
OPENING DIMS	8" OR 12" WIDE BEAM	REINFORCEMENT
0' - 0" - 4' - 0"	8" x 8" DEEP	2 - #5 CONT
4' - 0" - 8' - 0"	8" x 8" DEEP	2 - #5 CONT
8' - 0" - 12' - 0"	8" x 24" DEEP	2 - #5 CONT AND WIRE TRUSS TYPE - REINF AT JOINTS
0' - 0" - 4' - 0"	12" x 8" DEEP	2 - #5 CONT
4' - 0" - 8' - 0"	12" x 16" DEEP	2 - #5 CONT AND WIRE TRUSS TYPE - REINF AT JOINTS
8' - 0" - 12' - 0"	12" x 24" DEEP	2 - #5 TOP & BOT CONT AND WIRE TRUSS TYPE REINF AT JOINTS

MINIMUM CONCRETE MASONRY WALL REINFORCING SCHEDULE			
WALL LOCATION	WALL THICKNESS	VERTICAL REINFORCING	HORIZONTAL REINFORCING
SHEAR WALLS AND LOADING BEARING SHEAR WALLS SHOWN ON PLAN	8"	#3 @ 48"	1 - #5 IN BOND BEAM AT 48" ON CENTER
CLASS 'A' WALLS	8"	#6 @ 48"	1 - #5 IN BOND BEAM AT 48" ON CENTER
ALL EXTERIOR WALLS, STAIR WALLS, AND ELEVATOR SHAFT WALLS	8"	#7 @ 48"	1 - #5 IN BOND BEAM AT 48" ON CENTER
CLASS 'B' WALLS	12"	#8 @ 48"	2 - #5 IN BOND BEAM AT 48" ON CENTER
ALL INTERIOR CMU WALLS GREATER THAN 16' - 0" IN HEIGHT	ALL SIZES	#4 @ 48"	1 - #4 IN BOND BEAM AT 48" ON CENTER
CLASS 'C' WALLS	ALL SIZES	#4 @ 48"	1 - #4 IN BOND BEAM AT 48" ON CENTER

- NOTES:
- 1) REFER TO PLANS, SECTIONS, AND SPECIFICATIONS FOR REINFORCING REQUIREMENTS MORE STRINGENT THAN IN THE SCHEDULE.
 - 2) PROVIDE REINFORCED BOND BEAM WITHIN 16" OF TOP OF WALL.
 - 3) ALL VERTICAL REINFORCING TO BE IN SOLIDLY GROUTED CELLS. AND PROVIDE 64 DIAMETER LAP AT ALL BAR SPLICES TYPICAL.
 - 4) PROVIDE 9 GA HORIZONTAL JOINT REINFORCING AT 16" OC FOR ALL WALLS.

NOTE: ALL REINFORCING WITHIN THE MASONRY WALL SHALL BE FURNISHED BY THE MASONRY SUB CONTRACTOR (SPECIFICATION 04200). EXCEPT BONES EMBEDDED IN CONCRETE FOUNDATION ARE THE RESPONSIBILITY TO THE GENERAL CONTRACTOR AND THE CONCRETE SUB-CONTRACTOR



VERTICAL CONTROL JOINT DETAIL

END DETAIL

- NOTES:
- 1) PROVIDE 1 - CONTINUOUS BAR AT MASONRY OPENING 4' - 0" OR LESS IN WIDTH AT STRUTTED ENDS.
 - 2) PROVIDE 2 - CONTINUOUS BARS AT MASONRY OPENING 4' - 0" TO 8' - 0" IN WIDTH.

TYPICAL REINFORCING AT CMU WALLS WITH OPENINGS ELEVATION

8

NO SCALE

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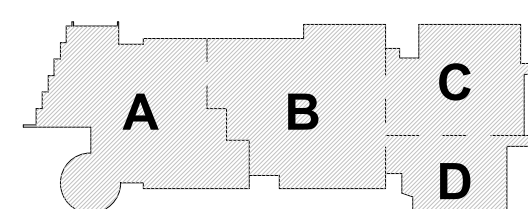
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03/31/2023 EARLY STRUCTURAL BID PACKAGE

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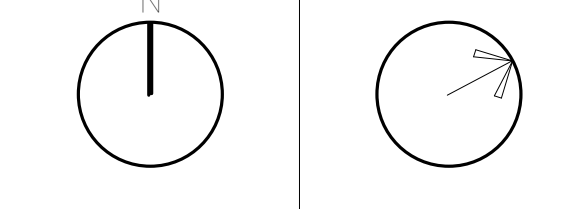
BID SET

August 28th, 2023



KEY PLAN

PROJECT NORTH MAGNETIC NORTH



TYPICAL DETAILS

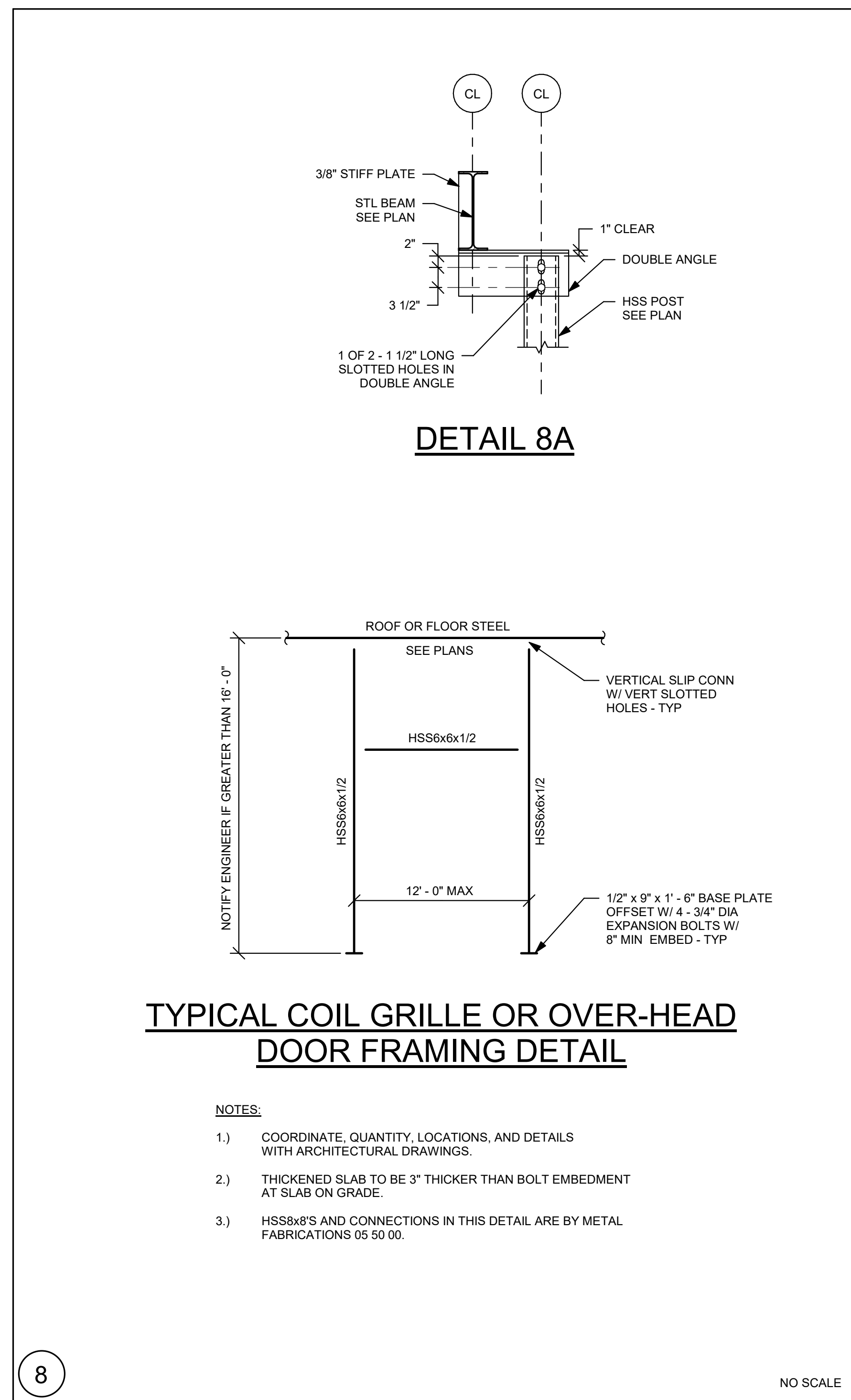
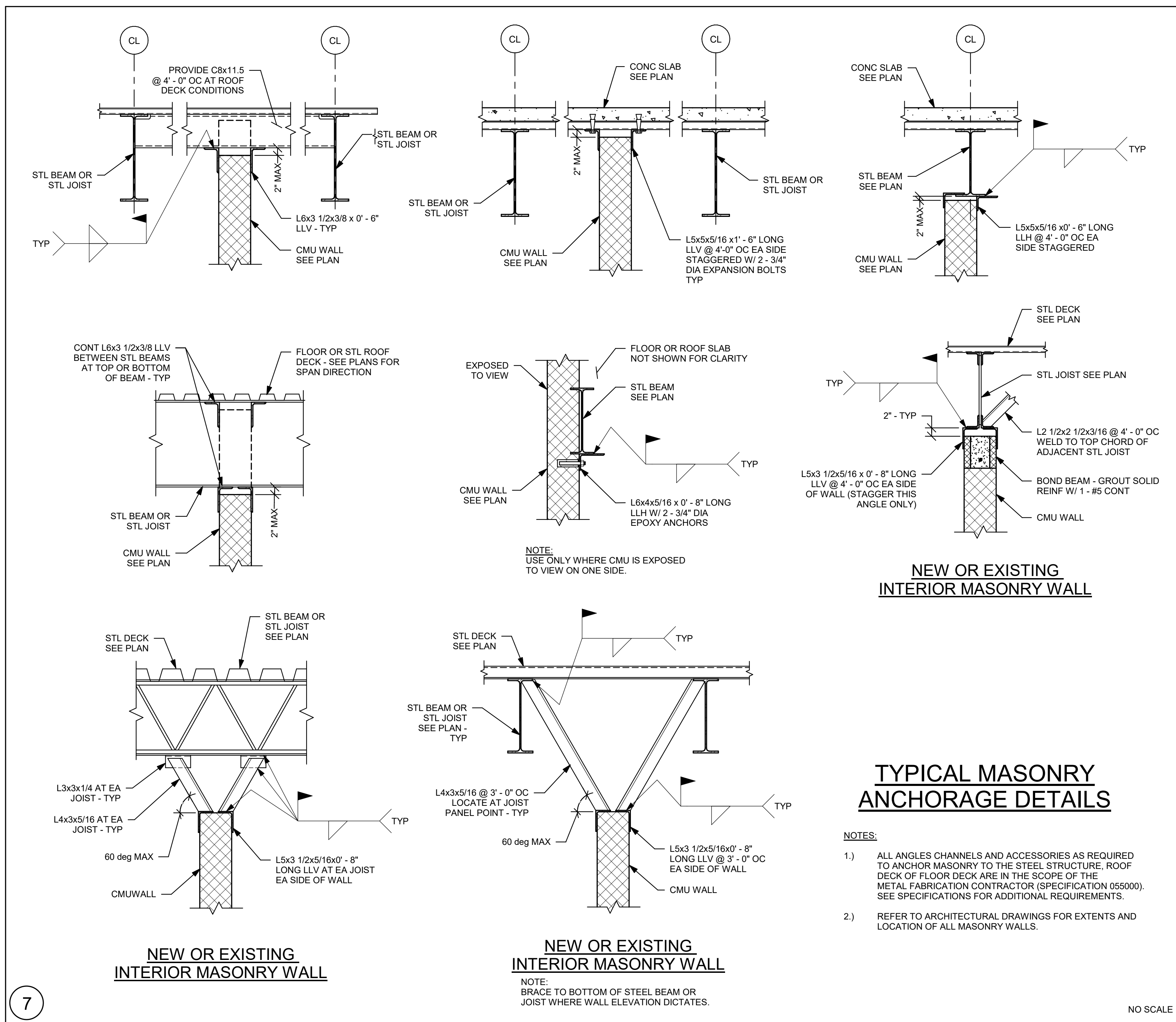
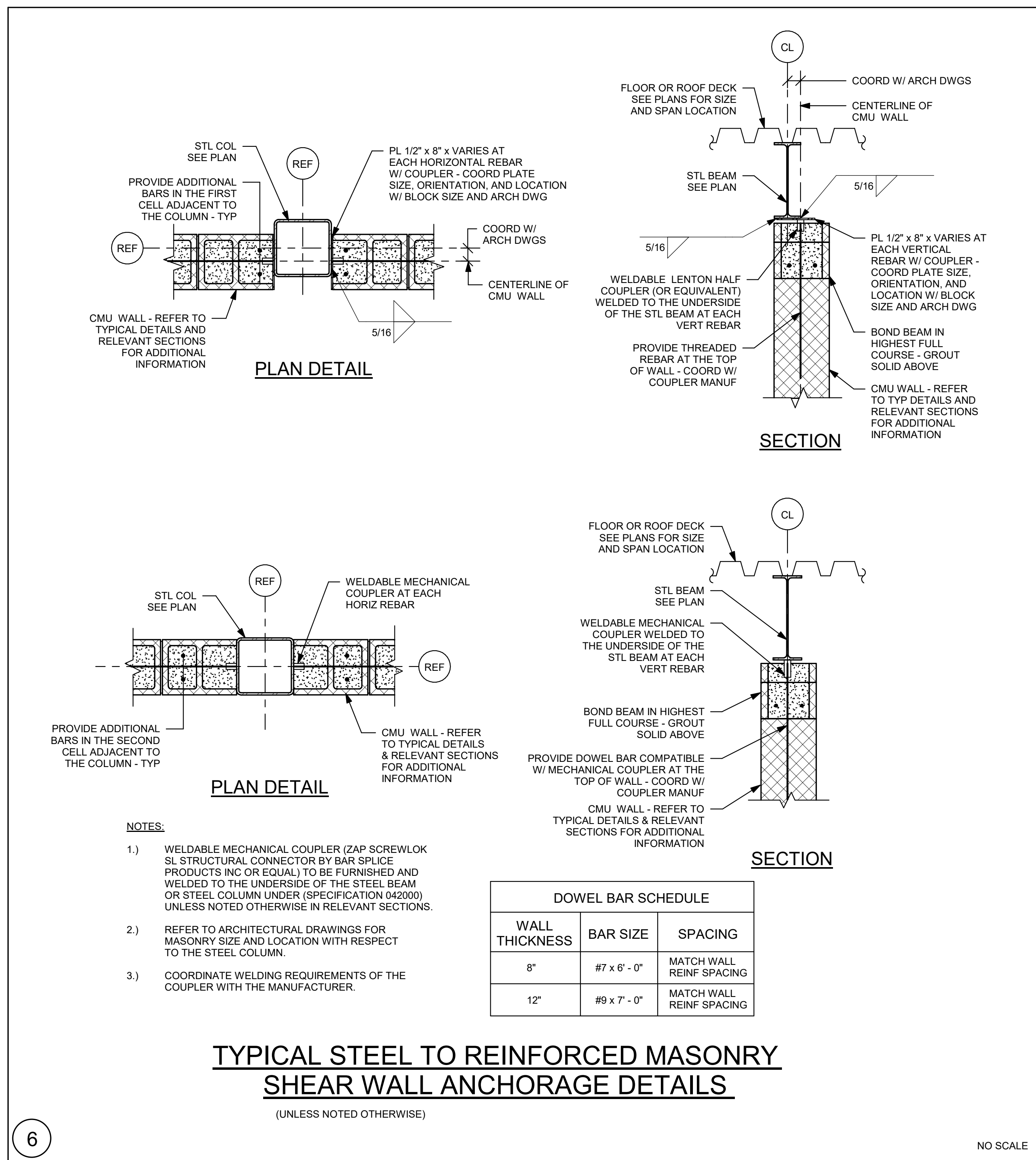
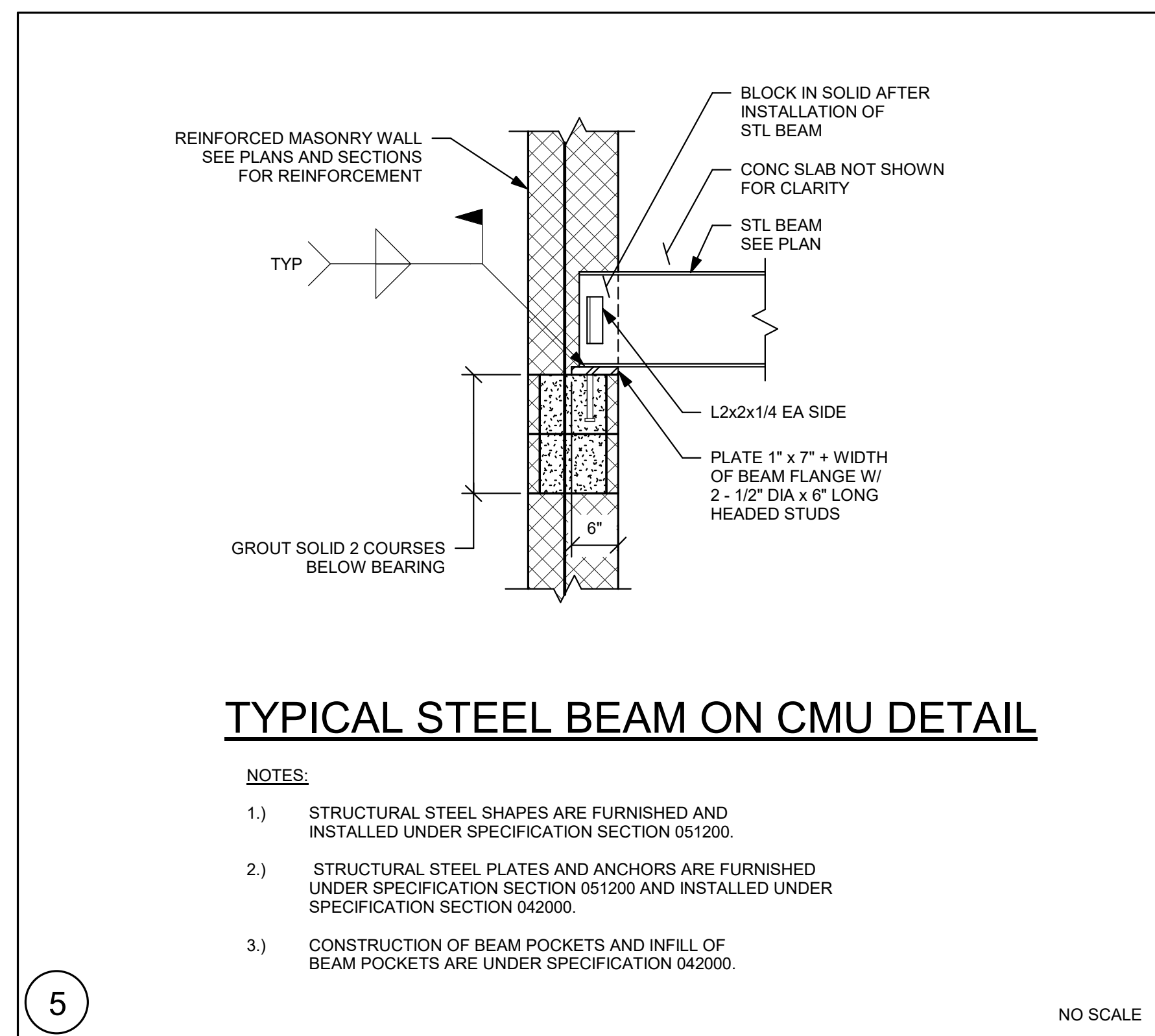
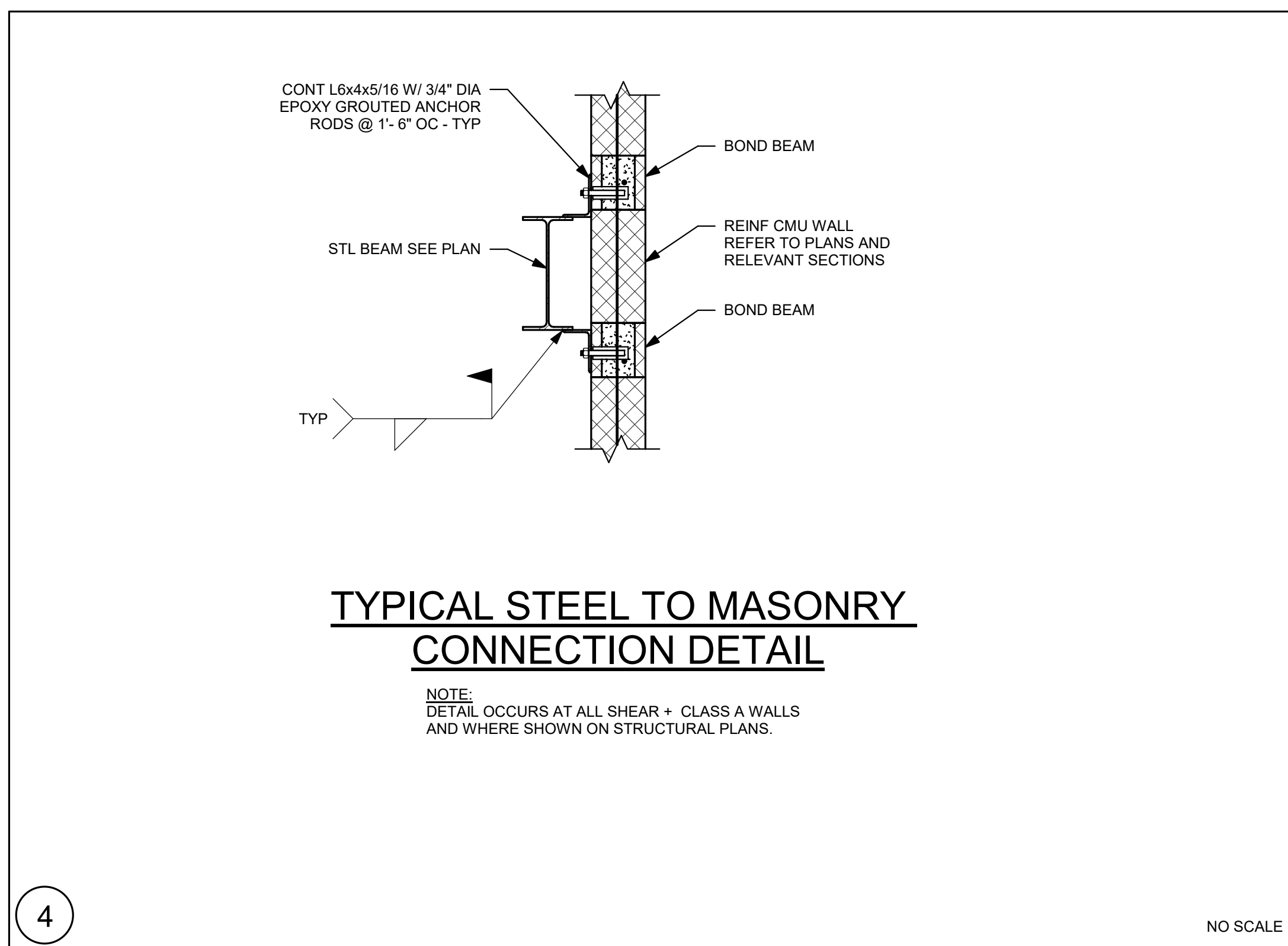
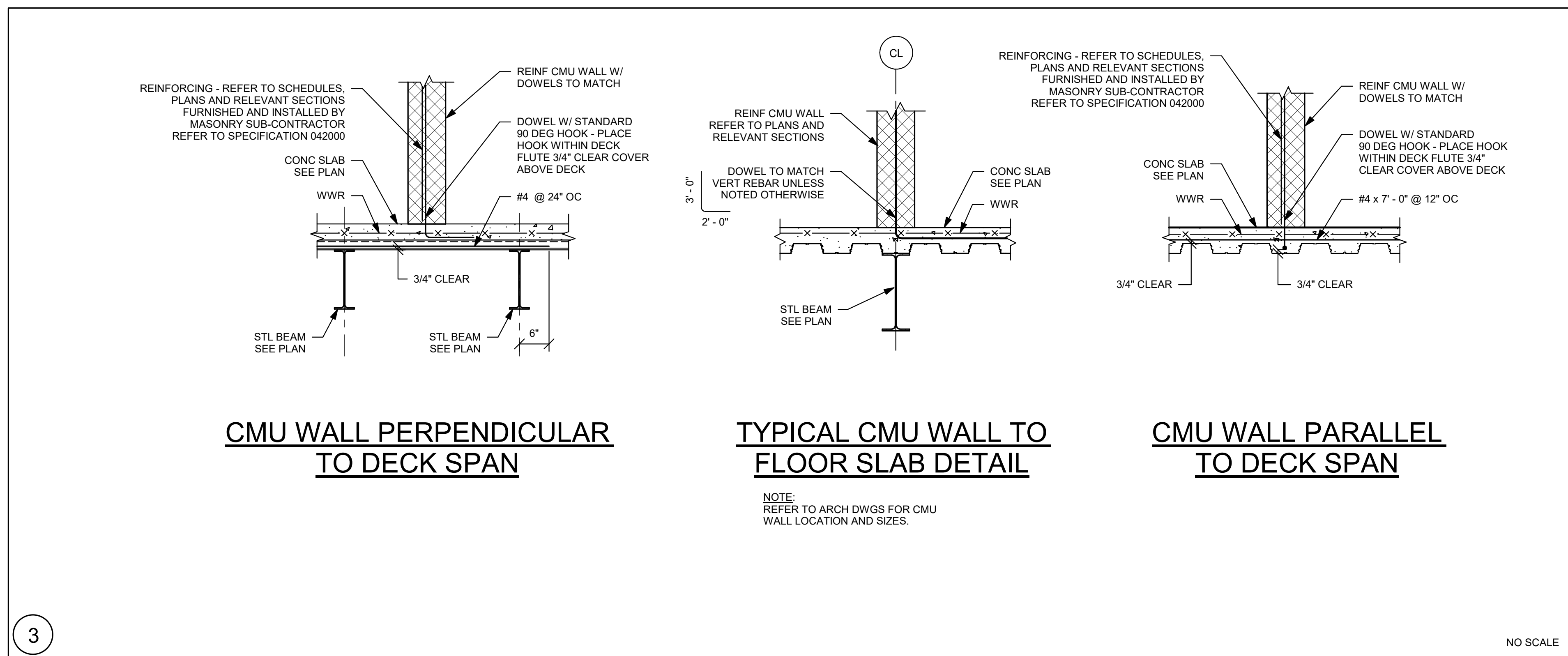
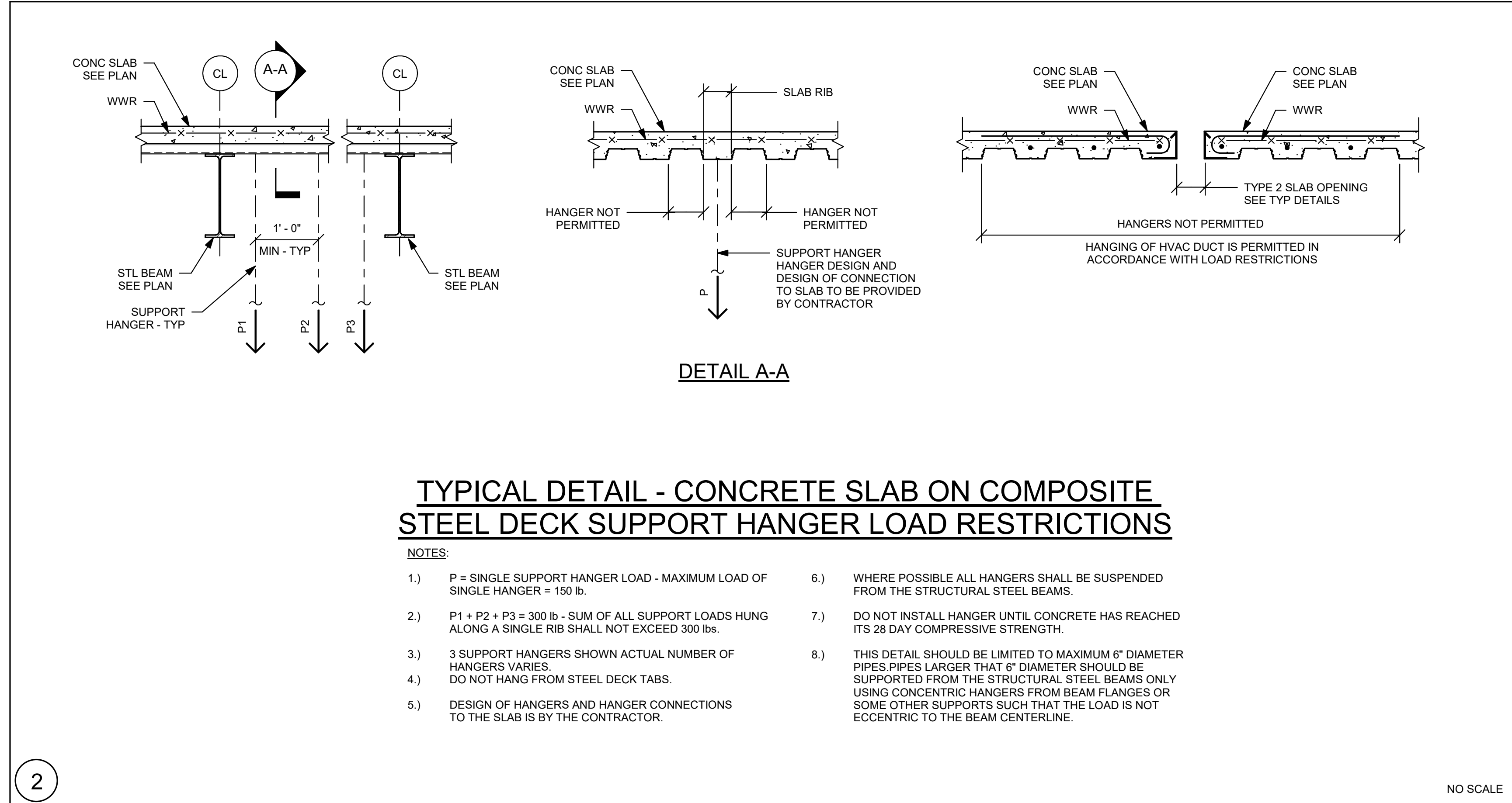
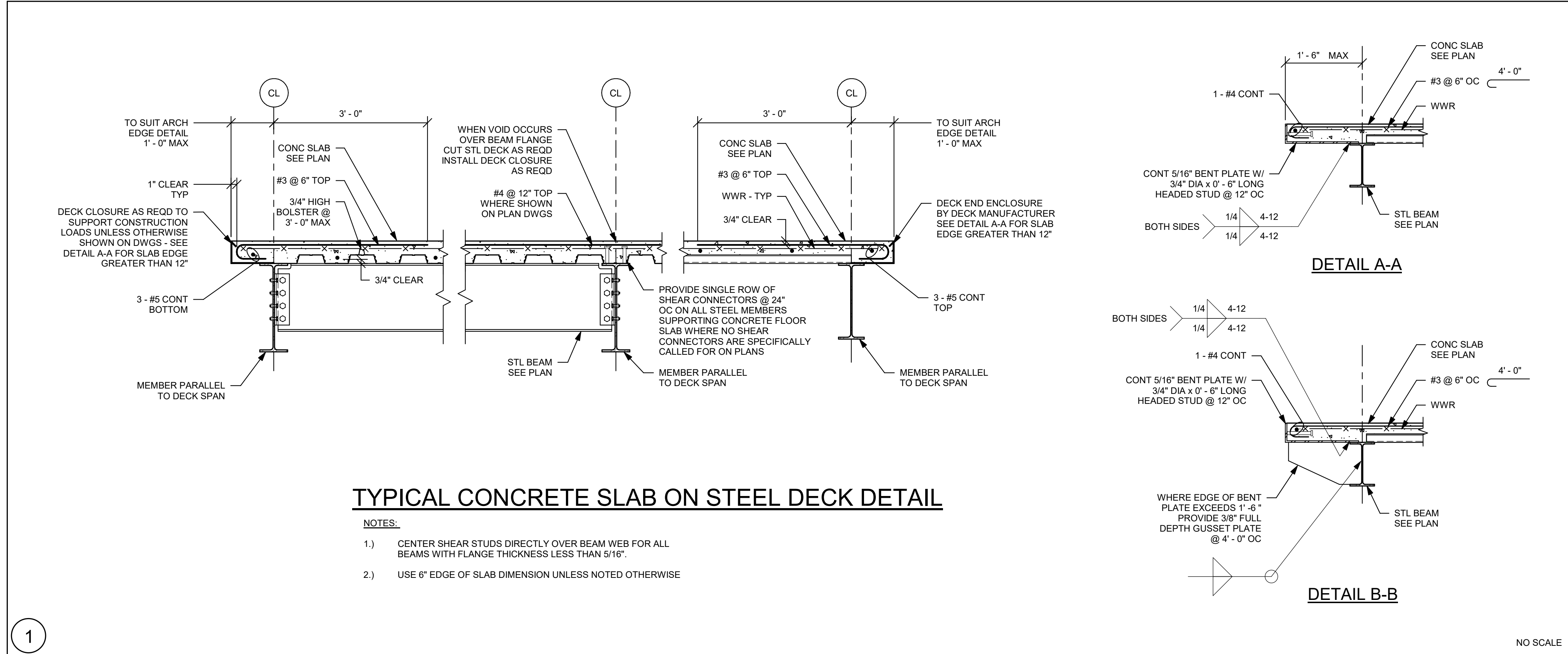
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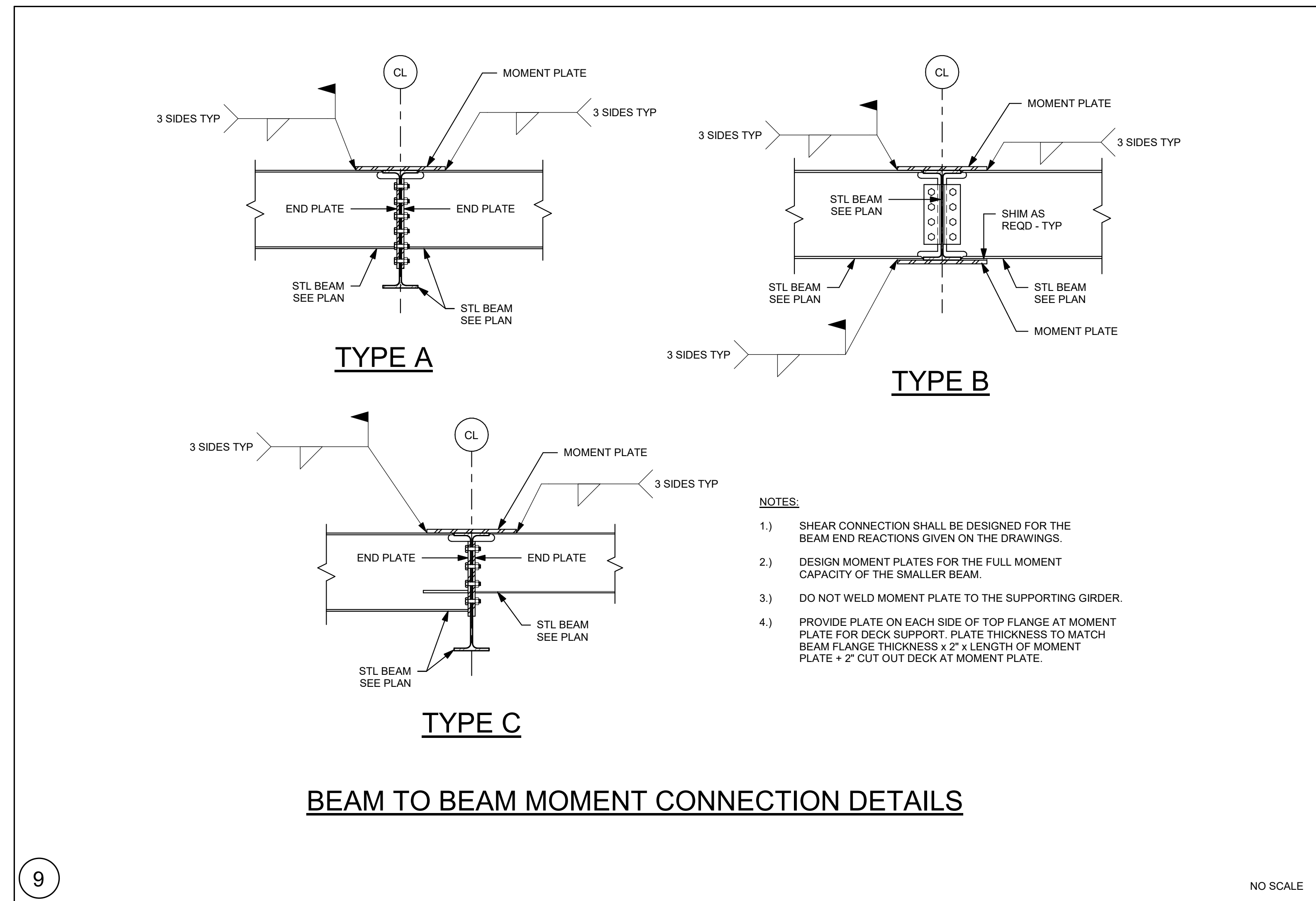
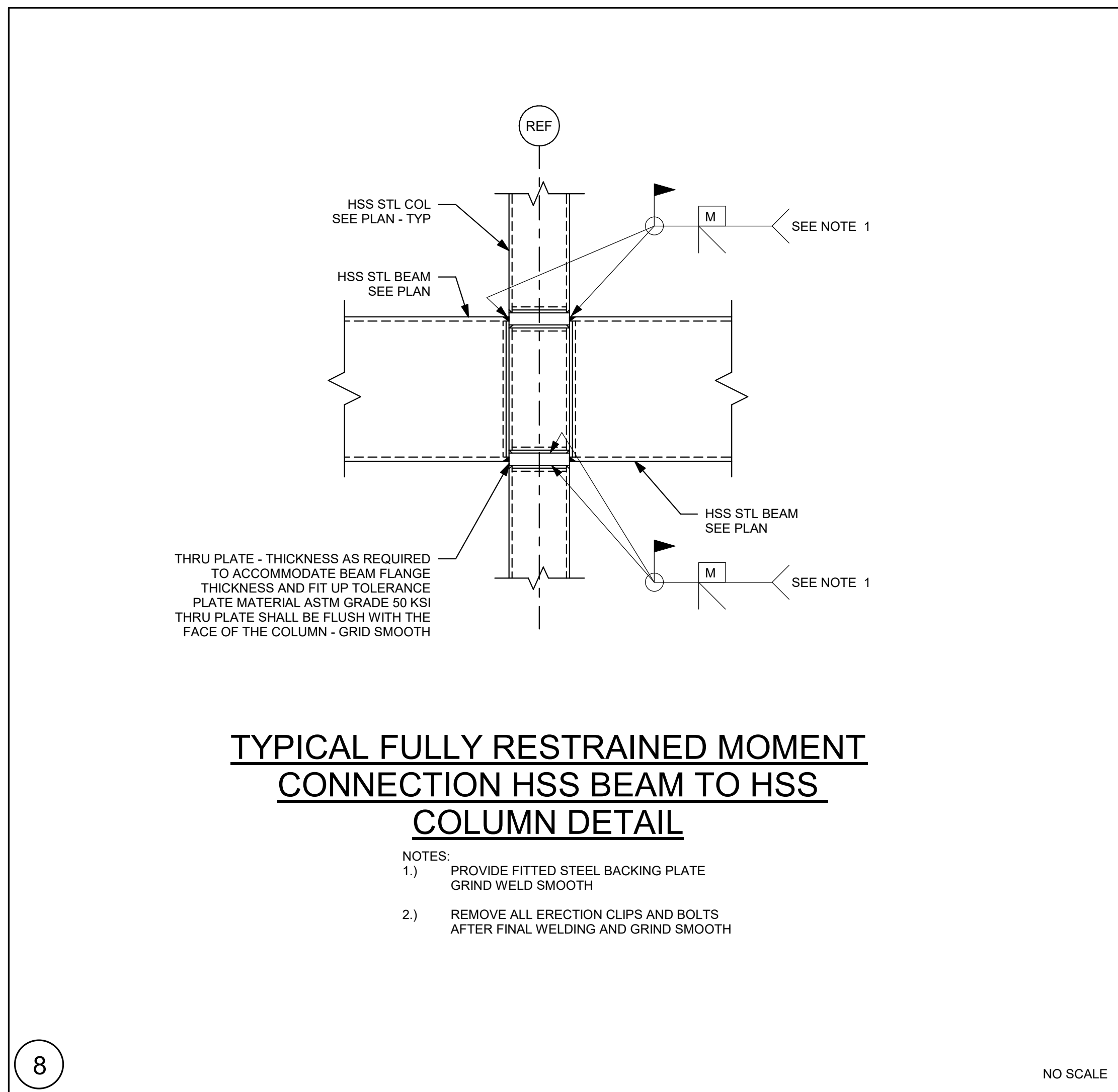
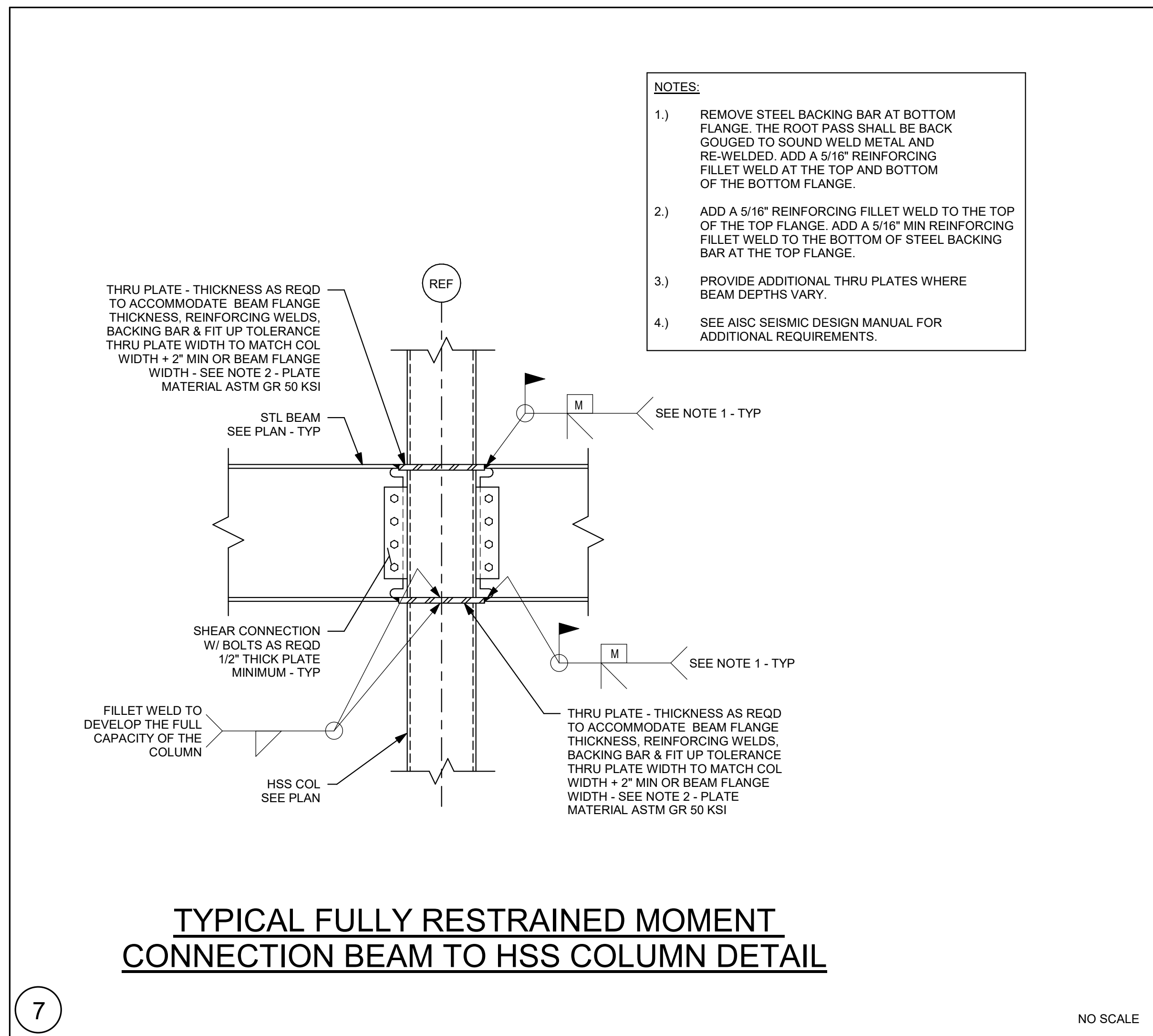
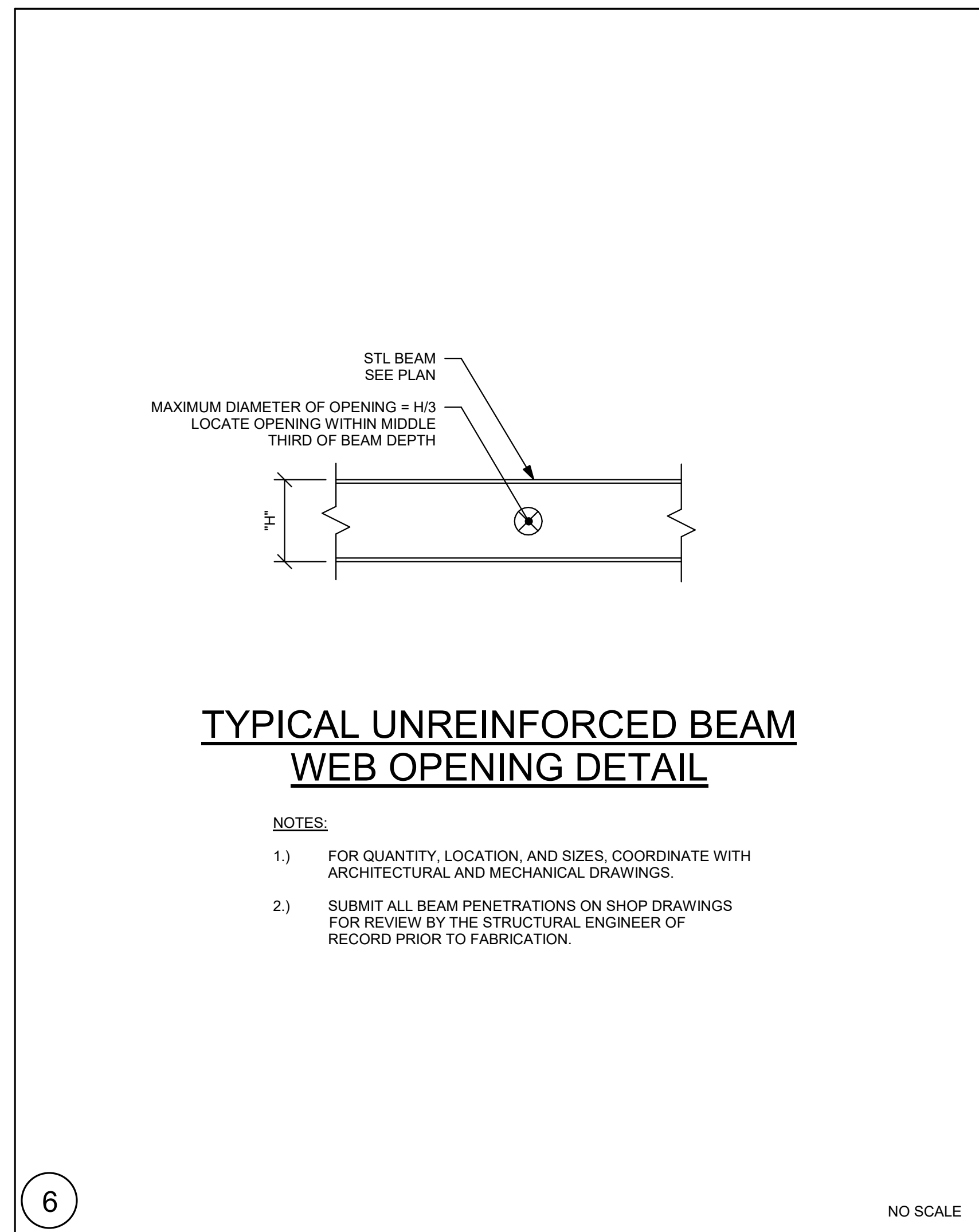
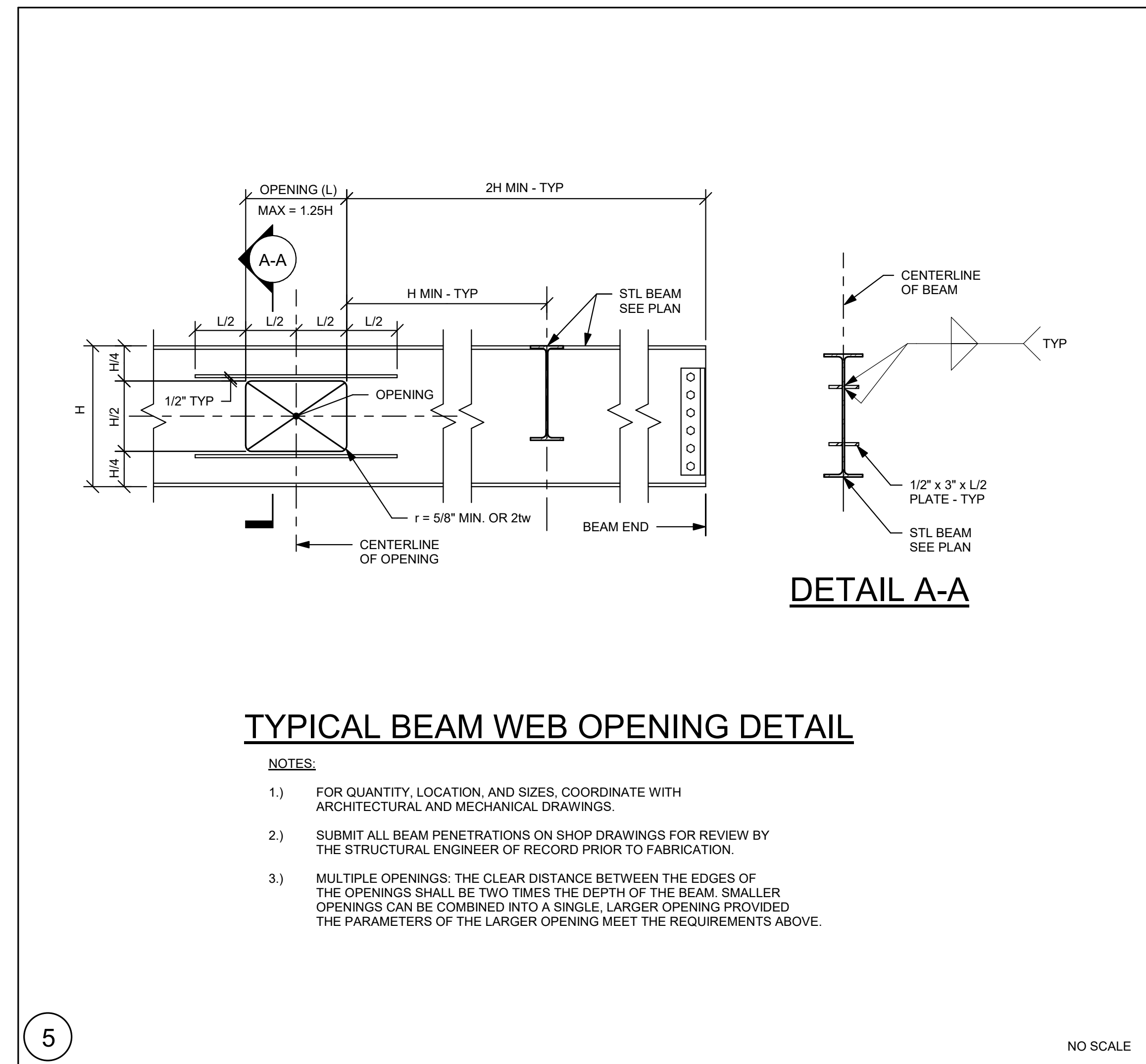
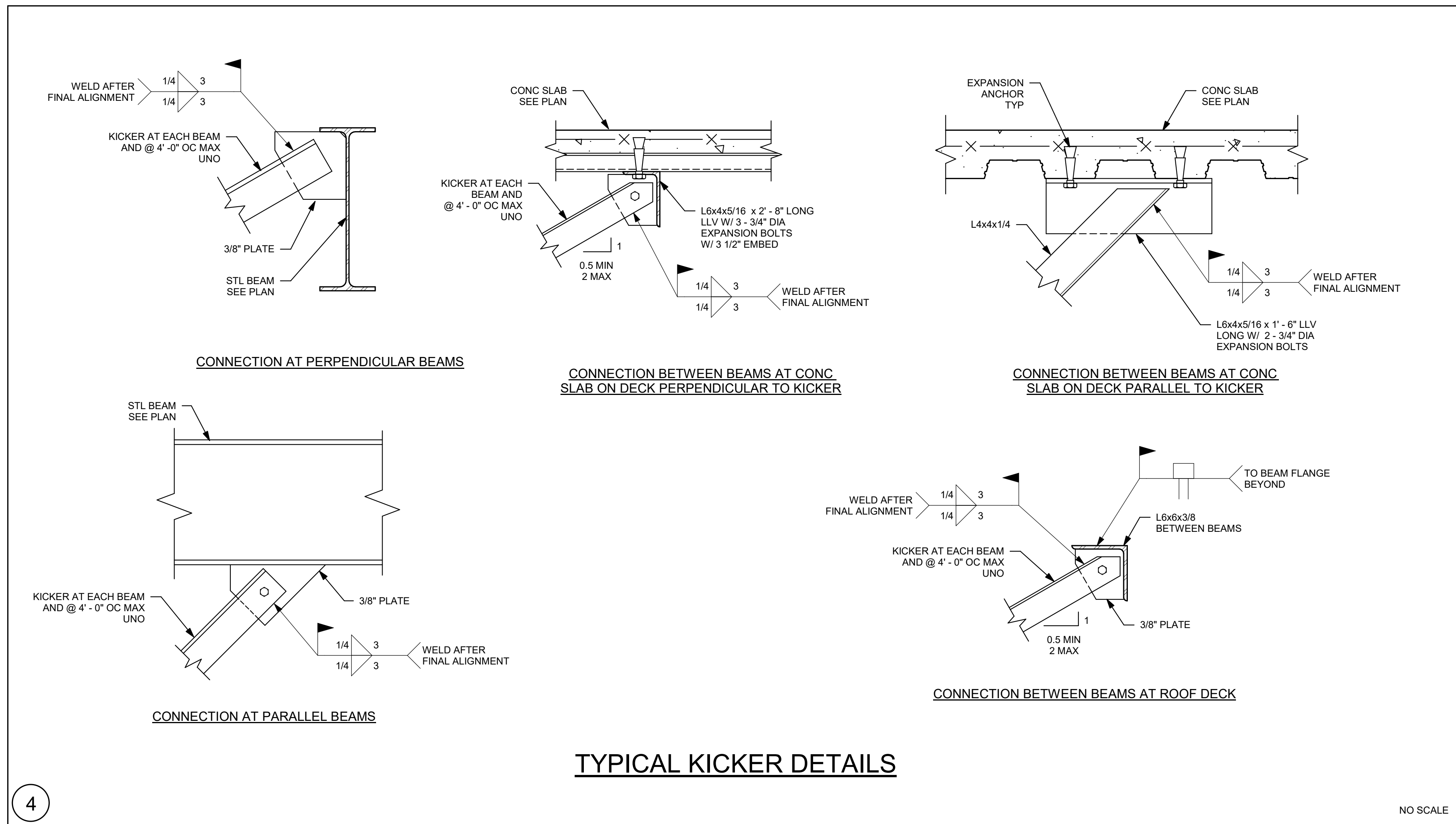
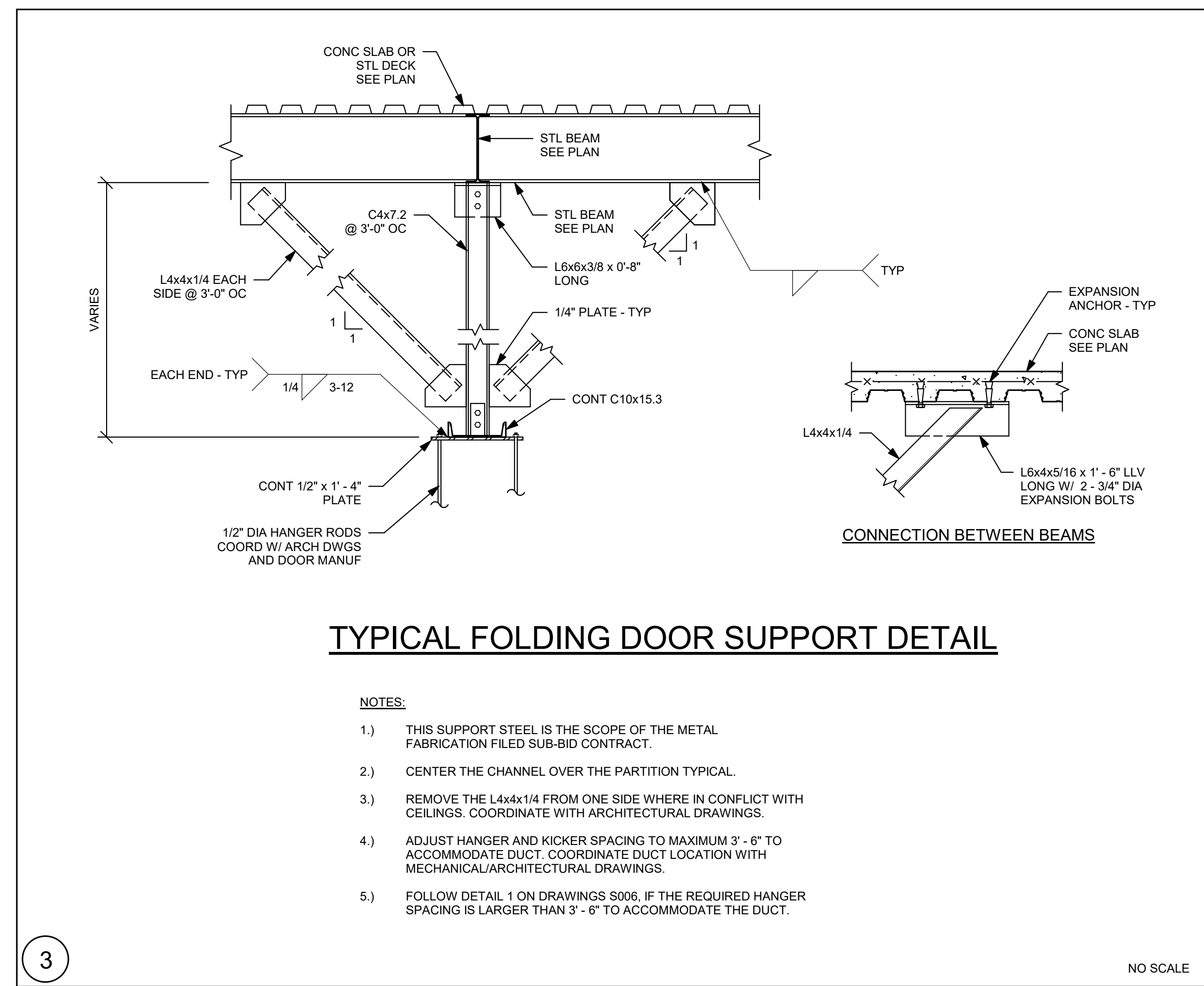
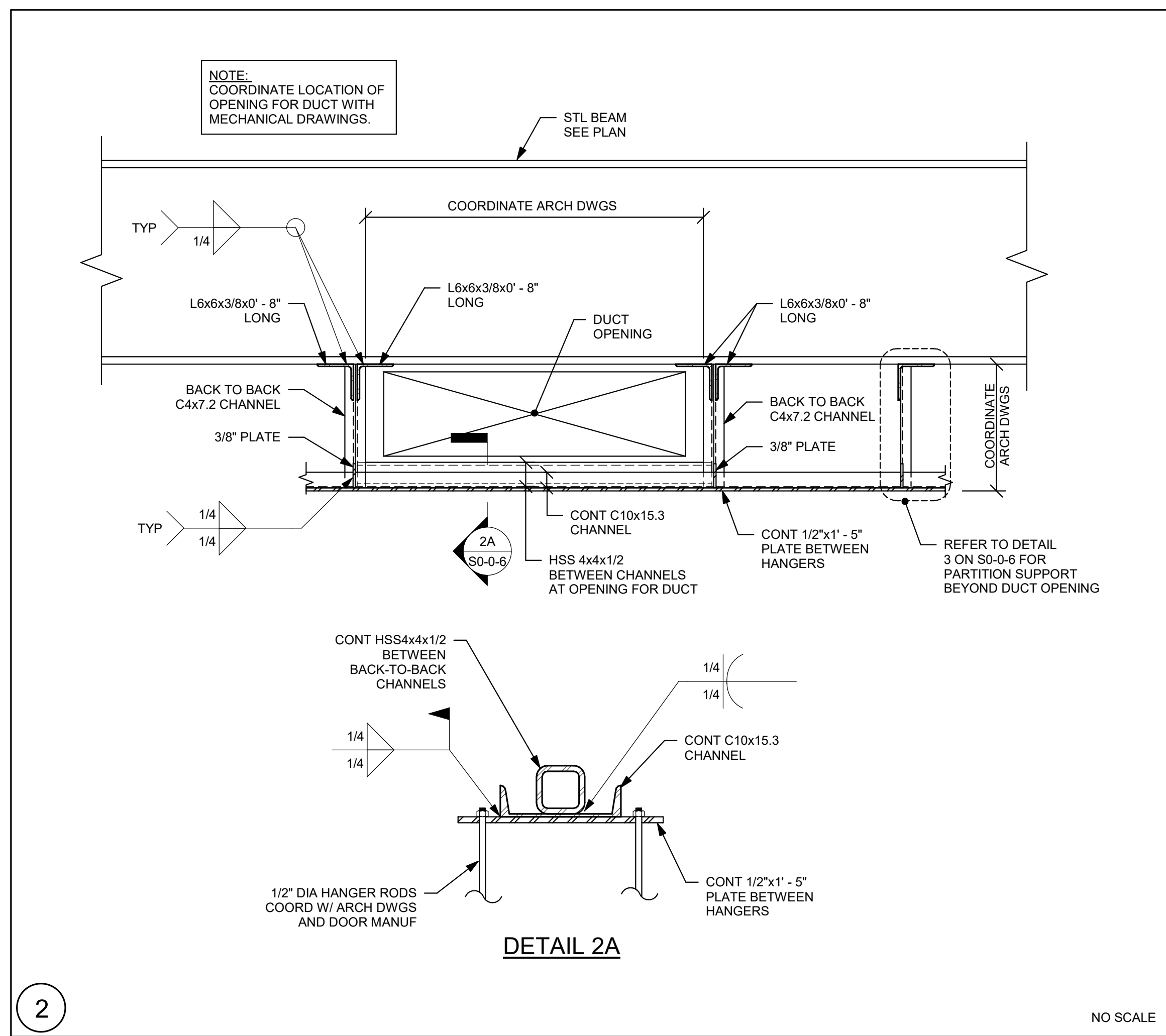
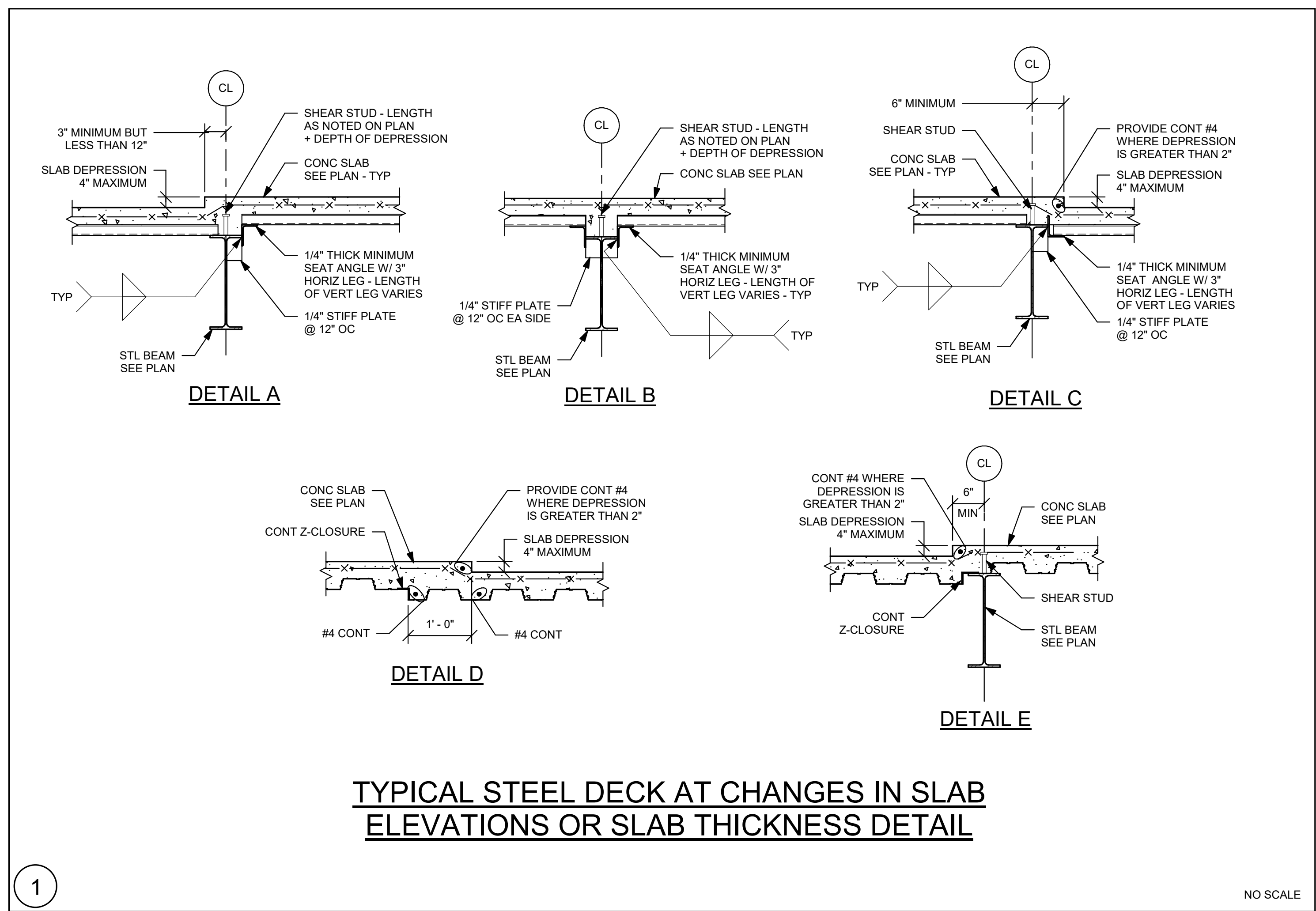
Job No.: 20202

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Date: August 28th, 2023

S0-0-4





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03/31/2023 EARLY STRUCTURAL BID PACKAGE

REVISION LIST

BID SET

August 28th, 2023

KEY PLAN

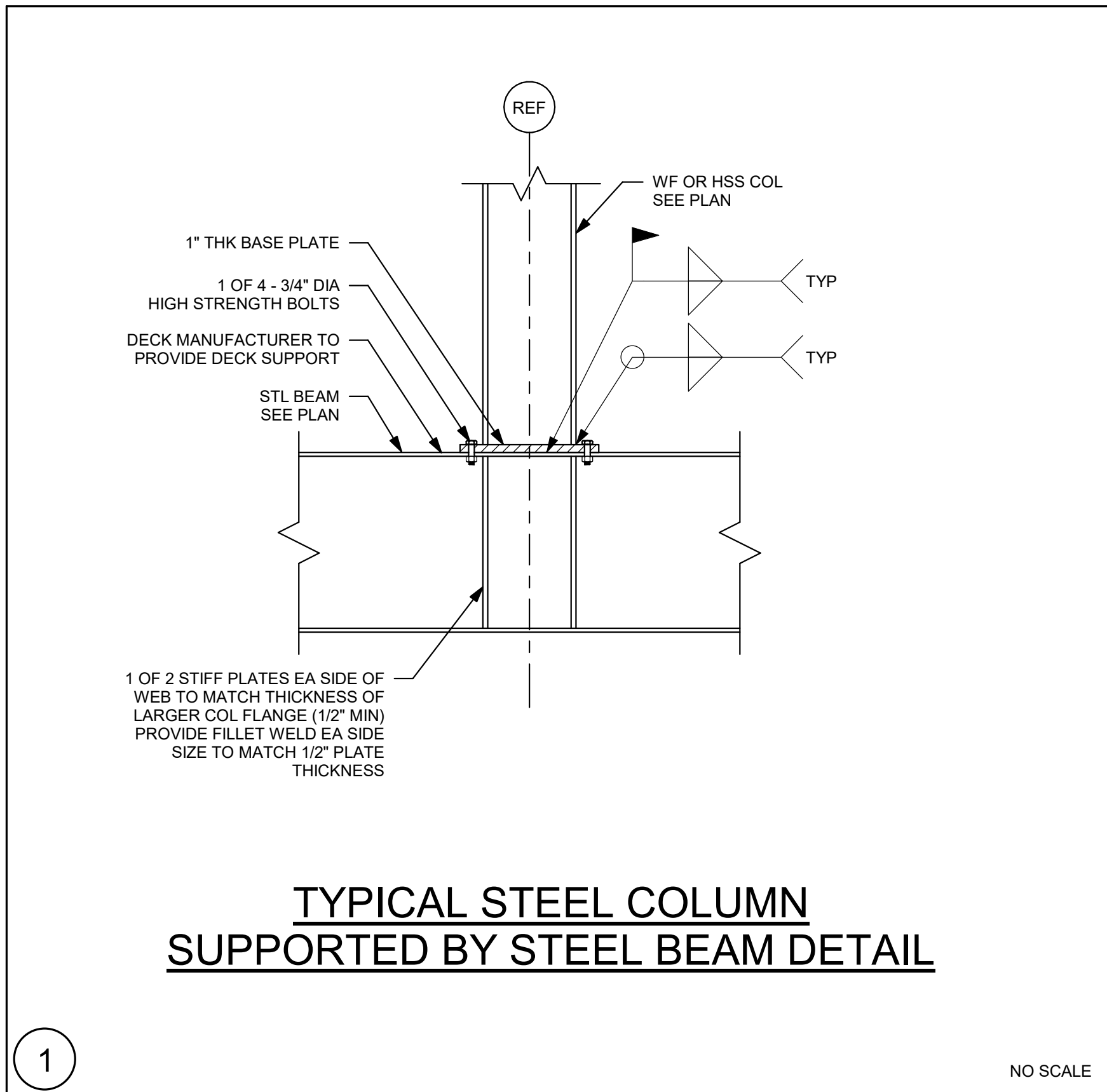
PROJECT NORTH MAGNETIC NORTH

TYPICAL DETAILS

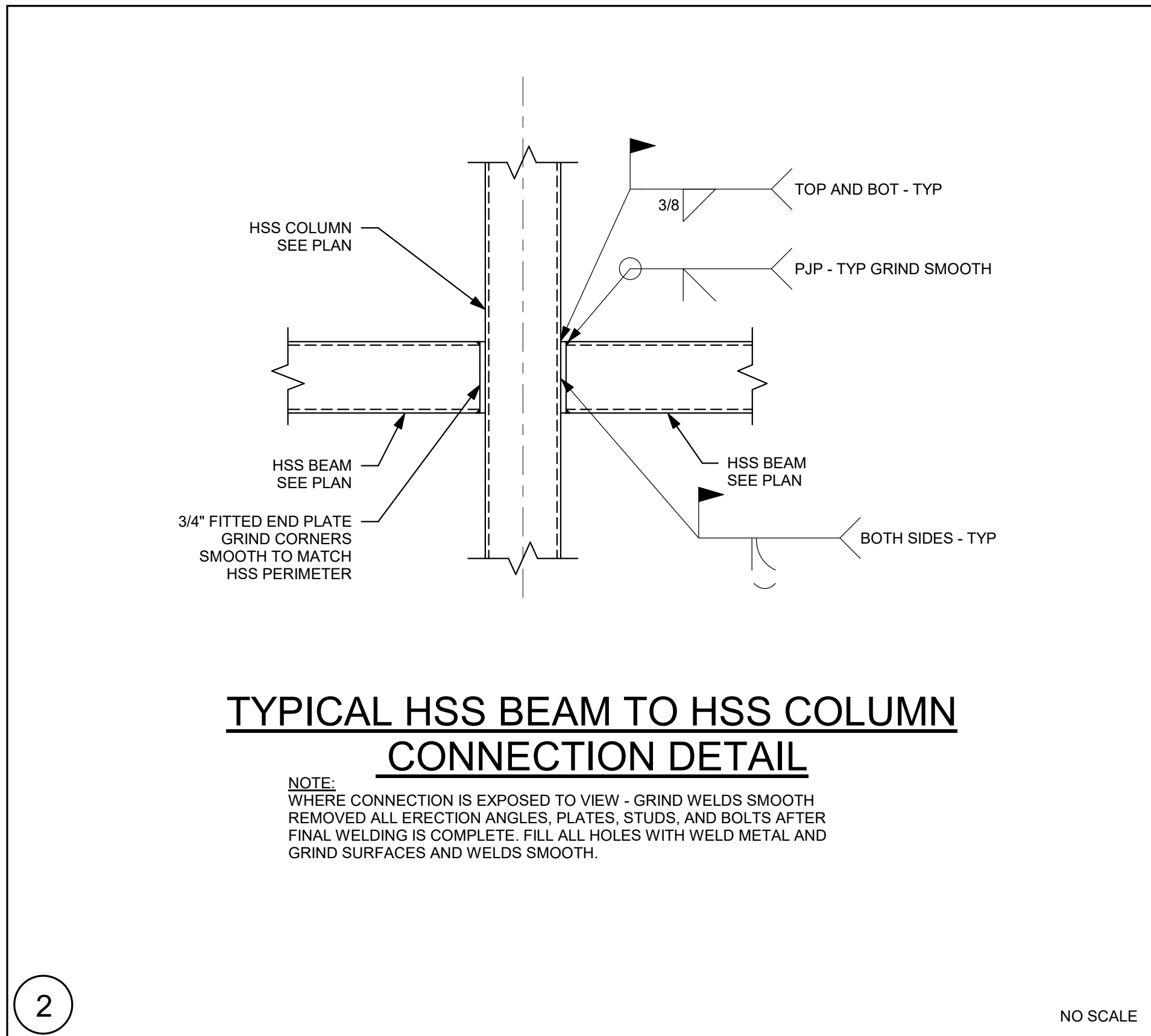
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S0-0-6

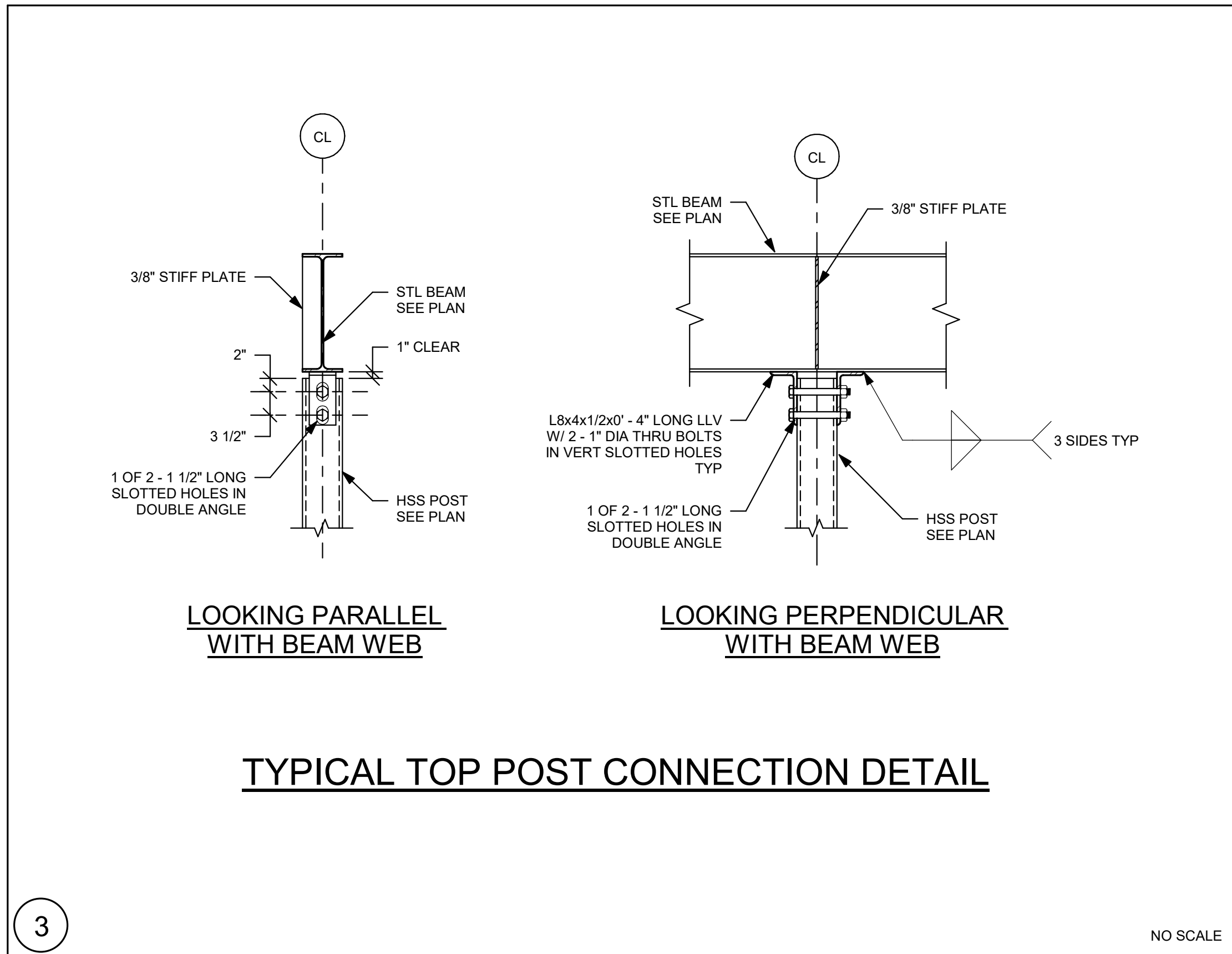
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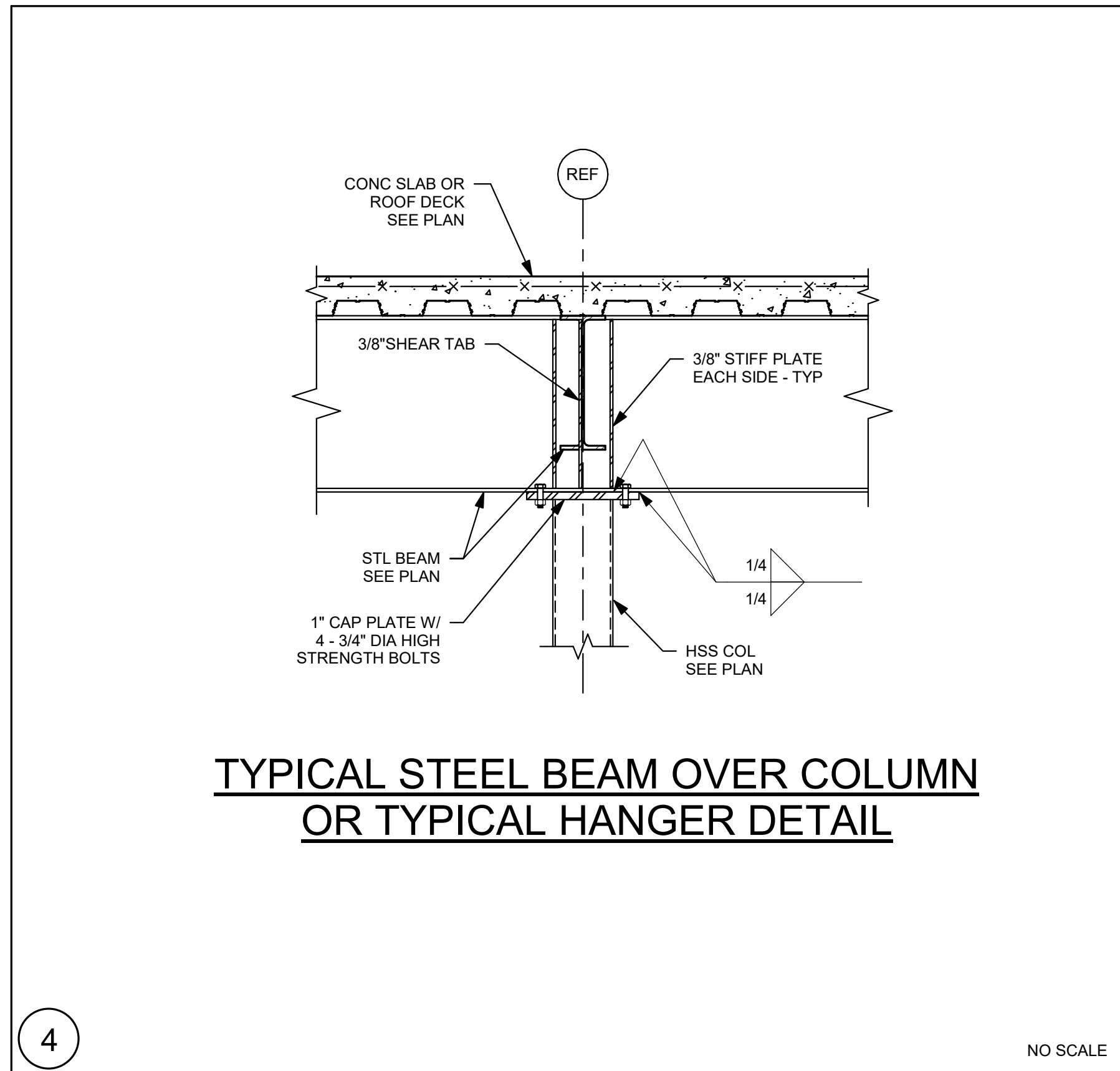
**TYPICAL STEEL COLUMN
SUPPORTED BY STEEL BEAM DETAIL**



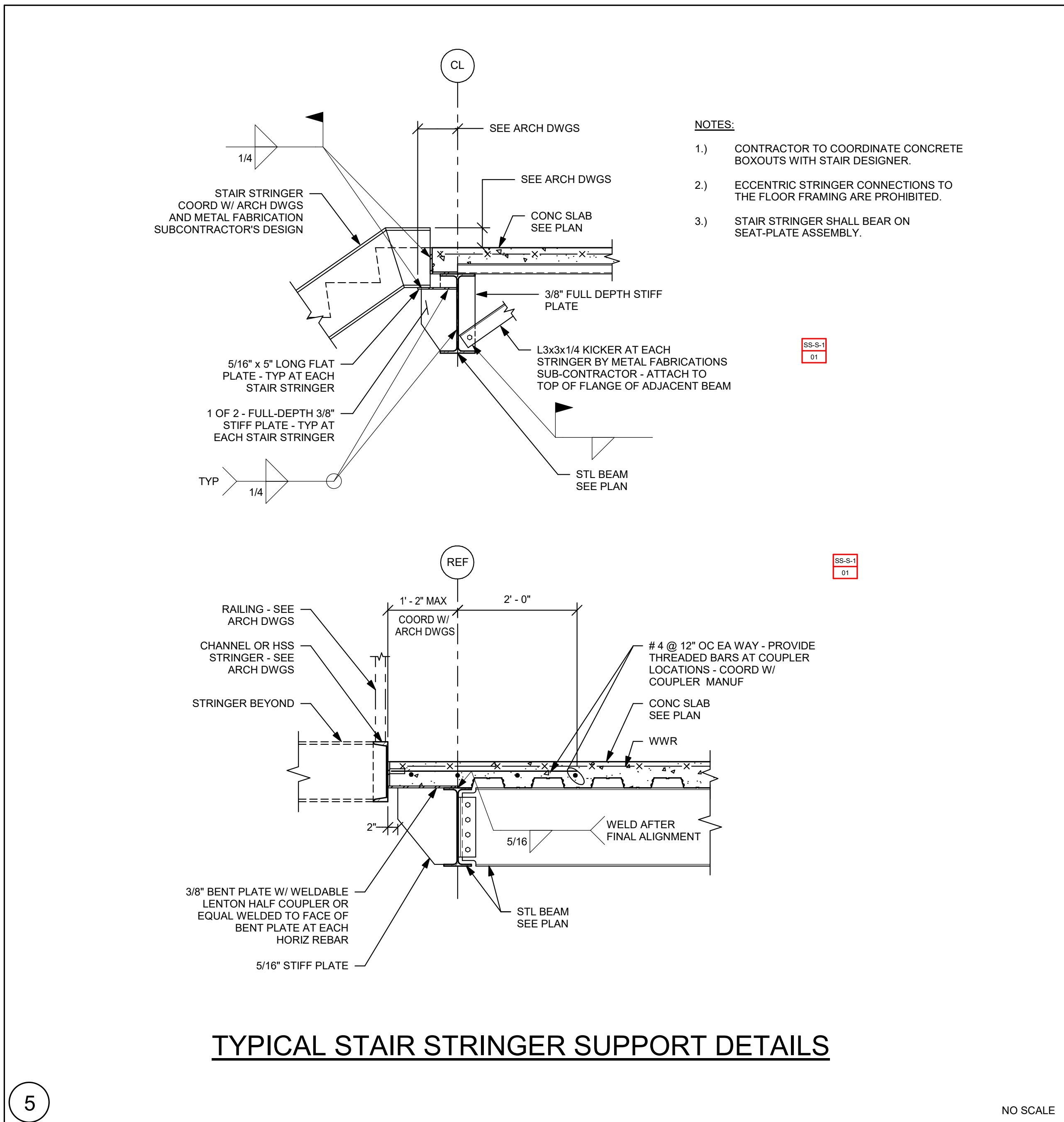
**TYPICAL HSS BEAM TO HSS COLUMN
CONNECTION DETAIL**



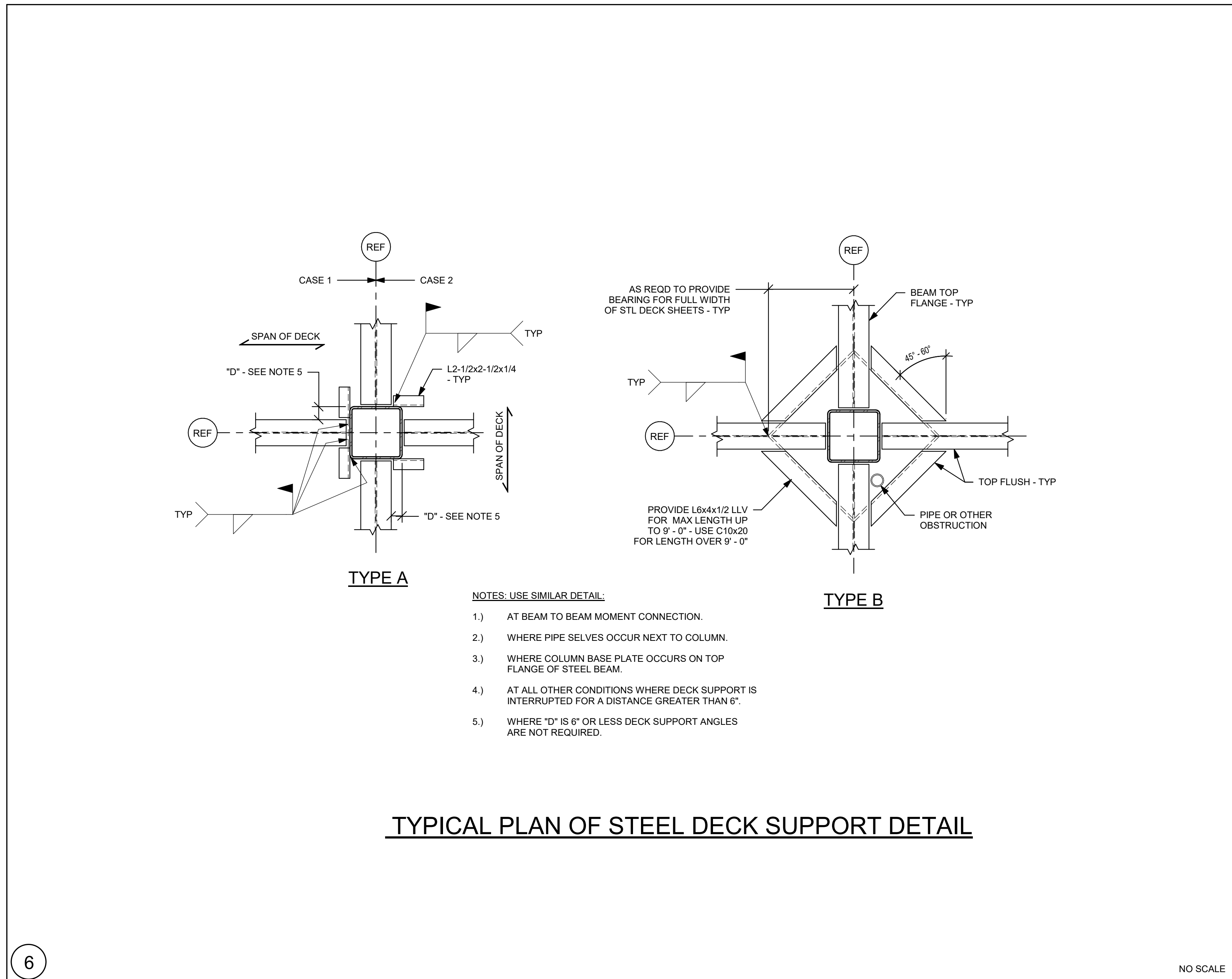
TYPICAL TOP POST CONNECTION DETAIL



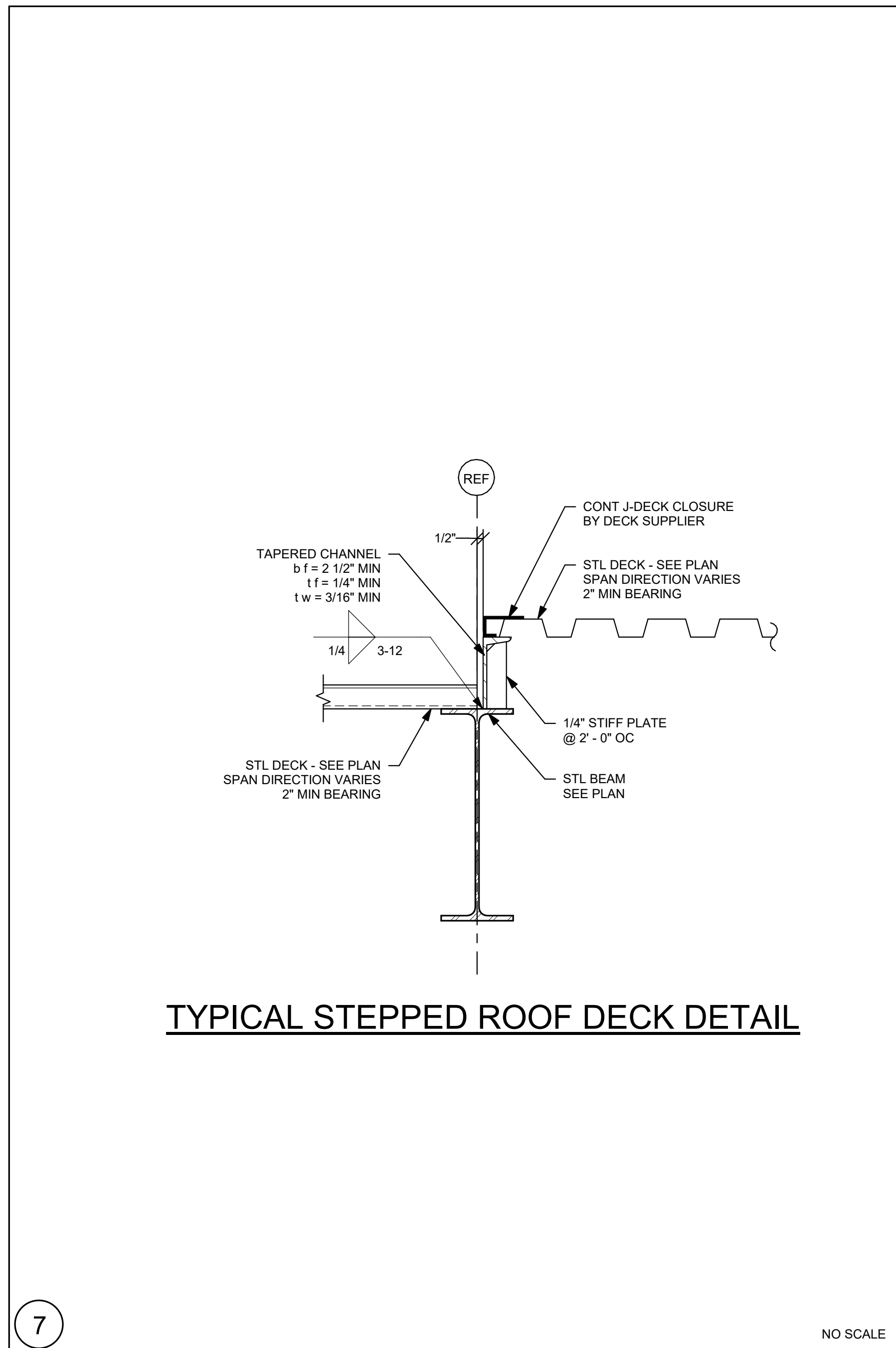
**TYPICAL STEEL BEAM OVER COLUMN
OR TYPICAL HANGER DETAIL**



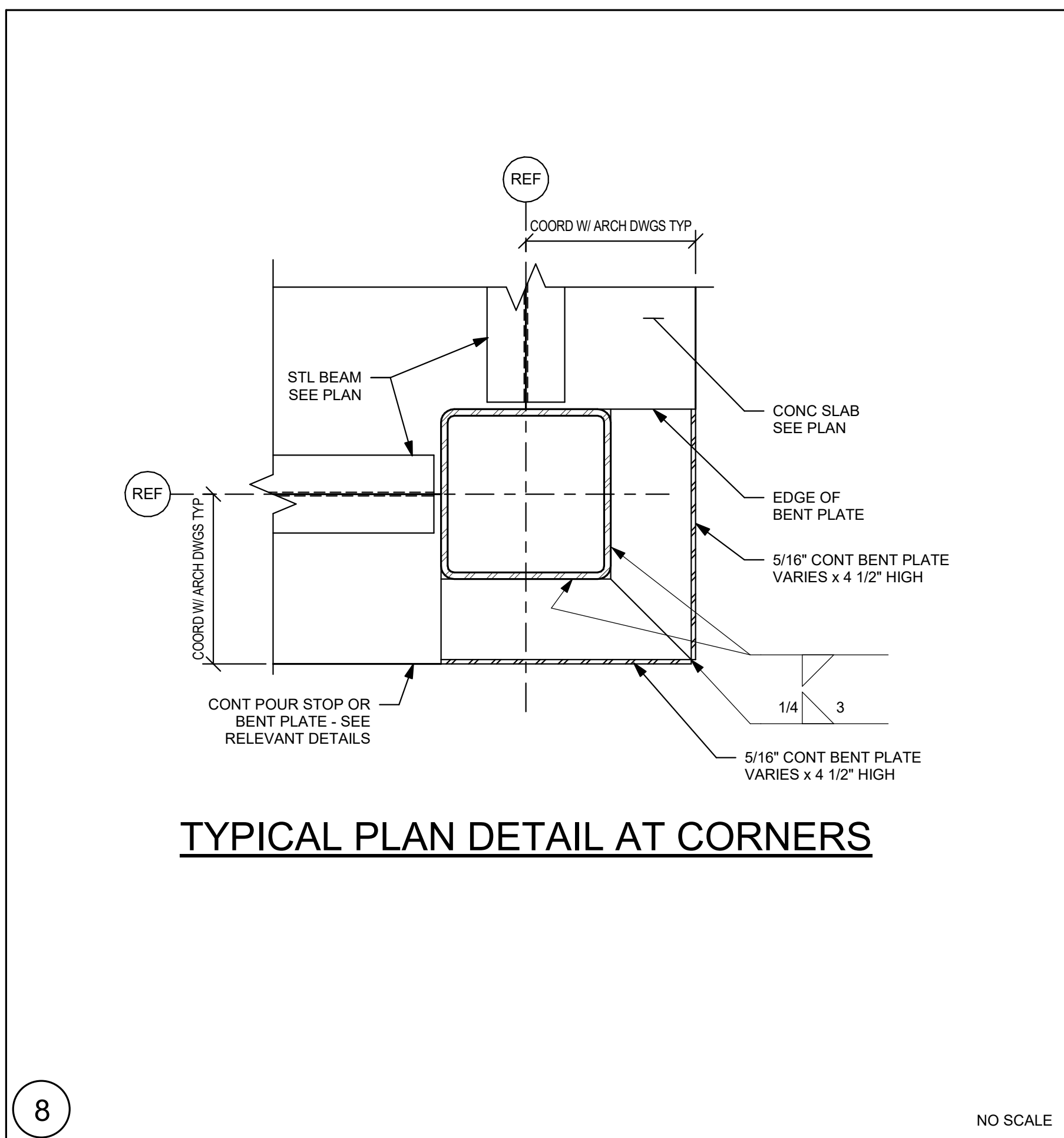
TYPICAL STAIR STRINGER SUPPORT DETAILS



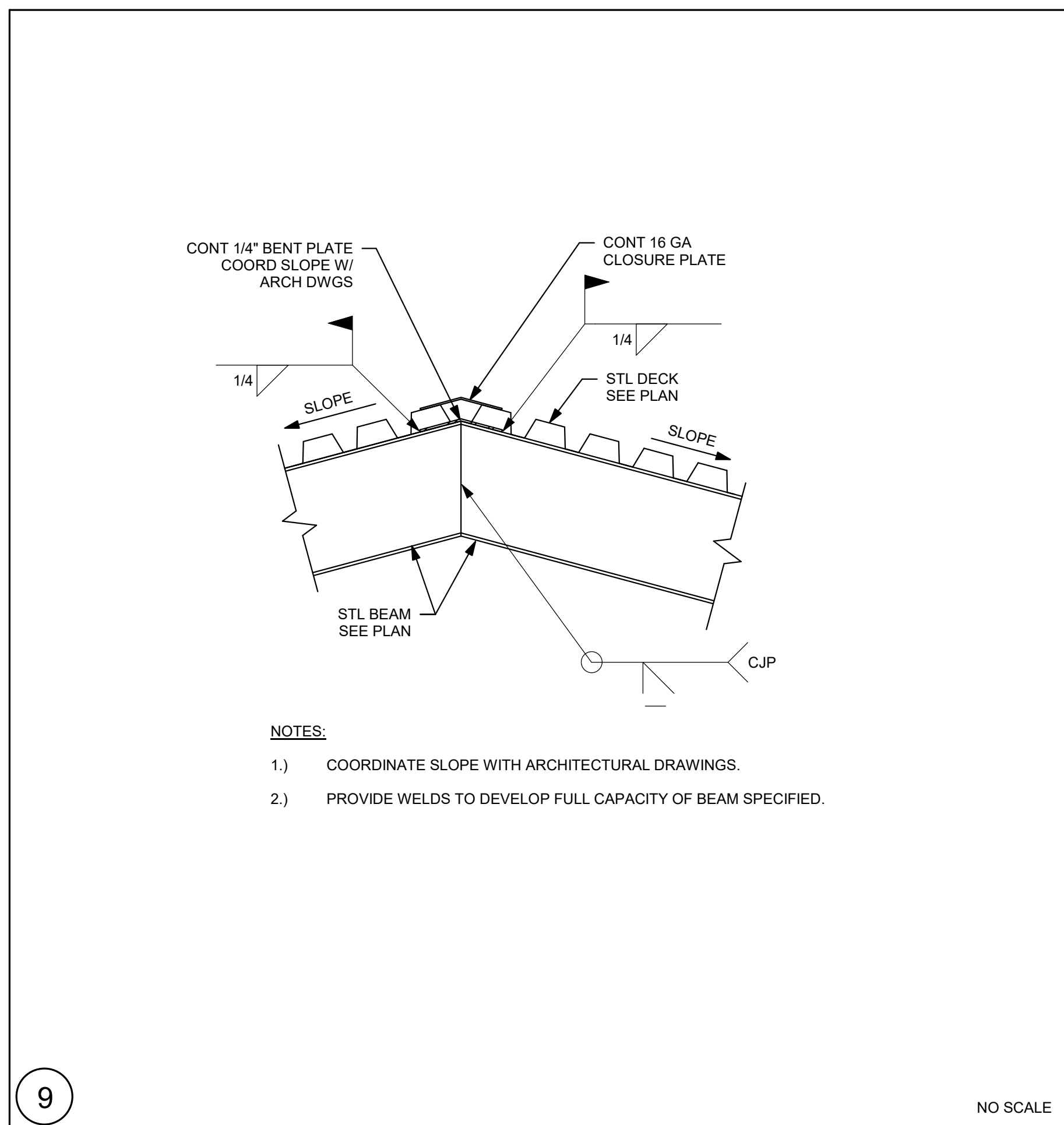
TYPICAL PLAN OF STEEL DECK SUPPORT DETAIL



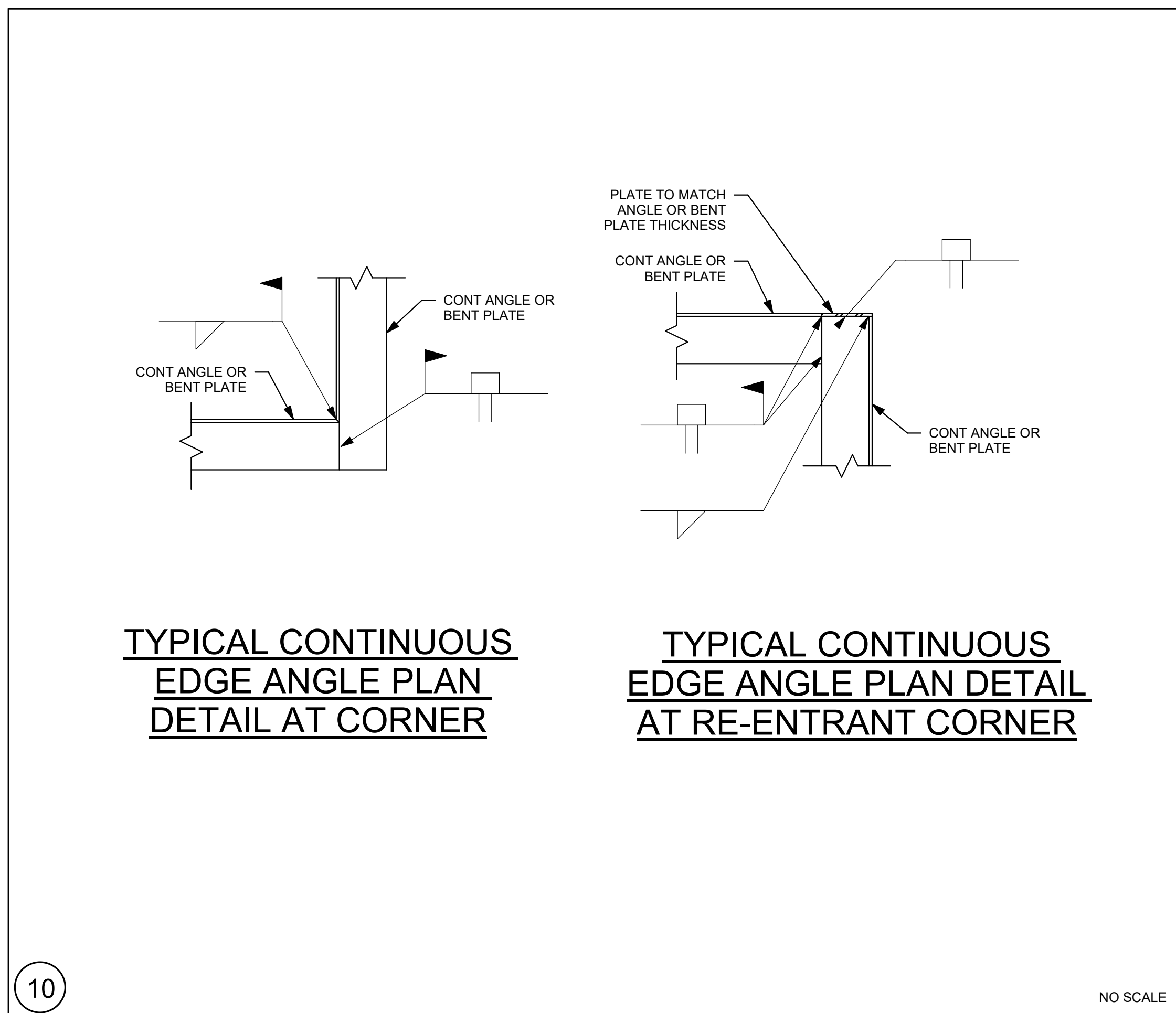
TYPICAL STEPPED ROOF DECK DETAIL



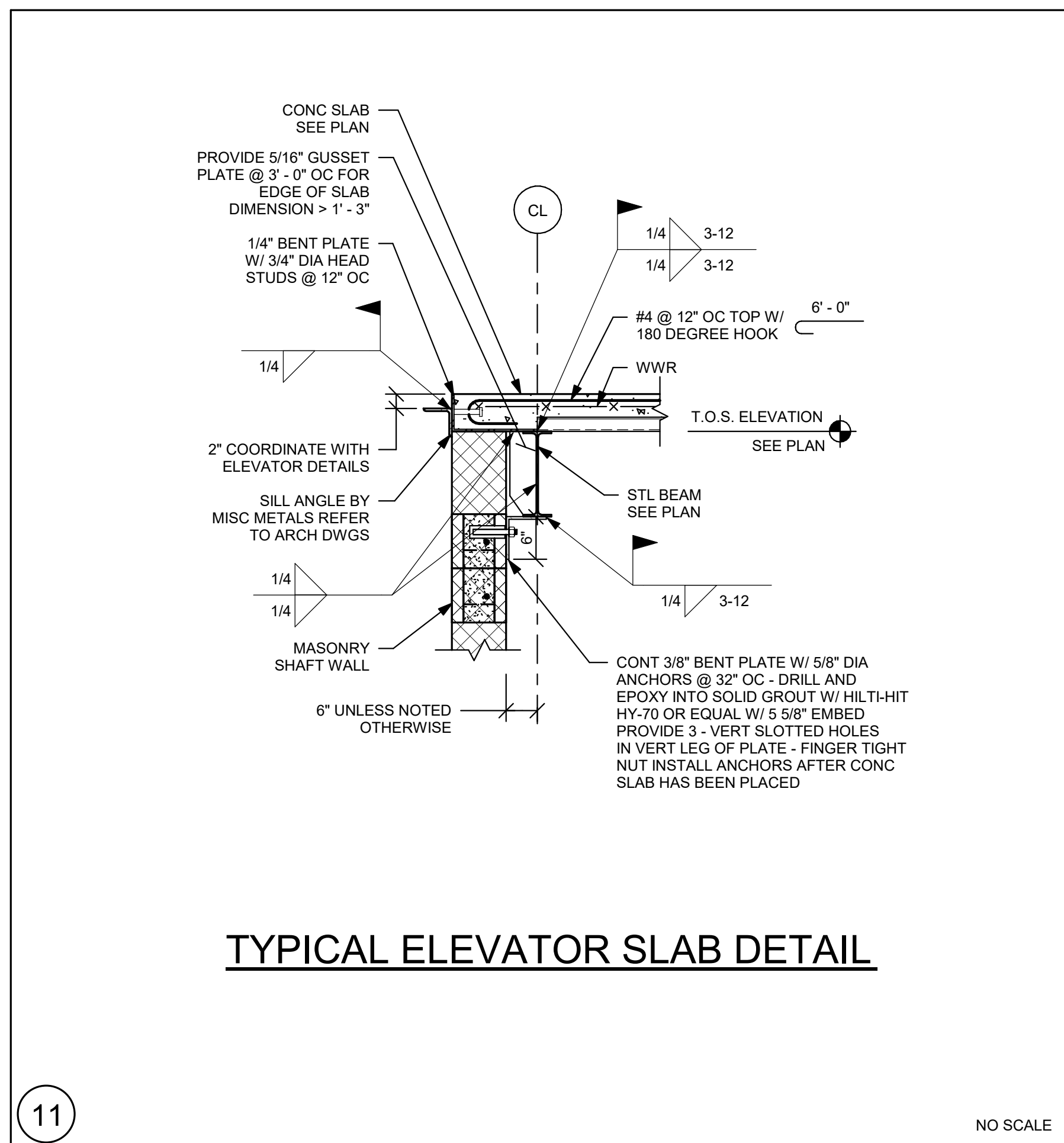
TYPICAL PLAN DETAIL AT CORNERS



**TYPICAL CONTINUOUS
EDGE ANGLE PLAN
DETAIL AT CORNER**



**TYPICAL CONTINUOUS
EDGE ANGLE PLAN DETAIL
AT RE-ENTRANT CORNER**



TYPICAL ELEVATOR SLAB DETAIL

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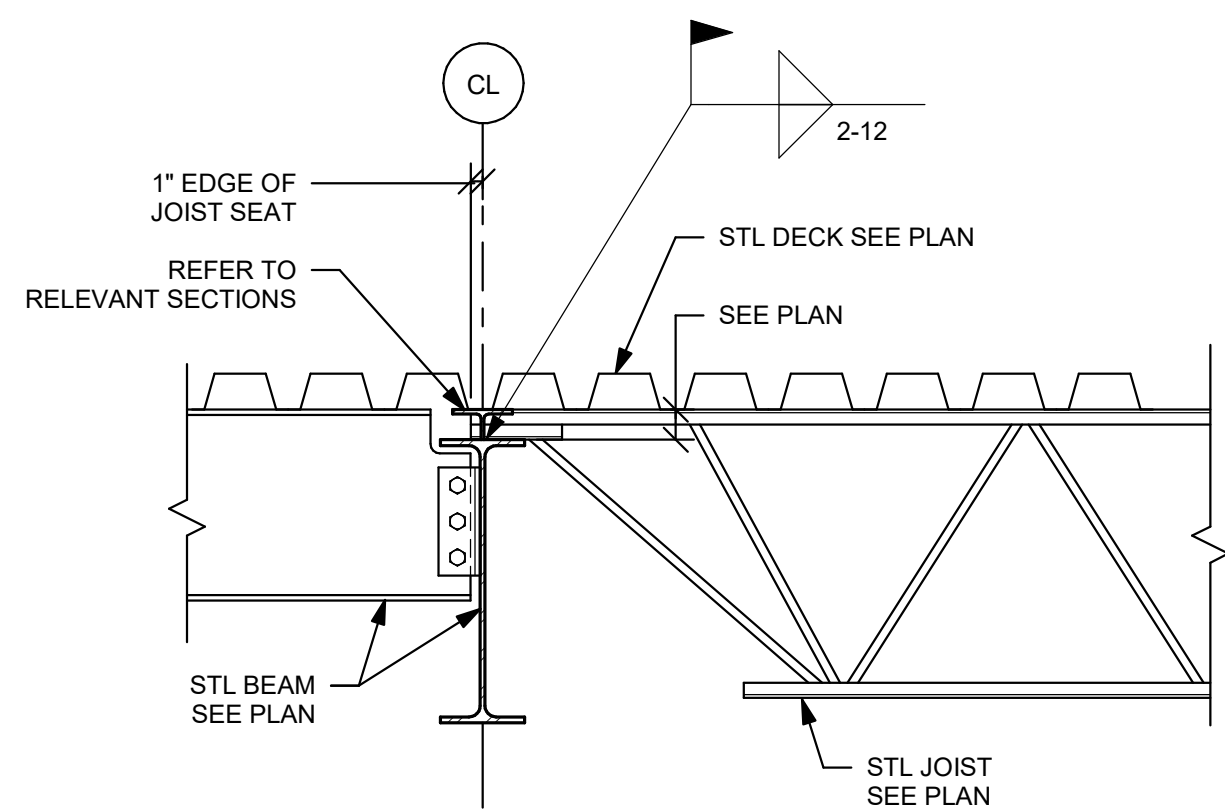
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MAGNETIC NORTH

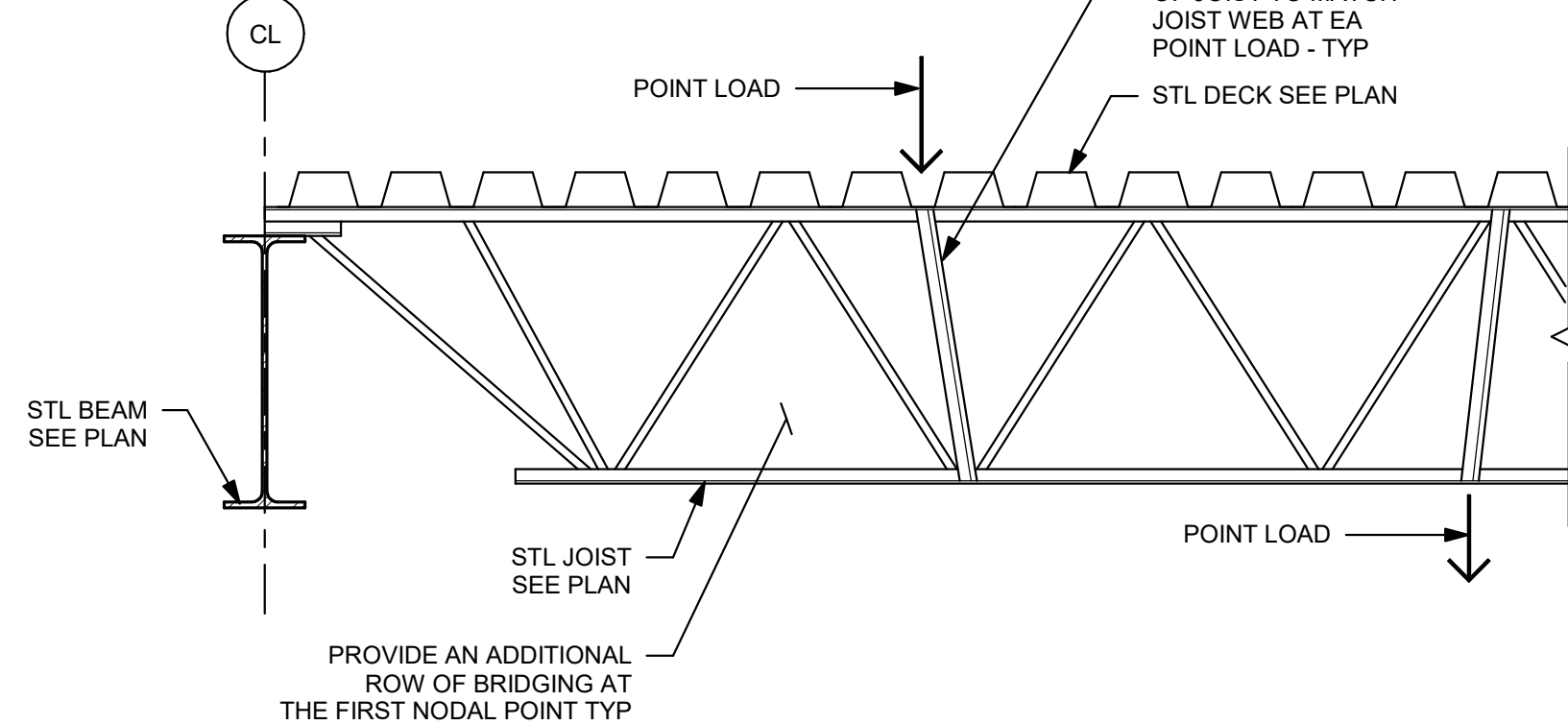
**TYPICAL
DETAILS**

Scale: As indicated
Job No.: 20202
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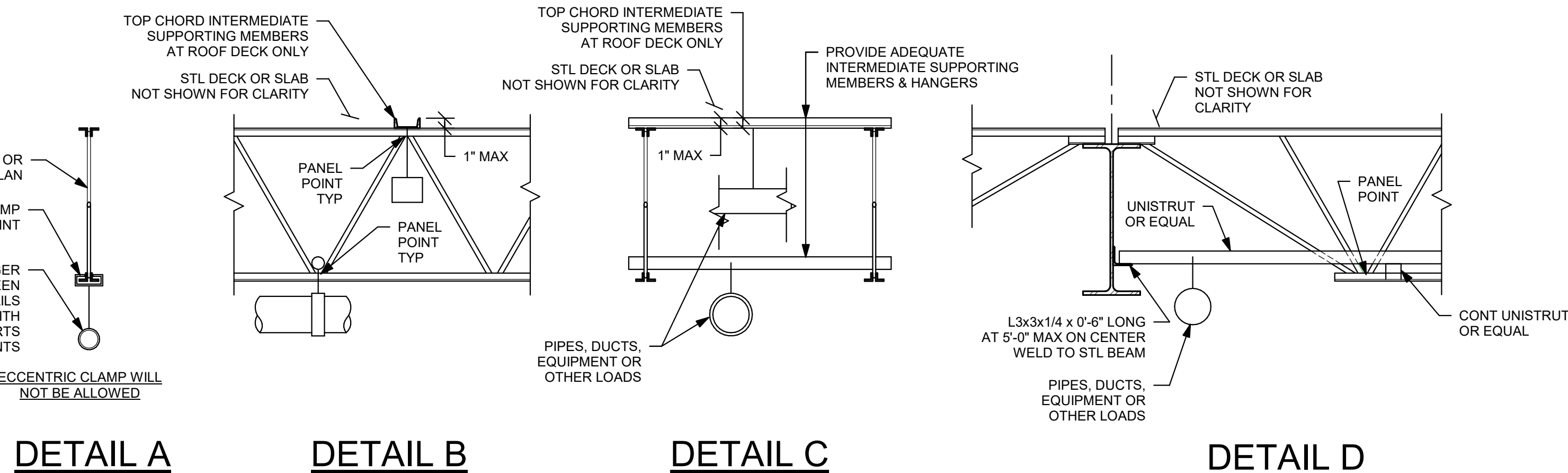
S0-0-7



**TYPICAL BEAM / JOIST
SUPPORT DETAIL AT ROOF**



**TYPICAL JOIST BOTTOM AND TOP
CHORD REINFORCING DETAIL**



DETAIL A

DETAIL B

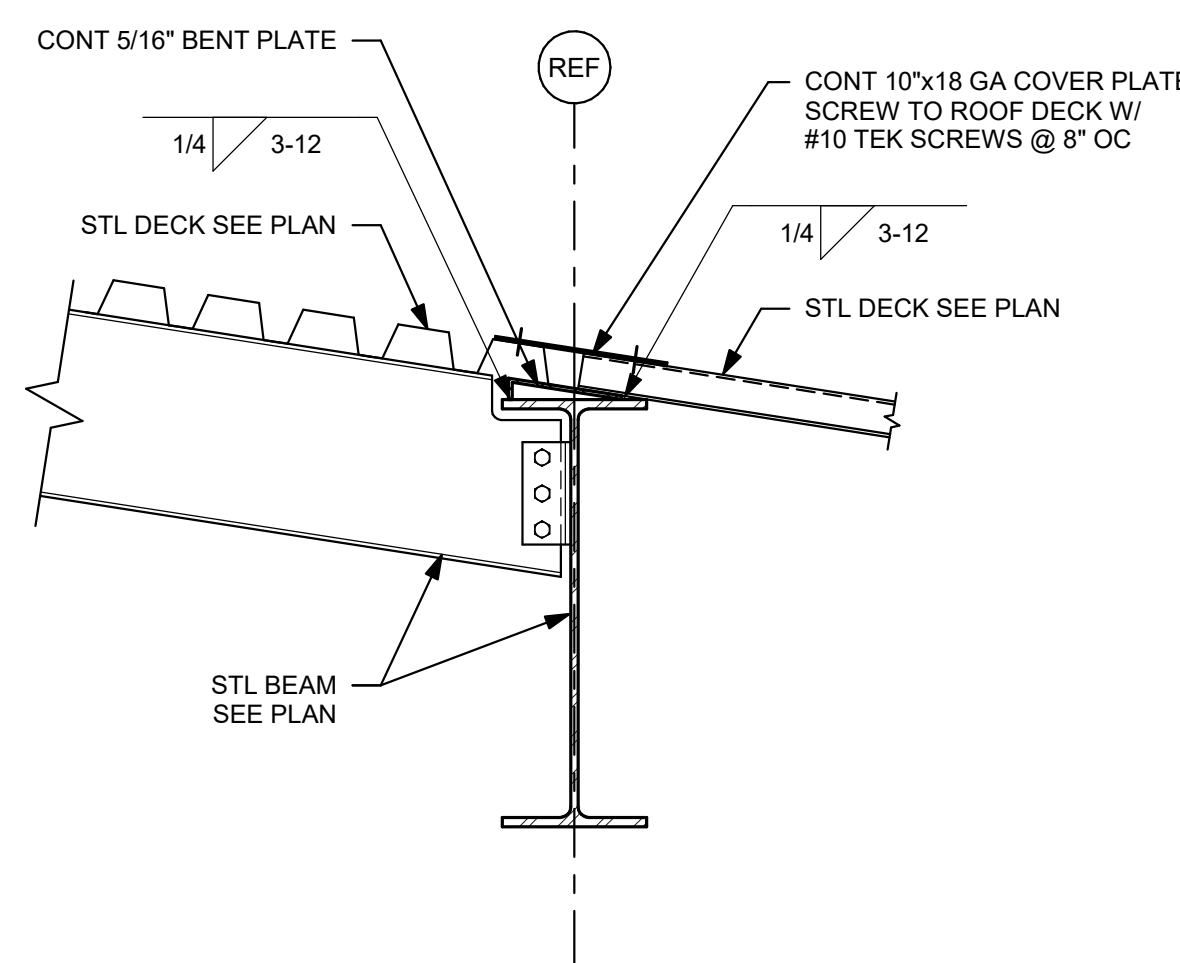
DETAIL C

DETAIL D

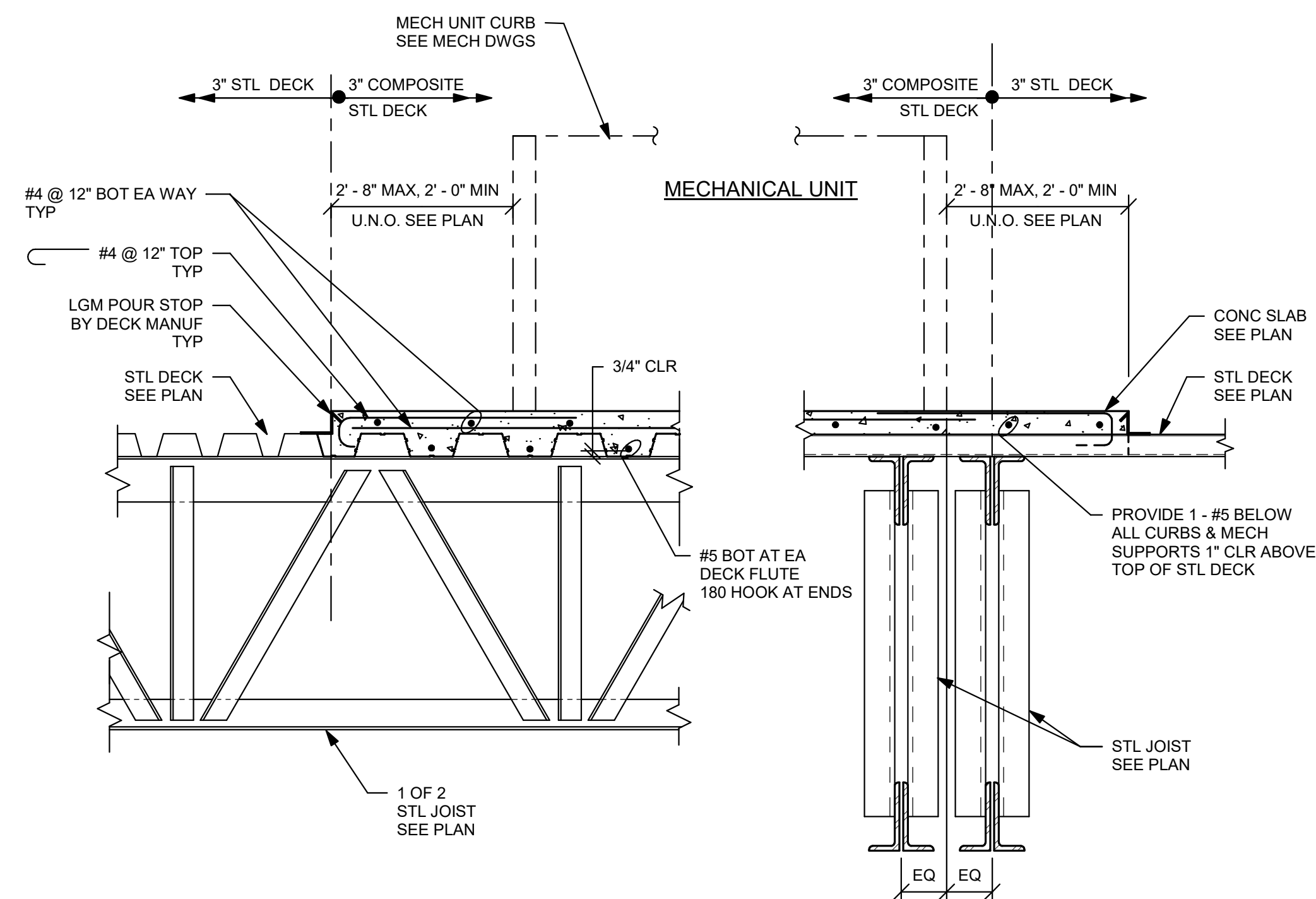
TYPICAL JOIST LOADING METHOD DETAIL

NOTES:

- WHERE POSSIBLE ALL HANGERS SHALL BE SUSPENDED FROM WIDE FLANGE BEAMS.
- PIPES, DUCTS, FIXTURES, ETC. SHALL NOT BE HUNG FROM NEW OR EXISTING STEEL DECK OR EXISTING CONCRETE SLAB OR BRIDGING ANGLES.
- THE SUM OF ALL BUILDING TRADES HANGER LOADS SHALL NOT EXCEED 250 LBS. PER JOIST. WHEN THIS MAXIMUM IS EXCEEDED A DETAIL OF PROPOSED METHOD OF HANGING SHALL BE SUBMITTED TO THE ARCHITECT FOR APPROVAL.
- HANGERS AND SUPPORTING MEMBERS SHALL BE LOCATED AT PANEL POINTS.
- PIPING WITH A 4" DIAMETER AND LARGER RUNNING PERPENDICULAR TO JOISTS SHALL BE SUPPORTED FROM EVERY JOIST.
- CONTRACTOR SHALL COORDINATE WITH ALL TRADES CONCERNED.
- USE THESE TYPICAL DETAILS WHERE APPLICABLE, UNLESS OTHERWISE SHOWN. FIELD MEASURE FOR SIZE OF EXISTING STRUCTURAL FRAMING.
- ALL ADDED UNISTRUTS, ANGLES, AND HANGERS TO BE SUPPLIED AND INSTALLED BY F.I.E.P. SUB-CONTRACTORS. REFER TO RELEVANT SPECIFICATIONS.
- REFER TO TYPICAL DETAIL 4 ON DRAWING S0-07 FOR ADDITIONAL REINFORCING WHEN HANGERS OR INTERMEDIATE SUPPORTING MEMBERS CAN NOT BE LOCATED AT PANEL POINT. ADDITIONAL REINFORCING SHALL BE SUPPLIED AND INSTALLED BY THE GENERAL CONTRACTOR.



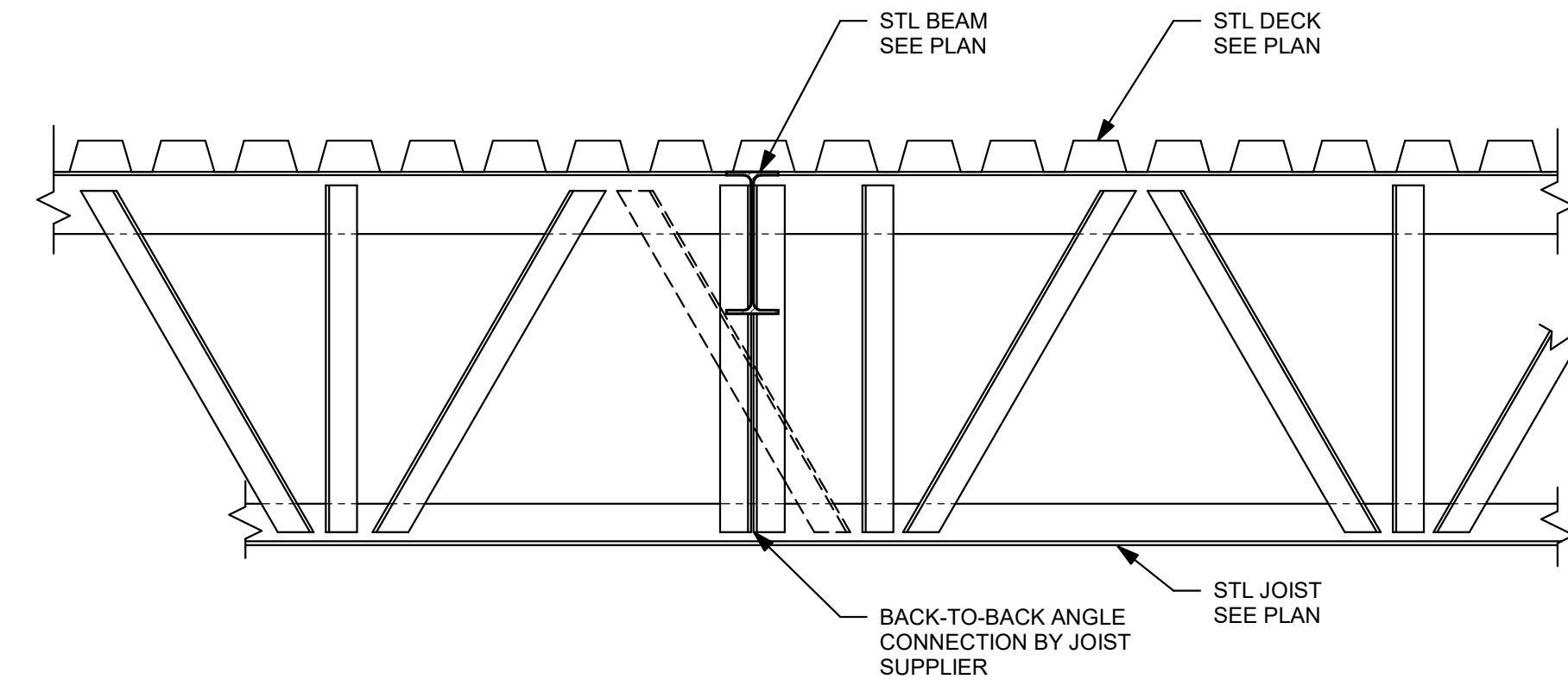
**TYPICAL SLOPED DECK SUPPORT
AT STEEL BEAM DETAIL**



TYPICAL MECHANICAL UNIT ROOF SLAB

NOTES:

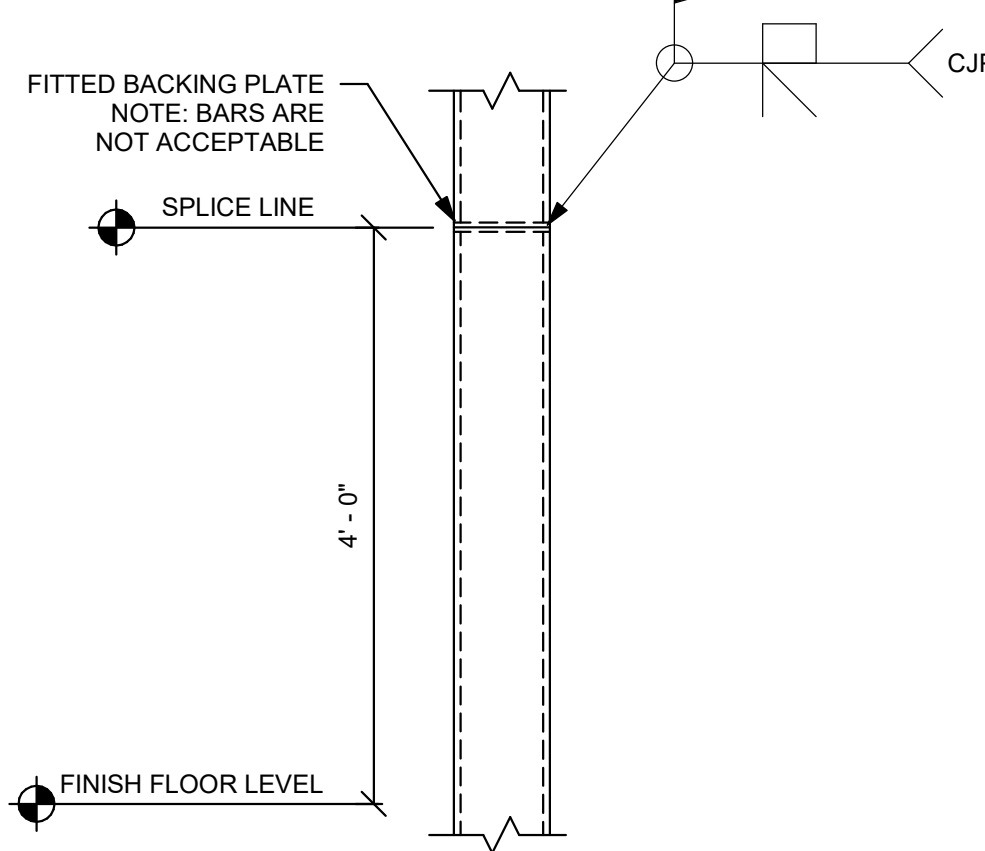
- 8" CONCRETE SLAB WITH 3" DEEP x 16 GAGE, GALVANIZED COMPOSITE METAL DECK.
- TOP OF SLAB ELEVATION VARIES WITH SLOPE OF DECK. MAINTAIN UNIFORM 6" SLAB THICKNESS.
- REFER TO PLANS FOR SLAB LOCATIONS.
- REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR MECHANICAL UNIT TO STRUCTURE CONNECTION AND ADDITIONAL DETAILS.
- COORDINATE SLAB OPENINGS WITH MECHANICAL DRAWINGS AND TYPICAL DETAILS.
- HOOK BARS AT RTU OPENINGS AND PROVIDE ADDITIONAL BARS AROUND OPENINGS PER TYPICAL DETAILS. OPENINGS GREATER THAN 2'-0" REQUIRE STEEL BEAMS PER TYPE 2 ON DETAIL 1 ON DRAWING S0-0-8.



**TYPICAL STEEL BEAM - TO - OPEN WEB
JOIST CONNECTION DETAIL**

NOTES:

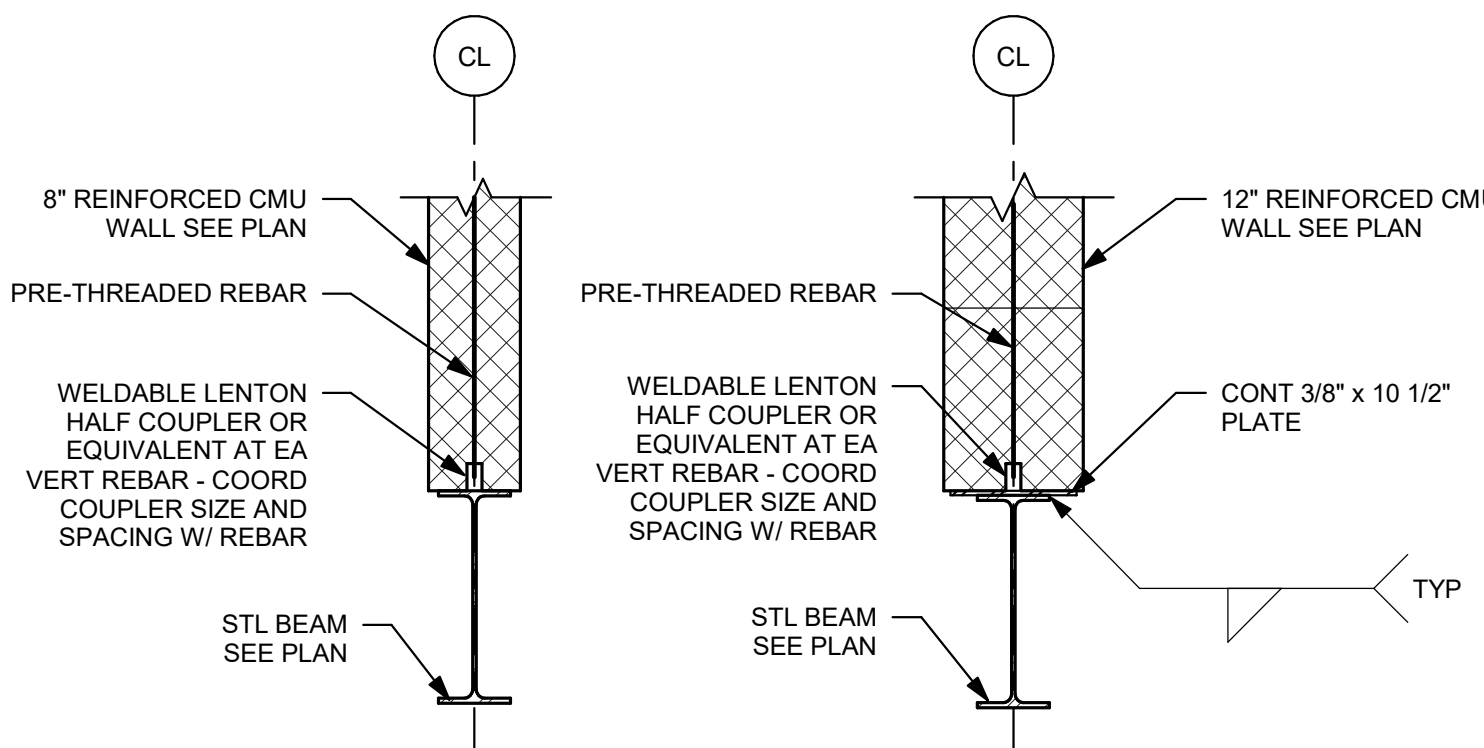
- COORDINATE DESIGN OF CONNECTION WITH STEEL BEAM REACTION TABLE ON S0-0-1.
- PROVIDE ADDITIONAL JOIST REINFORCEMENT FOR STEEL BEAMS NOT ALIGNED WITH PANEL POINTS AS REQUIRED. SEE DETAIL 2 ON S0-0-9 FOR ADDITIONAL INFORMATION.
- PROVIDE ADDITIONAL BRIDGING BETWEEN JOISTS AS REQUIRED.
- DO NOT SLOPE STEEL BEAM IF STEEL JOIST IS SLOPED. PROVIDE CONTINUOUS STEEL BENT PLATE ALONG STEEL BEAM AS REQUIRED FOR ADEQUATE DECK SUPPORT PER DETAIL 4 ON S0-0-9.



TYPICAL COLUMN SPLICE DETAIL

NOTES:

- SEE AISC STRUCTURAL STEEL MANUAL CASE X FOR INFORMATION NOT SHOWN.
- LIFTING AND ALIGNMENT CONNECTIONS TO BE DETAILED BY THE STEEL FABRICATOR.
- ALL LIFTING AND ALIGNMENT CONNECTIONS ARE TO BE REMOVED AND WELDS GROUND SMOOTH AFTER PERMANENT WELDING IS COMPLETE AND INSPECTED.



BEAM SCHEDULE			
MASONRY OPENING	0' - 0" - 5' - 0"	5' - 1" - 8' - 0"	8' - 1" - 12' - 0"
8" CMU	W8x24	W10x30	W12x35
12" CMU	W8x24	W10x33	W12x30

**TYPICAL LINTEL SCHEDULE AT
LOAD BEARING MASONRY**

NOTES:

- WELDABLE MECHANICAL COUPLER (ZAP SCREWLOK SL STRUCTURAL CONNECTOR) BY BAR SPLICE PRODUCTS INC OR EQUAL TO BE FURNISHED AND WELDED TO THE UNDERSIDE OF THE STEEL BEAM OR STEEL COLUMN UNDER (SPECIFICATION 040300) UNLESS NOTED OTHERWISE IN RELEVANT SECTIONS.
- REFER TO ARCHITECTURAL DRAWINGS FOR MASONRY SIZE AND LOCATION WITH RESPECT TO THE STEEL COLUMN.
- COORDINATE WELDING REQUIREMENTS OF THE COUPLER WITH THE MANUFACTURER.

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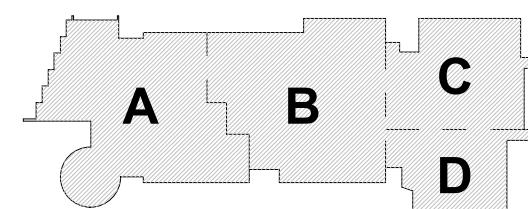
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PR-002	6/29/2023 MISCELLANEOUS STRUCTURAL REVISIONS

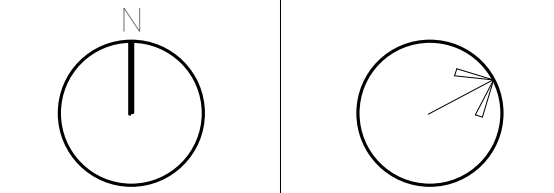
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August 28th, 2023



KEY PLAN

PROJECT NORTH MAGNETIC NORTH



**TYPICAL
DETAILS**

Scale: 3/4" = 1'-0"

Job No.: 20202

Drawn By: EDG

Date: August 28th, 2023

S0-0-9

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FOUNDATION NOTES:

- REFER TO GRADING DRAWINGS FOR PLAN AND GRADE ELEVATIONS. THE STRUCTURAL DRAWINGS USES A DATUM OF 100'-0" AT THE FIRST FLOOR LEVEL, EQUAL TO 163.50' ON THE SITE GRADING PLANS.
- FOR GENERAL NOTES AND TYPICAL DETAILS SEE DRAWINGS S0-0.1, S0-0.2, S0-0.3, S0-0.4, S0-0.5, S0-0.6, S0-0.7 AND S0-0.8.
- F3 ETC., INDICATES A FOOTING TYPE, FOR SIZE OF FOOTING AND REINFORCEMENT SEE SCHEDULE ON THIS DRAWING.
- TOP OF FOOTING ELEVATION TO BE 3'-6" MINIMUM BELOW LOWEST ADJACENT FINISHED GRADE AT EXTERIOR CONDITIONS AND 2'-0" BELOW TOP OF CONCRETE SLAB AT INTERIOR CONDITIONS. ALL OTHER TOP OF FOOTING ELEVATIONS ARE DENOTED AS THUS (X'-X") COMPUTED FROM A DATUM ELEVATION OF 100'-0" ON PLANS. CONTRACTOR TO COORDINATE AND VERIFY ALL TOP OF FOOTING ELEVATIONS WITH UNDERGROUND PLUMBING SUB-CONTRACTORS FIELD LAYOUT.
- ALL FOOTING ELEVATIONS NOTED ON PLAN ARE SHOWN ONLY TO ASSIST IN COORDINATION. ALL FOOTING ELEVATIONS MUST BE COORDINATED WITH STRUCTURAL REQUIREMENTS, TYPICAL DETAILS, ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS.
- ALL FOOTINGS TO BE CENTERED UNDER COLUMNS UNLESS NOTED OTHERWISE.
- SF INDICATES A STEPPED FOOTING REFER TO DETAIL 1 ON DRAWING S0-0.2.
- C1 ETC., INDICATES A COLUMN TYPE, FOR SIZE OF COLUMNS AND BASE PLATES SEE SCHEDULE ON THIS DRAWING.
- BOTTOM OF BASE PLATE ELEVATION TO BE 1'-11" MINIMUM BELOW TOP OF CONCRETE SLAB AT INTERIOR CONDITIONS, AND 1'-11" BELOW TOP OF CONCRETE SLAB AT EXTERIOR CONDITIONS. UNLESS NOTED OTHERWISE AS "XX" REFER TO ARCHITECTURAL DRAWINGS FOR BRICK SHELF ELEVATIONS.
- FOR UNDER SLAB DRAINAGE AND WALL DRAINS, COORDINATE WITH ARCHITECTURAL, STRUCTURAL, CIVIL, AND PLUMBING DRAWINGS.
- INDICATES A DEPRESSIONED SLAB ON GRADE. REFER TO DETAILS 9 AND 10 ON DRAWING S0-0.2 COORDINATE ALL SLAB DEPRESSIONS WITH REQUIREMENTS ON ARCHITECTURAL DRAWINGS.
- FOR TYPICAL EXTERIOR DOOR DETAIL REFER TO DETAIL 5 ON DRAWING S0-0.8 AND RELEVANT SECTIONS.
- BF-1 ETC., INDICATES A BRACED BAY. REFER TO BRACED FRAME ELEVATIONS AND DETAILS ON DRAWINGS S4-0.5, S4-0.6, S4-0.7, S4-0.8, AND S4-0.9 FOR ADDITIONAL INFORMATION.
- INDICATES A CMU WALL. REFER TO TYPICAL DETAIL 8 ON DRAWING S0-0.4 FOR REINFORCEMENT. DETAIL 6 ON DRAWING S0-0.5 FOR CONNECTION OF SHEAR WALLS TO BEAMS AND COLUMNS AND DETAIL 7 ON DRAWING S4-0.5 FOR CONNECTIONS TO STEEL BEAMS AND CONCRETE SLABS AT THE TOP OF WALL FOR NON-STRUCTURAL WALLS. REFER TO RELEVANT SECTIONS FOR CONNECTIONS OF SHEAR WALLS TO THE STRUCTURE.
- FOR DIMENSIONS AND ELEVATIONS NOT GIVEN REFER TO ARCHITECTURAL DRAWINGS.
- INDICATES CONCRETE PIER REFER TO TYPICAL DETAIL 5 ON DRAWING S0-0.2.
- INDICATES UNDERGROUND UTILITY LINES PLUMBING THROUGH CONCRETE FOUNDATION WALL TYPICAL. COORDINATE FOOTING ELEVATION WITH PIPE INVERTS AND TYPICAL STRUCTURAL DETAILS.
- CONCRETE PIER REINFORCING PER DETAIL 5 ON DRAWING S0-0.2 IS TO BE PROVIDED FOR ALL CONCRETE WALLS SUPPORTING COLUMNS. HORIZONTAL WALL REINFORCING MUST REMAIN CONTINUOUS.

COLUMN SCHEDULE *		
MARK	SIZE	BASE PLATE SIZE
C1	HSS8x8x3/8	1" x 16" x 1'-4"
C2	HSS8x8x1/2	1" x 16" x 1'-4"
C3	HSS12x12x3/8	1" x 20" x 1'-8"
C4	HSS12x12x1/2	1" x 20" x 1'-8"
C5	HSS12x12x5/8	1" x 20" x 1'-8"
C6	HSS12.75x500	1" x 20" x 1'-8"
C7	HSS20x12x1/2	1 1/2" x 20" x 2'-4"
C8	HSS8x8x3/8	1" x 16" x 1'-0"
C9	HSS16x500	1 1/2" x 24" x 2'-0"
C10	HSS12x6x1/2	1 1/2" x 20" x 1'-2"
C11	HSS10x500	1" x 16" x 1'-6"
C12	HSS8x8x3/8	1" x 14" x 1'-2"
C13	HSS6x6x3/8	1" x 12" x 1'-2"

* BASE PLATE LENGTH AND WIDTH SPECIFIED IN SCHEDULE IS THE MINIMUM SIZE FOR A COLUMN THAT IS PART OF A BRACED FRAME. SEE FOUNDATION NOTE ABOVE AND REFER TO DETAILS ON DRAWING S4-0.8 AND S4-0.9 FOR ADDITIONAL INFORMATION.

* PROVIDE 4 - 1" DIA F1554-5581 ANCHOR RODS TYPICALLY. REFER TO DETAILS ON DRAWING S4-0.8 AND S4-0.9 FOR ADDITIONAL ANCHOR RODS FOR COLUMN RECEIVING BRACINGS.

FOOTING SCHEDULE F SERIES

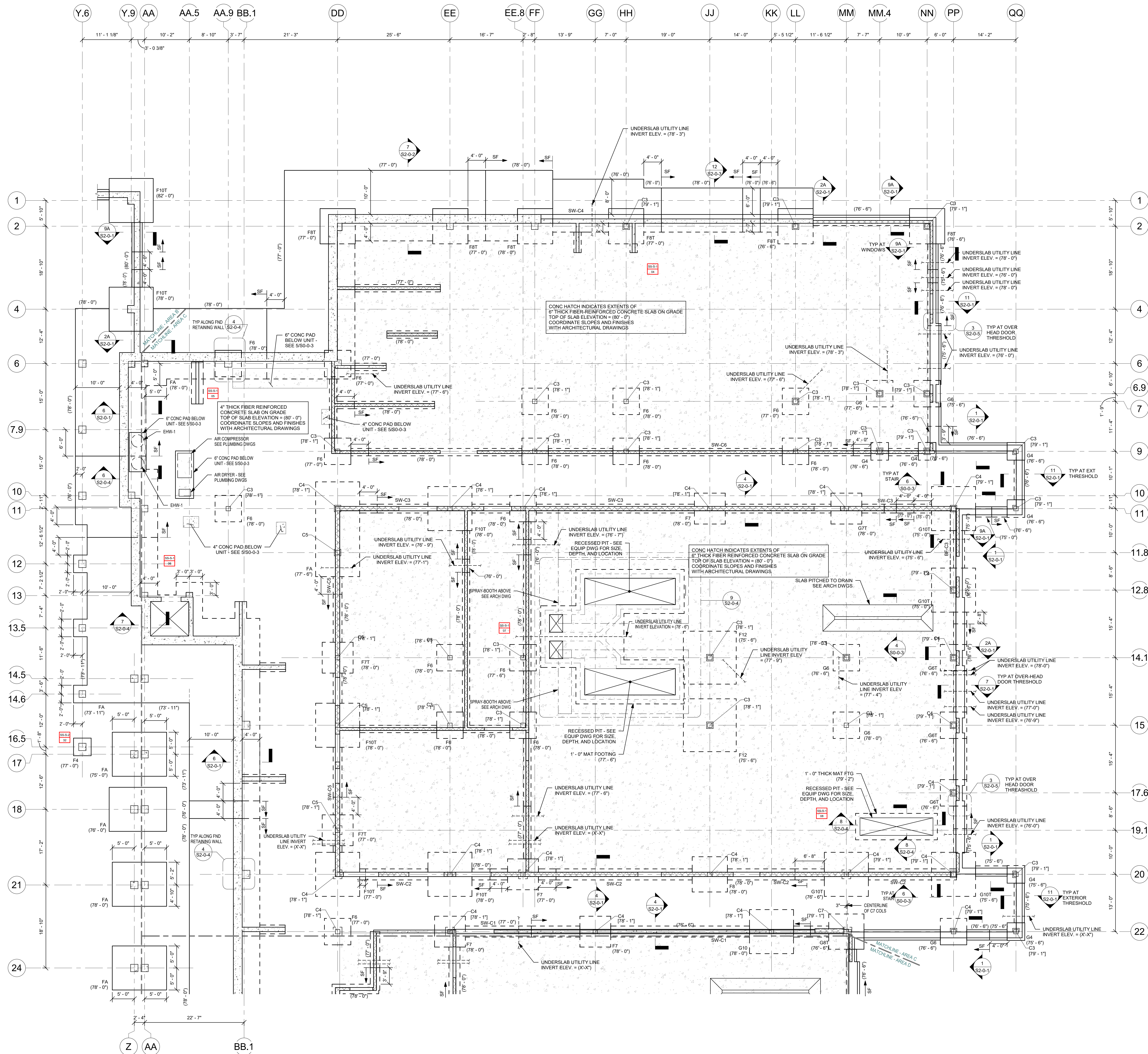
MARK	SIZE	REINFORCEMENT
F4	4'-0" x 4'-0" x 2'-0"	6 - #5 BOT EA WAY
F5	5'-0" x 5'-0" x 2'-0"	7 - #5 BOT EA WAY
F6	6'-0" x 6'-0" x 2'-0"	8 - #5 BOT EA WAY
F7	7'-0" x 7'-0" x 2'-0"	9 - #5 BOT EA WAY
F8	8'-0" x 8'-0" x 3'-0"	10 - #5 BOT EA WAY
F9	9'-0" x 9'-0" x 3'-0"	11 - #5 BOT EA WAY
F10	10'-0" x 10'-0" x 3'-0"	12 - #5 BOT EA WAY
F11	11'-0" x 11'-0" x 3'-0"	13 - #10 BOT EA WAY
F12	12'-0" x 12'-0" x 4'-0"	14 - #10 BOT EA WAY
FA	SEE PLAN X 2'-0"	#8 @ 12" OC TOP AND BOT EA WAY

T INDICATES TOP REINFORCING TO MATCH BOTTOM REINFORCING

FOOTING SCHEDULE G SERIES

MARK	SIZE	REINFORCEMENT
G4	4'-0" x 4'-0" x 2'-0"	5 - #5 BOT EA WAY
G5	5'-0" x 5'-0" x 2'-0"	6 - #5 BOT EA WAY
G6	6'-0" x 6'-0" x 2'-0"	7 - #5 BOT EA WAY
G7	7'-0" x 7'-0" x 2'-0"	8 - #5 BOT EA WAY
G8	8'-0" x 8'-0" x 2'-0"	9 - #5 BOT EA WAY
G9	9'-0" x 9'-0" x 2'-0"	10 - #7 BOT EA WAY
G10	10'-0" x 10'-0" x 2'-0"	11 - #7 BOT EA WAY
G11	11'-0" x 11'-0" x 2'-0"	12 - #8 BOT EA WAY
G12	12'-0" x 12'-0" x 3'-0"	13 - #8 BOT EA WAY
G13	13'-0" x 13'-0" x 3'-0"	14 - #9 BOT EA WAY
G14	14'-0" x 14'-0" x 3'-0"	15 - #9 BOT EA WAY
G15	15'-0" x 15'-0" x 3'-0"	16 - #9 BOT EA WAY
GA	SEE PLAN X 2'-0"	#8 @ 12" OC TOP AND BOT EA WAY

T INDICATES TOP REINFORCING TO MATCH BOTTOM REINFORCING



FIREPROOFING NOTES:

- STEEL COLUMNS, SHOWN ON THIS DRAWING, SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING UNLESS INDICATED. AS A ROUND HSS COLUMN, EXPOSED TO VIEW PORTION OF ROUND HSS COLUMNS SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING. CONCEALED FROM VIEW PORTION OF ROUND HSS COLUMN SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
- BRACED FRAME SHOWN ALONG COLUMN GRID AA BETWEEN GRIDS 6 AND 11, SHALL RECEIVE 2-HOUR RATING BY CEMENTITIOUS FIREPROOFING. ALL OTHER EXPOSED TO VIEW BRACED FRAMES SHOWN ON THIS DRAWING, SHALL RECEIVE 2-HOUR RATING BY INTUMESCENT MASTIC FIREPROOFING.
- COORDINATE FIREPROOFING REQUIREMENTS WITH THE SPECIFICATIONS AND ARCHITECTURAL DRAWINGS.

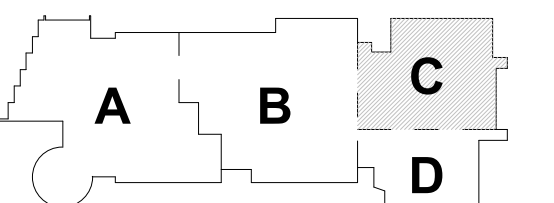
BRACE FRAME KEY

0" TYP	WF	BF-X	INDICATES A BRACE FRAME ABOVE AND BELOW LEVEL
0" TYP	BF-X	WF	INDICATES A BRACE FRAME ABOVE LEVEL
0" TYP	WF	BF-X	INDICATES A BRACE FRAME BELOW LEVEL

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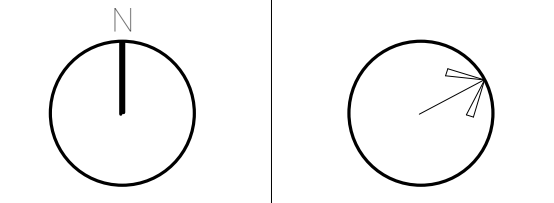
BID SET

August 28th, 2023



PROJECT NORTH

MAGNETIC NORTH



LOWER LEVEL
FOUNDATION
PLAN - AREA C

Scale: 1/8" = 1'-0"

Job No.: 20202

Drawn By: EDG

Date: August 28th, 2023

S1-1-0C

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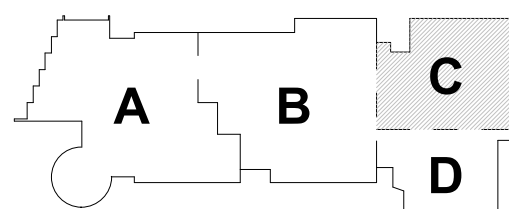
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03/31/2023	EARLY STRUCTURAL BID PACKAGE
REVISION LIST	
SS-S-1	4/14/2023 STRUCTURAL STEEL ADDENDUM 1

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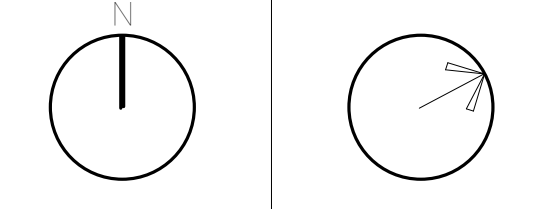
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August 28th, 2023



KEY PLAN

PROJECT NORTH
MAGNETIC NORTH



LOWER LEVEL
FOUNDATION
PLAN - AREA D

Scale: 1/8" = 1'-0"

Job No.: 20202

Drawn By: EDG

Date: August 28th, 2023

S1-1-0D

FOUNDATION NOTES:

- REFER TO GRADING DRAWINGS FOR PLAN AND GRADE ELEVATIONS. THE STRUCTURAL DRAWINGS USES A DATUM OF 100'-0" AT THE FIRST FLOOR LEVEL, EQUAL TO (163.50') ON THE SITE GRADING PLANS.
- FOR GENERAL NOTES AND TYPICAL DETAILS SEE DRAWINGS S0-0-1, S0-0-2, S0-0-3, S0-0-4, S0-0-5, S0-0-6, S0-0-7 AND S0-0-8.
- ETC., INDICATES A FOOTING TYPE, FOR SIZE OF FOOTING AND REINFORCEMENT SEE SCHEDULE ON THIS DRAWING.
- TOP OF FOOTING ELEVATION TO BE 3'-6" MINIMUM BELOW LOWEST ADJACENT FINISHED GRADE AT EXTERIOR CONDITIONS AND 2'-0" BELOW TOP OF CONCRETE SLAB AT INTERIOR CONDITIONS. ALL OTHER TOP OF FOOTING ELEVATIONS ARE DENOTED AS THUS (4'-1") COMPUTED FROM A DATUM ELEVATION OF 100'-0" ON PLANS. CONTRACTOR TO COORDINATE AND VERIFY ALL TOP OF FOOTING ELEVATIONS WITH UNDERGROUND PLUMBING SUB-CONTRACTORS FIELD LAYOUT.
- ALL FOOTING ELEVATIONS NOTED ON PLAN ARE SHOWN ONLY TO ASSIST IN COORDINATION. ALL FOOTING ELEVATIONS MUST BE COORDINATED WITH STRUCTURAL REQUIREMENTS, TYPICAL DETAILS, ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS.
- ALL FOOTINGS TO BE CENTERED UNDER COLUMNS UNLESS NOTED OTHERWISE.
- SF INDICATES A STEPPED FOOTING REFER TO DETAIL 1 ON DRAWING S0-0-2.
- C1 ETC., INDICATES A COLUMN TYPE, FOR SIZE OF COLUMNS AND BASE PLATES SEE SCHEDULE ON THIS DRAWING.
- BOTTOM OF BASE PLATE ELEVATION TO BE 1'-11" MINIMUM BELOW TOP OF CONCRETE SLAB AT EXTERIOR CONDITIONS, AND 0'-11" BELOW TOP OF CONCRETE SLAB AT INTERIOR CONDITIONS. UNLESS NOTED OTHERWISE AS (X'-X") REFER TO ARCHITECTURAL DRAWINGS FOR BRICK SHELF ELEVATIONS.
- FOR UNDER SLAB DRAINAGE AND WALL DRAINS, COORDINATE WITH ARCHITECTURAL, STRUCTURAL, CIVIL, AND PLUMBING DRAWINGS.
- INDICATES A DEPRESSED SLAB ON GRADE. REFER TO DETAILS 9 AND 10 ON DRAWING S0-0-2 COORDINATE ALL SLAB DEPRESSIONS WITH REQUIREMENTS ON ARCHITECTURAL DRAWINGS.
- FOR TYPICAL EXTERIOR DOOR DETAIL REFER TO DETAIL 6 ON DRAWING S0-0-8 AND RELEVANT SECTIONS.
- BF-1 ETC., INDICATES A BRACED BAY. REFER TO BRACED FRAME ELEVATIONS AND DETAILS ON DRAWINGS S4-0-5, S4-0-6, S4-0-7, S4-0-8, AND S4-0-9 FOR ADDITIONAL INFORMATION.
- INDICATES A CMU WALL. REFER TO TYPICAL DETAIL 8 ON DRAWING S0-0-4 FOR REINFORCEMENT. DETAIL 5 ON DRAWING S0-0-5 FOR CONNECTION OF SHEAR WALLS TO BEAMS AND COLUMNS. DETAIL 7 ON DRAWING S0-0-5 FOR CONNECTIONS TO STEEL BEAMS AND CONCRETE SLABS AT THE TOP OF WALL. FOR NON-STRUCTURAL WALLS, REFER TO RELEVANT SECTIONS FOR CONNECTIONS OF SHEAR WALLS TO THE STRUCTURE.
- FOR DIMENSIONS AND ELEVATIONS NOT GIVEN REFER TO ARCHITECTURAL DRAWINGS.
- INDICATES CONCRETE PIER REFER TO TYPICAL DETAIL 5 ON DRAWING S0-0-2.
- INDICATES UNDERGROUND UTILITY LINES PLUMBING THROUGH CONCRETE FOUNDATION WALLS. COORDINATE FOOTING ELEVATION WITH PIPE INVERTS AND TYPICAL STRUCTURAL DETAILS.
- CONCRETE PIER REINFORCING PER DETAIL 5 ON DRAWING S0-0-2 IS TO BE PROVIDED FOR ALL CONCRETE WALLS SUPPORTING COLUMNS. HORIZONTAL WALL REINFORCING MUST REMAIN CONTINUOUS.

COLUMN SCHEDULE *		
MARK	SIZE	BASE PLATE SIZE
C1	HSS8x8x3/8	1' x 16' x 1' - 4"
C2	HSS8x8x1/2	1' x 16' x 1' - 4"
C3	HSS12x12x3/8	1' x 20' x 1' - 8"
C4	HSS12x12x1/2	1' x 20' x 1' - 8"
C5	HSS12x12x5/8	1' x 20' x 1' - 8"
C6	HSS12.75x6.500	1' x 20' x 1' - 8"
C7	HSS20x12x1/2	1 1/2' x 20' x 2' - 4"
C8	HSS8x4x3/8	1' x 16' x 1' - 0"
C9	HSS16x6.500	1 1/2' x 24' x 2' - 0"
C10	HSS12x6x1/2	1 1/2' x 20' x 1' - 2"
C11	HSS10x6.500	1' x 18' x 1' - 6"
C12	HSS6x6x3/8	1' x 14' x 1' - 2"
C13	HSS6x4x3/8	1' x 12' x 1' - 2"

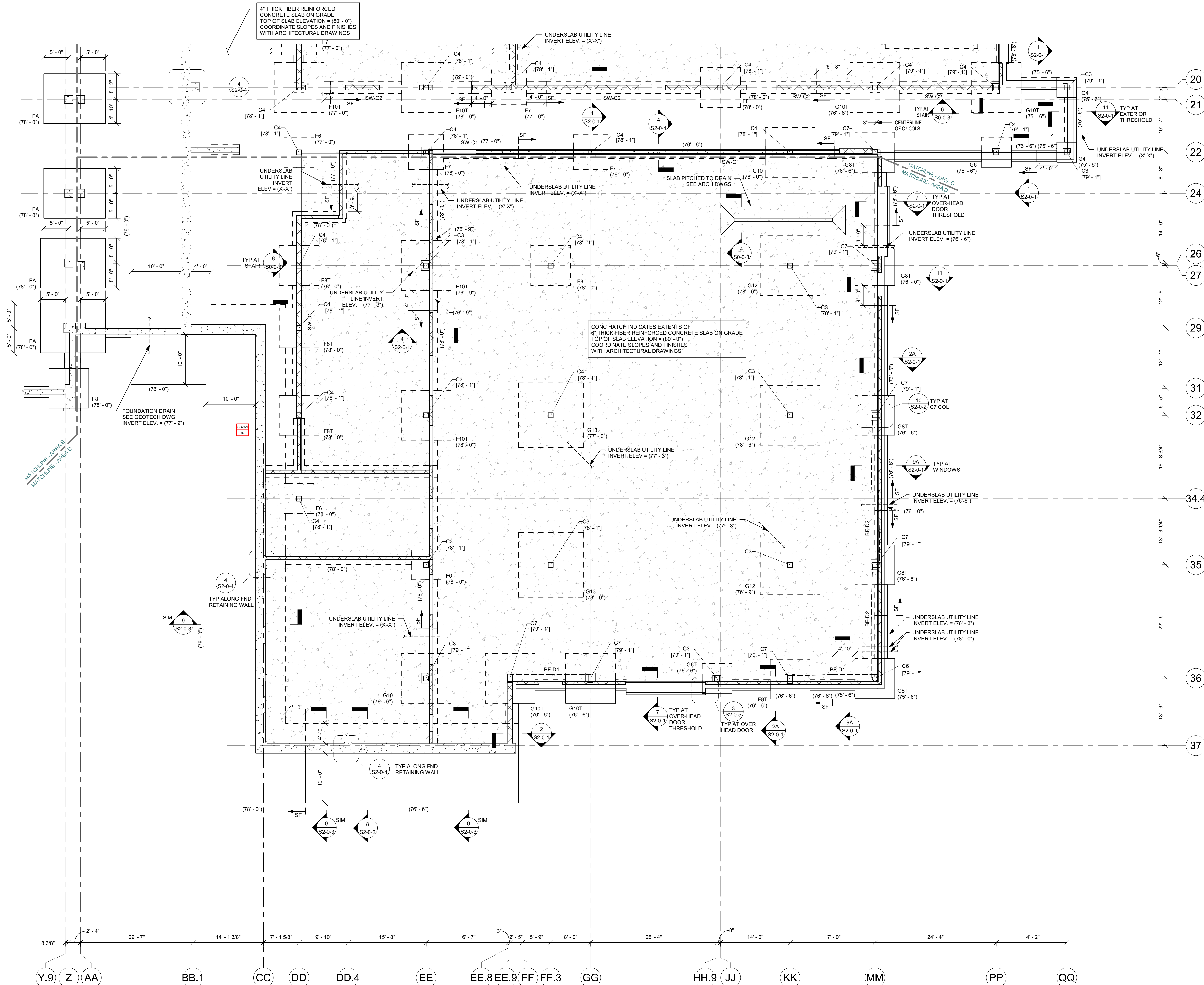
* BASE PLATE LENGTH AND WIDTH SPECIFIED IN SCHEDULE IS THE MINIMUM SIZE FOR A COLUMN THAT IS PART OF A BRACED FRAME. SEE FOUNDATION NOTE ABOVE AND REFER TO DETAILS ON DRAWING S4-0-8 AND S4-0-9 FOR ADDITIONAL INFORMATION.
* PROVIDE 4" DIA F1554-5551 ANCHOR RODS TYPICALLY REFER TO DETAILS ON DRAWING S4-0-8 AND S4-0-9 FOR ADDITIONAL ANCHOR RODS FOR COLUMN RECEIVING BRACING.

FOOTING SCHEDULE F SERIES		
MARK	SIZE	REINFORCEMENT
F4	4'-0" x 4'-0" x 2'-0"	6 - #5 BOT EA WAY
F5	5'-0" x 5'-0" x 2'-0"	7 - #5 BOT EA WAY
F6	6'-0" x 6'-0" x 2'-0"	8 - #6 BOT EA WAY
F7	7'-0" x 7'-0" x 2'-0"	9 - #6 BOT EA WAY
F8	8'-0" x 8'-0" x 3'-0"	10 - #8 BOT EA WAY
F9	9'-0" x 9'-0" x 3'-0"	11 - #9 BOT EA WAY
F10	10'-0" x 10'-0" x 3'-6"	12 - #9 BOT EA WAY
F11	11'-0" x 11'-0" x 3'-6"	13 - #10 BOT EA WAY
F12	12'-0" x 12'-0" x 4'-0"	14 - #10 BOT EA WAY
FA	SEE PLAN x 2' - 0"	#8 @ 12" OC TOP AND BOT EA WAY

T INDICATES TOP REINFORCING TO MATCH BOTTOM REINFORCING

FOOTING SCHEDULE G SERIES		
MARK	SIZE	REINFORCEMENT
G4	4'-0" x 4'-0" x 2'-0"	5 - #5 BOT EA WAY
G5	5'-0" x 5'-0" x 2'-0"	6 - #5 BOT EA WAY
G6	6'-0" x 6'-0" x 2'-0"	7 - #6 BOT EA WAY
G7	7'-0" x 7'-0" x 2'-0"	8 - #6 BOT EA WAY
G8	8'-0" x 8'-0" x 2'-0"	9 - #6 BOT EA WAY
G9	9'-0" x 9'-0" x 2'-6"	10 - #7 BOT EA WAY
G10	10'-0" x 10'-0" x 2'-6"	11 - #7 BOT EA WAY
G11	11'-0" x 11'-0" x 2'-6"	12 - #8 BOT EA WAY
G12	12'-0" x 12'-0" x 3'-0"	13 - #8 BOT EA WAY
G13	13'-0" x 13'-0" x 3'-0"	14 - #9 BOT EA WAY
G14	14'-0" x 14'-0" x 3'-0"	15 - #9 BOT EA WAY
G15	15'-0" x 15'-0" x 3'-0"	16 - #9 BOT EA WAY
GA	SEE PLAN x 2' - 0"	#8 @ 12" OC TOP AND BOT EA WAY

T INDICATES TOP REINFORCING TO MATCH BOTTOM REINFORCING



- FIREPROOFING NOTES:**
- STEEL COLUMNS, SHOWN ON THIS DRAWING, SHALL RECEIVE 2-HOUR FIRE RATINGS BY CEMENTITIOUS FIREPROOFING.
 - EXPOSED TO VIEW BRACED FRAMES SHOWN ON THIS DRAWING, SHALL RECEIVE 2-HOUR RATING BY INTUMESCENT MASTIC FIREPROOFING.
 - COORDINATE FIREPROOFING REQUIREMENTS WITH THE SPECIFICATIONS AND ARCHITECTURAL DRAWINGS.

BRACE FRAME KEY		
0" TYP	WF	BF-X
0" TYP	WF	BF-X
0" TYP	WF	BF-X

INDICATES A BRACE FRAME ABOVE AND BELOW LEVEL
INDICATES A BRACE FRAME ABOVE LEVEL
INDICATES A BRACE FRAME BELOW LEVEL

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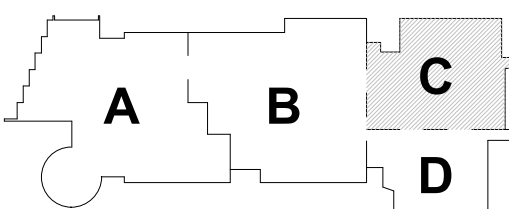
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03/31/2023 EARLY STRUCTURAL BID PACKAGE

REVISION LIST
SS-S-1 4/14/2023 STRUCTURAL STEEL
ADDENDUM 1

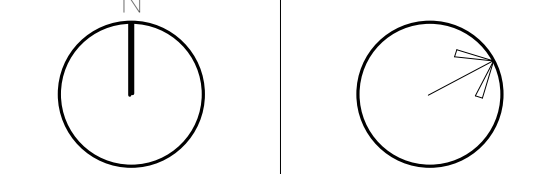
BID SET

August 28th, 2023



KEY PLAN

PROJECT NORTH MAGNETIC NORTH



MEZZANINE FLOOR FRAMING PLAN - AREA C

Scale: 1/8" = 1'-0"
Job No.: 20202
Drawn By: EDG
Date: August 28th, 2023

S1-1-0MC

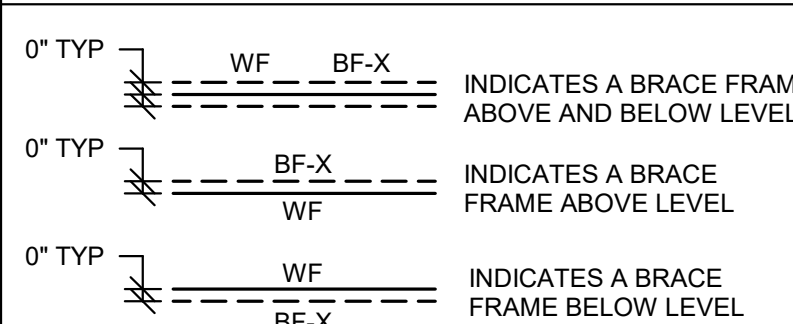
FRAMING NOTES:

- FOR GENERAL NOTES AND TYPICAL DETAILS SEE DRAWINGS S0-0-1, S0-0-2, S0-0-3, S0-0-4, S0-0-5, S0-0-6, S0-0-7 AND S0-0-8.
- REFER TO ARCHITECTURAL DRAWINGS FOR ELEVATIONS AND VERTICAL DIMENSIONS. PITCH ALL STEEL UNIFORMLY TO LOW POINTS AT THE COLUMNS AND BENT BEAMS AS SHOWN ON THE ARCHITECTURAL DRAWINGS.
- BF-1 ETC. INDICATES A BRACED BAY. REFER TO BRACED FRAME ELEVATIONS AND DETAILS ON DRAWINGS S4-0-1, S4-0-2, S4-0-3, S4-0-4, S4-0-5, S4-0-6, S4-0-7 AND S4-0-8 FOR ADDITIONAL INFORMATION.
- [XX] INDICATES THE NUMBER OF 3/4" DIAMETER x 4 1/4" LONG HEADED STUDS WELDED TO THE TOP FLANGE OF THE BEAM. SPACE STUDS EVENLY ALONG THE BEAM UNLESS NOTED OTHERWISE.
- INDICATES A MOMENT CONNECTION TO DEVELOP THE FULL CAPACITY OF THE MEMBER. REFER TO TYPICAL DETAILS 7, 8 AND 9 ON DRAWING S0-0-6.
- INDICATES A 5/16" FILLET WELD ALL AROUND (HSS BEAM TO HSS COLUMN) WHERE BEAM DIMENSIONS EXCEED COLUMN DIMENSIONS PROVIDE 1/2" THICK STEEL CAP PLATE TO ACHIEVE ALL AROUND WELD. REFER TO TYPICAL DETAIL 2 ON DRAWING S0-0-7.
- <X'> INDICATES UPWARD CAMBER AT THE MID-SPAN OF THE MEMBER.
- 5-1/4" INDICATES SPAN DIRECTION OF 2" DEEP, 20 GAGE GALVANIZED COMPOSITE STEEL DECK WITH 3 1/4" LIGHT WEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 5 1/4". REINFORCE WITH 6x6 - W2.1xW2.1 WWR.
- 1-1/2" INDICATES SPAN DIRECTION OF 1 1/2" DEEP, 20 GAGE TYPE B, GALVANIZED STEEL ROOF DECK.
- 3" NA INDICATES SPAN DIRECTION OF 3" DEEP, 20 GAGE TYPE NA, GALVANIZED ACOUSTIC STEEL ROOF DECK.
- 4" LW INDICATES SPAN DIRECTION OF 1 1/2" DEEP, 20 GAGE GALVANIZED COMPOSITE STEEL DECK WITH 3 1/2" LIGHT WEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 4" REINFORCE WITH 6x6 - W2.1xW2.1 WWR.
- FOR EXACT NUMBER, SIZE, AND LOCATION OF OPENING IN STEEL DECKING REFER TO MECHANICAL AND ARCHITECTURAL DRAWINGS FOR FRAMING INFORMATION. REFER TO DETAILS 1, 2 AND 3 ON DRAWING S0-0-8.
- 6" HATCHED AREA INDICATES LOCATION OF CONCRETE SLAB WITH 3" DEEP, 18 GAGE GALVANIZED COMPOSITE STEEL DECK WITH 4" NORMAL WEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 6". REINFORCE WITH 6x6 - W2.1xW2.1 WWR. REFER TO TYPICAL DETAIL 5 ON DRAWING S0-0-7 FOR ADDITIONAL INFORMATION. USE 3/4" DIA x 2' LONG HEADED STUDS.
- INDICATES A ROOF DRAIN. REFER TO TYPICAL STRUCTURAL DETAILS 1 AND 2 ON DRAWING S0-0-8 AND DETAIL 1 ON DRAWING S0-0-4. FOR DECKING SUPPORT, REFER TO DETAIL 5 ON DRAWING S0-0-7. REFER TO PLUMBING AND ARCHITECTURAL DRAWINGS FOR OPENING SIZES AND LOCATIONS.
- CT INDICATES A COLUMN TERMINATES AT THIS LEVEL.
- WF INDICATES A BEND IN THE STEEL BEAM. REFER TO TYPICAL DETAIL 9 ON DRAWING S0-0-7.
- INDICATES A CMU WALL. REFER TO TYPICAL DETAIL 8 ON DRAWING S0-0-4 FOR REINFORCEMENT. DETAIL 6 ON DRAWING S0-0-3 FOR CONNECTION OF SHEAR WALLS TO BEAMS AND COLUMNS. DETAIL 7 ON DRAWING S0-0-5 FOR CONNECTIONS TO STEEL BEAMS AND CONCRETE SLABS AT THE TOP OF WALL FOR NON-STRUCTURAL WALLS. REFER TO RELEVANT SECTIONS FOR CONNECTIONS OF SHEAR WALLS TO THE STRUCTURE.
- 6" x 2" PC PLANK INDICATES SPAN OF 2" DEEP PRESTRESSED, PRECAST CONCRETE HOLLOW CORE PLANK WITH A MINIMUM 2" OF NORMAL WEIGHT CONCRETE TOPPING. PLANK AND CONCRETE TOPPING SLAB TO BE DESIGNED TO SUPPORT A MINIMUM LIVE LOAD CAPACITY OF 150 PSF. PRECAST CONCRETE PLANK TO BE DESIGNED FOR MINIMUM OF 2 HOUR FIRE RATING.
- 10" x 2" PC PLANK INDICATES SPAN OF 10" DEEP PRESTRESSED, PRECAST CONCRETE HOLLOW CORE PLANK WITH A MINIMUM 2" OF NORMAL WEIGHT CONCRETE TOPPING. PLANK AND CONCRETE TOPPING SLAB TO BE DESIGNED TO SUPPORT A MINIMUM LIVE LOAD CAPACITY OF 150 PSF. PRECAST CONCRETE PLANK TO BE DESIGNED FOR MINIMUM OF 2 HOUR FIRE RATING.
- FOR DIMENSIONS AND ELEVATIONS NOT GIVEN REFER TO ARCHITECTURAL DRAWINGS.
- WF INDICATES A SPLICE CONNECTION ALONG A CONTINUOUS STEEL BEAM. DESIGN SPLICE CONNECTION FOR FULL CAPACITY OF STEEL BEAM.
- INDICATES A SLAB DEPRESSION. REFER TO DETAILS 1 ON DRAWING S0-0-6.
- 9/16" INDICATES SPAN DIRECTION OF 9/16" DEEP, 20 GAGE GALVANIZED STEEL ROOF DECK.
- XXXXX INDICATES OPENING IN WEB OF STEEL BEAM FOR MECH/ELECT/PLUMB/FIRE PROTECT/ETC COORDINATE WITH APPLICABLE DRAWING FOR OPENING SIZE. SEE TYPICAL DETAIL 5 AND 6 ON S0-0-6 FOR ADDITIONAL INFORMATION.
- 3" INDICATES SPAN DIRECTION OF 3" DEEP, 20 GAGE TYPE N, GALVANIZED STEEL ROOF DECK.
- STEEL BEAMS ARE EQUALLY SPACED BETWEEN COLUMN GRIDLINES, UNLESS NOTED OTHERWISE.

FIREPROOFING NOTES:

- STEEL COLUMNS, SHOWN ON THIS DRAWING, SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING UNLESS INDICATED AS A ROUND HSS COLUMN. EXPOSED TO VIEW PORTION OF ROUND HSS COLUMNS SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING. CONCEALED FROM VIEW PORTION OF ROUND HSS COLUMN SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
- BRACED FRAME SHOWN ALONG COLUMN GRID AA BETWEEN GRIDS 6 AND 11, SHALL RECEIVE 2-HOUR RATING BY CEMENTITIOUS FIREPROOFING. ALL OTHER EXPOSED TO VIEW BRACED FRAMES SHOWN ON THIS DRAWING, SHALL RECEIVE 2-HOUR RATING BY INTUMESCENT MASTIC FIREPROOFING.
- COORDINATE FIREPROOFING REQUIREMENTS WITH THE SPECIFICATIONS AND ARCHITECTURAL DRAWINGS.

BRACE FRAME KEY



FRAMING ELEVATION NOTES:

- TYPICAL TOP-OF-CONCRETE TOPPING ELEVATION = (8'-6") AT THE MEZZANINE FLOOR LEVELS, IN THE AREAS BOUNDED BY GRIDS (DD) - (EE) AND (G) - (H), AND GRIDS (DD) - (FF) AND (I) - (J).
- FRAMING ELEVATIONS ARE BASED ON TYPICAL BOTTOM-OF-PLANK ELEVATION = (8'-6"), SHOWN ON THE ARCHITECTURAL DRAWINGS.
- COORDINATE ALL ELEVATIONS WITH ARCHITECTURAL DRAWINGS.

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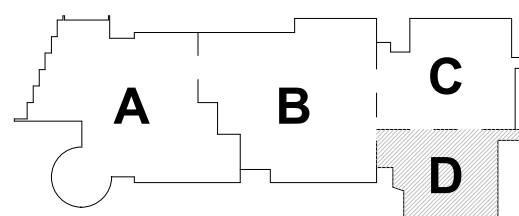
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03/31/2023	EARLY STRUCTURAL BID PACKAGE
REVISION LIST	
SS-S-1	4/14/2023 STRUCTURAL STEEL ADDENDUM 1

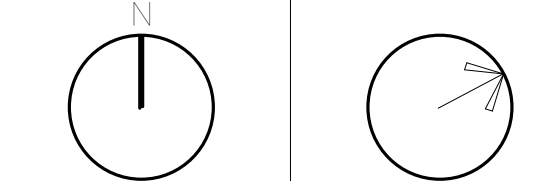
BID SET

August 28th, 2023



KEY PLAN

PROJECT NORTH
MAGNETIC NORTH



MEZZANINE
FLOOR FRAMING
PLAN - AREA D

Scale: 1/8" = 1'-0"

Job No.: 20202

Drawn By: EDG

Date: August 28th, 2023

S1-1-0MD

FRAMING NOTES:

- FOR GENERAL NOTES AND TYPICAL DETAILS SEE DRAWINGS S0-0-1, S0-0-2, S0-0-3, S0-0-4, S0-0-5, S0-0-6, S0-0-7 AND S0-0-8.
- REFER TO ARCHITECTURAL DRAWINGS FOR ELEVATIONS AND VERTICAL DIMENSIONS. FITCH ALL STEEL UNIFORMITY TO LOW POINTS AT THE COLUMNS AND BENT BEAMS AS SHOWN ON THE ARCHITECTURAL DRAWINGS.
- BF-1 ETC... INDICATES A BRACED BAY. REFER TO BRACED FRAME ELEVATIONS AND DETAILS ON DRAWINGS S4-0-1, S4-0-2, S4-0-3, S4-0-4, S4-0-5, S4-0-6, S4-0-7 AND S4-0-8 FOR ADDITIONAL INFORMATION.
- [XX] INDICATES THE NUMBER OF 3/4" DIAMETER x 4 1/4" LONG HEADED STUDS WELDED TO THE TOP FLANGE OF THE BEAM. SPACE STUDS EVENLY ALONG THE BEAM UNLESS NOTED OTHERWISE.
- INDICATES A MOMENT CONNECTION TO DEVELOP THE FULL CAPACITY OF THE MEMBER. REFER TO TYPICAL DETAILS 1, 2 AND 3 ON DRAWING S0-0-6.
- INDICATES A 5/16" FILLET WELD ALL AROUND (HSS BEAM TO HSS COLUMN) WHERE BEAM DIMENSIONS EXCEED COLUMN DIMENSIONS PROVIDE 1/2" THICK STEEL CAP PLATE TO ACHIEVE ALL AROUND WELD. REFER TO TYPICAL DETAIL 2 ON DRAWING S0-0-7.
- < X' > INDICATES UPWARD CAMBER AT THE MID-SPAN OF THE MEMBER.
- 5'-14" INDICATES SPAN DIRECTION OF 2" DEEP, 20 GAGE GALVANIZED COMPOSITE STEEL DECK WITH 3 1/4" LIGHT WEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 5 1/4". REINFORCE WITH 6x6 - W2.1W2.1 WWR.
- 1'-12" INDICATES SPAN DIRECTION OF 1 1/2" DEEP, 20 GAGE TYPE B, GALVANIZED STEEL ROOF DECK.
- 3" INDICATES SPAN DIRECTION OF 3" DEEP, 20 GAGE TYPE NA, GALVANIZED ACOUSTIC STEEL ROOF DECK.
- 4" INDICATES SPAN DIRECTION OF 1 1/2" DEEP, 20 GAGE GALVANIZED COMPOSITE STEEL DECK WITH 2 1/2" LIGHT WEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 4". REINFORCE WITH 6x6 - W2.1W2.1 WWR.
- FOR EXACT NUMBER, SIZE, AND LOCATION OF OPENING IN STEEL DECKING REFER TO MECHANICAL AND ARCHITECTURAL DRAWINGS. FOR FRAMING INFORMATION, REFER TO DETAILS 1, 2 AND 3 ON DRAWING S0-0-6.
- HATCHED AREA INDICATES LOCATION OF CONCRETE SLAB WITH 3" DEEP, 18 GAGE GALVANIZED COMPOSITE STEEL DECK WITH 4" NORMAL WEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 6". REINFORCE WITH 6x6 - W2.1W2.1 WWR. REFER TO TYPICAL DETAIL 5 ON DRAWING S0-0-7 FOR ADDITIONAL INFORMATION. USE 3/4" DIA x 5' LONG HEADED STUDS.
- INDICATES A ROOF DRAIN. REFER TO TYPICAL STRUCTURAL DETAILS 1 AND 2 ON DRAWING S0-0-8 AND DETAIL 1 ON DRAWING S0-0-8 FOR DECKING SUPPORT. REFER TO DETAIL 8 ON DRAWING S0-0-7. REFER TO PLUMBING AND ARCHITECTURAL DRAWINGS FOR OPENING SIZES AND LOCATIONS.
- CT INDICATES A COLUMN TERMINATES AT THIS LEVEL.
- WF INDICATES A BEND IN THE STEEL BEAM. REFER TO TYPICAL DETAIL 9 ON DRAWING S0-0-7.
- INDICATES A CMU WALL. REFER TO TYPICAL DETAIL 8 ON DRAWING S0-0-4 FOR REINFORCEMENT, DETAIL 6 ON DRAWING S0-0-5 FOR CONNECTION OF SHEAR WALLS TO BEAMS AND COLUMNS, AND DETAIL 7 ON DRAWING S0-0-5 FOR CONNECTIONS TO STEEL BEAMS AND CONCRETE SLABS AT THE TOP OF WALL FOR NON-STRUCTURAL WALLS. REFER TO RELEVANT SECTIONS FOR CONNECTIONS OF SHEAR WALLS TO THE STRUCTURE.
- 8" x 2" PC PLANK INDICATES SPAN OF 8" DEEP PRESTRESSED, PRECAST CONCRETE HOLLOW CORE PLANK WITH A MINIMUM 2" OF NORMAL WEIGHT CONCRETE TOPPING. PLANK AND CONCRETE TOPPING SLAB TO BE DESIGNED TO SUPPORT A MINIMUM LIVE LOAD CAPACITY OF 150 PSF. PRECAST CONCRETE PLANK TO BE DESIGNED FOR MINIMUM OF 2 HOUR FIRE RATING.
- 10" x 2" PC PLANK INDICATES SPAN OF 10" DEEP PRESTRESSED, PRECAST CONCRETE HOLLOW CORE PLANK WITH A MINIMUM 2" OF NORMAL WEIGHT CONCRETE TOPPING. PLANK AND CONCRETE TOPPING SLAB TO BE DESIGNED TO SUPPORT A MINIMUM LIVE LOAD CAPACITY OF 150 PSF. PRECAST CONCRETE PLANK TO BE DESIGNED FOR MINIMUM OF 2 HOUR FIRE RATING.
- FOR DIMENSIONS AND ELEVATIONS NOT GIVEN REFER TO ARCHITECTURAL DRAWINGS.
- WF INDICATES A SPLICE CONNECTION ALONG A CONTINUOUS STEEL BEAM. DESIGN SPLICE CONNECTION FOR FULL CAPACITY OF STEEL BEAM.
- INDICATES A SLAB DEPRESSION. REFER TO DETAILS 1 ON DRAWING S0-0-6.
- 9/16" INDICATES SPAN DIRECTION OF 9/16" DEEP, 20 GAGE GALVANIZED STEEL ROOF DECK.
- XX'XXX" INDICATES OPENING IN WEB OF STEEL BEAM FOR MECH/ELECT/PLUMB/FIRE PROTECT/ ETC COORDINATE WITH APPLICABLE DRAWING FOR OPENING SIZE. SEE TYPICAL DETAIL 5 AND 6 ON S0-0-6 FOR ADDITIONAL INFORMATION.
- 3" INDICATES SPAN DIRECTION OF 3" DEEP, 20 GAGE TYPE N, GALVANIZED STEEL ROOF DECK.
- STEEL BEAMS ARE EQUALLY SPACED BETWEEN COLUMN GRIDLINES, UNLESS NOTED OTHERWISE.

FRAMING ELEVATION NOTES:

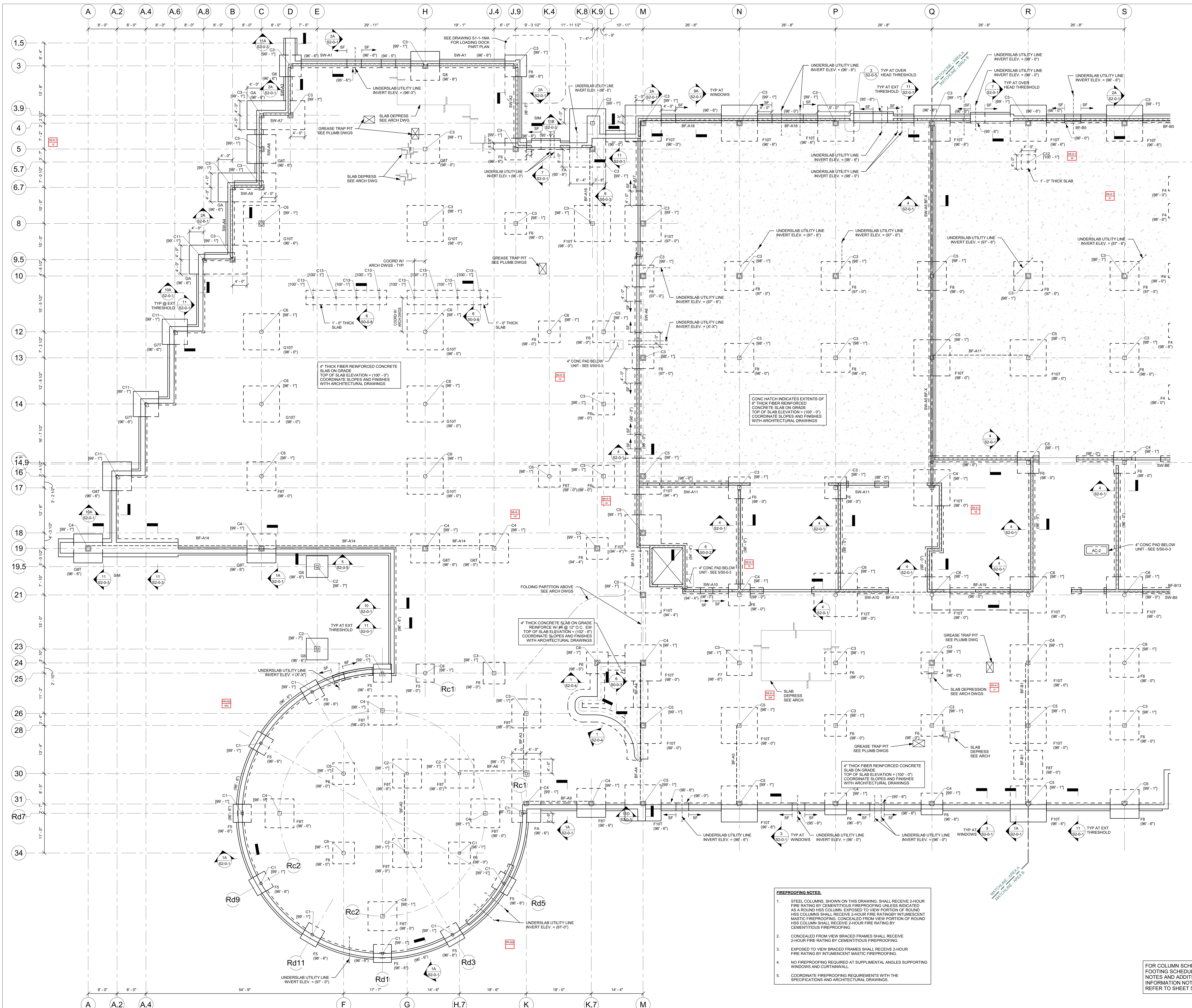
- TYPICAL TOP-OF-CONCRETE TOPPING ELEVATION = (89' - 6") AT THE MEZZANINE FLOOR LEVELS. IN THE AREA BOUNDED BY GRID (CC) - (EE) AND (22) - (35).
- FRAMING ELEVATIONS ARE BASED ON TYPICAL BOTTOM-OF-PLANK ELEVATION = (88' - 6"), SHOWN ON THE ARCHITECTURAL DRAWINGS.
- COORDINATE ALL ELEVATIONS WITH ARCHITECTURAL DRAWINGS.

FIREPROOFING NOTES:

- STEEL COLUMNS, SHOWN ON THIS DRAWING, SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING UNLESS INDICATED AS A ROUND HSS COLUMN. EXPOSED TO VIEW PORTION OF ROUND HSS COLUMNS SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING. CONCEALED FROM VIEW PORTION OF ROUND HSS COLUMN SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
- EXPOSED TO VIEW BRACED FRAMES SHOWN ON THIS DRAWING, SHALL RECEIVE 2-HOUR RATING BY INTUMESCENT MASTIC FIREPROOFING.
- COORDINATE FIREPROOFING REQUIREMENTS WITH THE SPECIFICATIONS AND ARCHITECTURAL DRAWINGS.

BRACE FRAME KEY

- 0' TYP
- WF BF-X
- INDICATES A BRACE FRAME ABOVE AND BELOW LEVEL.
- 0' TYP
- BF-X WF
- INDICATES A BRACE FRAME ABOVE LEVEL.
- 0' TYP
- WF BF-X
- INDICATES A BRACE FRAME BELOW LEVEL.



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03/31/2023	EARLY STRUCTURAL BID PACKAGE
REVISION LIST	
SS-S-1	4/14/2023 STRUCTURAL STEEL ADDENDUM 1
PR-002	6/29/2023 MISCELLANEOUS STRUCTURAL REVISIONS

BID SET

August 28th, 2023

KEY PLAN

PROJECT NORTH

MAGNETIC NORTH

FIRST FLOOR FOUNDATION PLAN - AREA A

Scale: 1/8" = 1'-0"
Job No.: 20202
Drawn By: EDG
Date: August 28th, 2023

S1-1-1A

FIREPROOFING NOTES:

- STEEL COLUMNS, SHOWN ON THIS DRAWING, SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING UNLESS INDICATED AS A ROUND HSS COLUMN. EXPOSED TO VIEW PORTION OF ROUND HSS COLUMNS SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING. CONCEALED FROM VIEW PORTION OF ROUND HSS COLUMNS SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
- CONCEALED FROM VIEW BRACED FRAMES SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
- EXPOSED TO VIEW BRACED FRAMES SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING.
- NO FIREPROOFING REQUIRED AT SUPPLEMENTAL ANGLES SUPPORTING WINDOWS AND CURTAINWALL.
- COORDINATE FIREPROOFING REQUIREMENTS WITH THE SPECIFICATIONS AND ARCHITECTURAL DRAWINGS.

FOR COLUMN SCHEDULE, FOOTING SCHEDULE, PLAN NOTES AND ADDITIONAL INFORMATION NOT SHOWN, REFER TO SHEET S1-1-1B

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- FOUNDATION NOTES:**
- 1.) REFER TO GRADING DRAWINGS FOR PLAN AND GRADE ELEVATIONS. THE STRUCTURAL DRAWINGS USE A DATUM OF 100'-0" AT THE FIRST FLOOR LEVEL, EQUAL TO 163.50' ON THE SITE GRADING PLANS.
 - 2.) FOR GENERAL NOTES AND TYPICAL DETAILS SEE DRAWINGS S0-0.1, S0-0.2, S0-0.3, S0-0.4, S0-0.5, S0-0.6, S0-0.7 AND S0-0.8.
 - 3.) F3 ETC., INDICATES A FOOTING TYPE, FOR SIZE OF FOOTING AND REINFORCEMENT SEE SCHEDULE ON THIS DRAWING.
 - 4.) TOP OF FOOTING ELEVATION TO BE 3'-6" MINIMUM BELOW LOWEST ADJACENT FINISHED GRADE AT EXTERIOR CONDITIONS AND 2'-0" BELOW TOP OF CONCRETE SLAB AT INTERIOR CONDITIONS. ALL OTHER TOP OF FOOTING ELEVATIONS ARE DENOTED AS THUS (X'-Y) COMPUTED FROM A DATUM ELEVATION OF 100'-0" ON PLANS. CONTRACTOR TO COORDINATE AND VERIFY ALL TOP OF FOOTING ELEVATIONS WITH UNDERGROUND PLUMBING SUB-CONTRACTORS FIELD LAYOUT.
 - 5.) ALL FOOTING ELEVATIONS NOTED ON PLAN ARE SHOWN ONLY TO ASSIST IN COORDINATION. ALL FOOTING ELEVATIONS MUST BE COORDINATED WITH STRUCTURAL REQUIREMENTS, TYPICAL DETAILS, ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS.
 - 6.) ALL FOOTINGS TO BE CENTERED UNDER COLUMNS UNLESS NOTED OTHERWISE.
 - 7.) SF INDICATES A STEPPED FOOTING REFER TO DETAIL 1 ON DRAWING S0-0.2.
 - 8.) C1 ETC., INDICATES A COLUMN TYPE, FOR SIZE OF COLUMNS AND BASE PLATES SEE SCHEDULE ON THIS DRAWING.
 - 9.) BOTTOM OF BASE PLATE ELEVATION TO BE 1'-11" MINIMUM BELOW TOP OF CONCRETE SLAB AT INTERIOR CONDITIONS, AND 0'-11" BELOW TOP OF CONCRETE SLAB AT EXTERIOR CONDITIONS. UNLESS NOTED OTHERWISE AS (XX'-XX") REFER TO ARCHITECTURAL DRAWINGS FOR BRICK SHELF ELEVATIONS.
 - 10.) FOR UNDER SLAB DRAINAGE AND WALL DRAINS, COORDINATE WITH ARCHITECTURAL, STRUCTURAL, CIVIL AND PLUMBING DRAWINGS.
 - 11.) INDICATES A DEPRESSIONED SLAB ON GRADE. REFER TO DETAILS 9 AND 10 ON DRAWING S0-0.2 COORDINATE ALL SLAB DEPRESSIONS WITH REQUIREMENTS ON ARCHITECTURAL DRAWINGS.
 - 12.) FOR TYPICAL EXTERIOR DOOR DETAIL REFER TO DETAIL 6 ON DRAWING S0-0.8 AND RELEVANT SECTIONS.
 - 13.) BF-1 ETC., INDICATES A BRACED BAY, REFER TO BRACED FRAME ELEVATIONS AND DETAILS 5 ON DRAWING S4-0.5, S4-0.6, S4-0.7, S4-0.8, AND S4-0.9 FOR ADDITIONAL INFORMATION.
 - 14.) INDICATES A CMU WALL. REFER TO TYPICAL DETAIL 8 ON DRAWING S0-0.4 FOR REINFORCEMENT, DETAIL 6 ON DRAWING S0-0.5 FOR CONNECTION OF SHEAR WALLS TO BEAMS AND COLUMNS AND DETAIL 7 ON DRAWING S0-0.5 FOR CONNECTIONS TO STEEL BEAMS AND CONCRETE SLABS AT THE TOP OF WALL FOR NON-STRUCTURAL WALLS. REFER TO RELEVANT SECTIONS FOR CONNECTIONS OF SHEAR WALLS TO THE STRUCTURE.
 - 15.) FOR DIMENSIONS AND ELEVATIONS NOT GIVEN REFER TO ARCHITECTURAL DRAWINGS.
 - 16.) INDICATES CONCRETE PIER REFER TO TYPICAL DETAIL 5 ON DRAWING S0-0.2.
 - 17.) INDICATES UNDERGROUND UTILITY LINES PLUMBING THROUGH CONCRETE PIER. SEE FOUNDATION NOTE ABOVE AND REFER TO DETAILS ON DRAWING S4-0.8 AND S4-0.9 FOR ADDITIONAL INFORMATION.
 - 18.) CONCRETE PIER REINFORCING PER DETAIL 5 ON DRAWING S0-0.2 IS TO BE PROVIDED FOR ALL CONCRETE WALLS SUPPORTING COLUMNS. HORIZONTAL WALL REINFORCING MUST REMAIN CONTINUOUS.

COLUMN SCHEDULE *

MARK	SIZE	BASE PLATE SIZE
C1	HSS8x8x3/8	1' x 16' x 1'-4"
C2	HSS8x8x1/2	1' x 16' x 1'-4"
C3	HSS12x12x3/8	1' x 20' x 1'-8"
C4	HSS12x12x1/2	1' x 20' x 1'-8"
C5	HSS12x12x5/8	1' x 20' x 1'-8"
C6	HSS12.75x10.500	1' x 20' x 1'-8"
C7	HSS20x12x1/2	1 1/2' x 20' x 2'-4"
C8	HSS8x4x3/8	1' x 16' x 1'-0"
C9	HSS16x10.500	1 1/2' x 24' x 2'-0"
C10	HSS12x6x1/2	1 1/2' x 20' x 1'-2"
C11	HSS10x50.500	1' x 18' x 1'-0"
C12	HSS6x3/8	1' x 14' x 1'-2"
C13	HSS6x4x3/8	1' x 12' x 1'-2"

* BASE PLATE LENGTH AND WIDTH SPECIFIED IN SCHEDULE IS THE MINIMUM SIZE FOR A COLUMN THAT IS PART OF A BRACED FRAME. SEE FOUNDATION NOTE ABOVE AND REFER TO DETAILS ON DRAWING S4-0.8 AND S4-0.9 FOR ADDITIONAL INFORMATION.

* PROVIDE 4 - 1" DIA F1554-5551 ANCHOR RODS TYPICALLY REFER TO DETAILS ON DRAWING S4-0.8 AND S4-0.9 FOR ADDITIONAL ANCHOR RODS FOR COLUMN RECEIVING BRACING.

FOOTING SCHEDULE F SERIES

MARK	SIZE	REINFORCEMENT
F4	4'-0" x 4'-0" x 2'-0"	6 - #5 BOT EA WAY
F5	5'-0" x 5'-0" x 2'-0"	7 - #5 BOT EA WAY
F6	6'-0" x 6'-0" x 2'-0"	8 - #6 BOT EA WAY
F7	7'-0" x 7'-0" x 2'-0"	9 - #6 BOT EA WAY
F8	8'-0" x 8'-0" x 3'-0"	10 - #8 BOT EA WAY
F9	9'-0" x 9'-0" x 3'-0"	11 - #9 BOT EA WAY
F10	10'-0" x 10'-0" x 3'-0"	12 - #9 BOT EA WAY
F11	11'-0" x 11'-0" x 3'-0"	13 - #10 BOT EA WAY
F12	12'-0" x 12'-0" x 4'-0"	14 - #10 BOT EA WAY
FA	SEE PLAN x 2'-0"	#8 @ 12" OC TOP AND BOT EA WAY

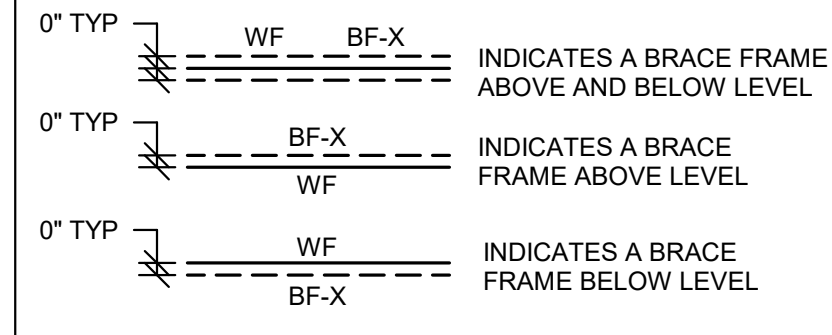
T INDICATES TOP REINFORCING TO MATCH BOTTOM REINFORCING

FOOTING SCHEDULE Q SERIES

MARK	SIZE	REINFORCEMENT
G4	4'-0" x 4'-0" x 2'-0"	5 - #5 BOT EA WAY
G5	5'-0" x 5'-0" x 2'-0"	6 - #5 BOT EA WAY
G6	6'-0" x 6'-0" x 2'-0"	7 - #6 BOT EA WAY
G7	7'-0" x 7'-0" x 2'-0"	8 - #6 BOT EA WAY
G8	8'-0" x 8'-0" x 2'-0"	9 - #6 BOT EA WAY
G9	9'-0" x 9'-0" x 2'-0"	10 - #7 BOT EA WAY
G10	10'-0" x 10'-0" x 2'-0"	11 - #7 BOT EA WAY
G11	11'-0" x 11'-0" x 3'-0"	12 - #8 BOT EA WAY
G12	12'-0" x 12'-0" x 3'-0"	13 - #8 BOT EA WAY
G13	13'-0" x 13'-0" x 3'-0"	14 - #9 BOT EA WAY
G14	14'-0" x 14'-0" x 3'-0"	15 - #9 BOT EA WAY
G15	15'-0" x 15'-0" x 3'-0"	16 - #9 BOT EA WAY
GA	SEE PLAN x 2'-0"	#8 @ 12" OC TOP AND BOT EA WAY

T INDICATES TOP REINFORCING TO MATCH BOTTOM REINFORCING

BRACE FRAME KEY

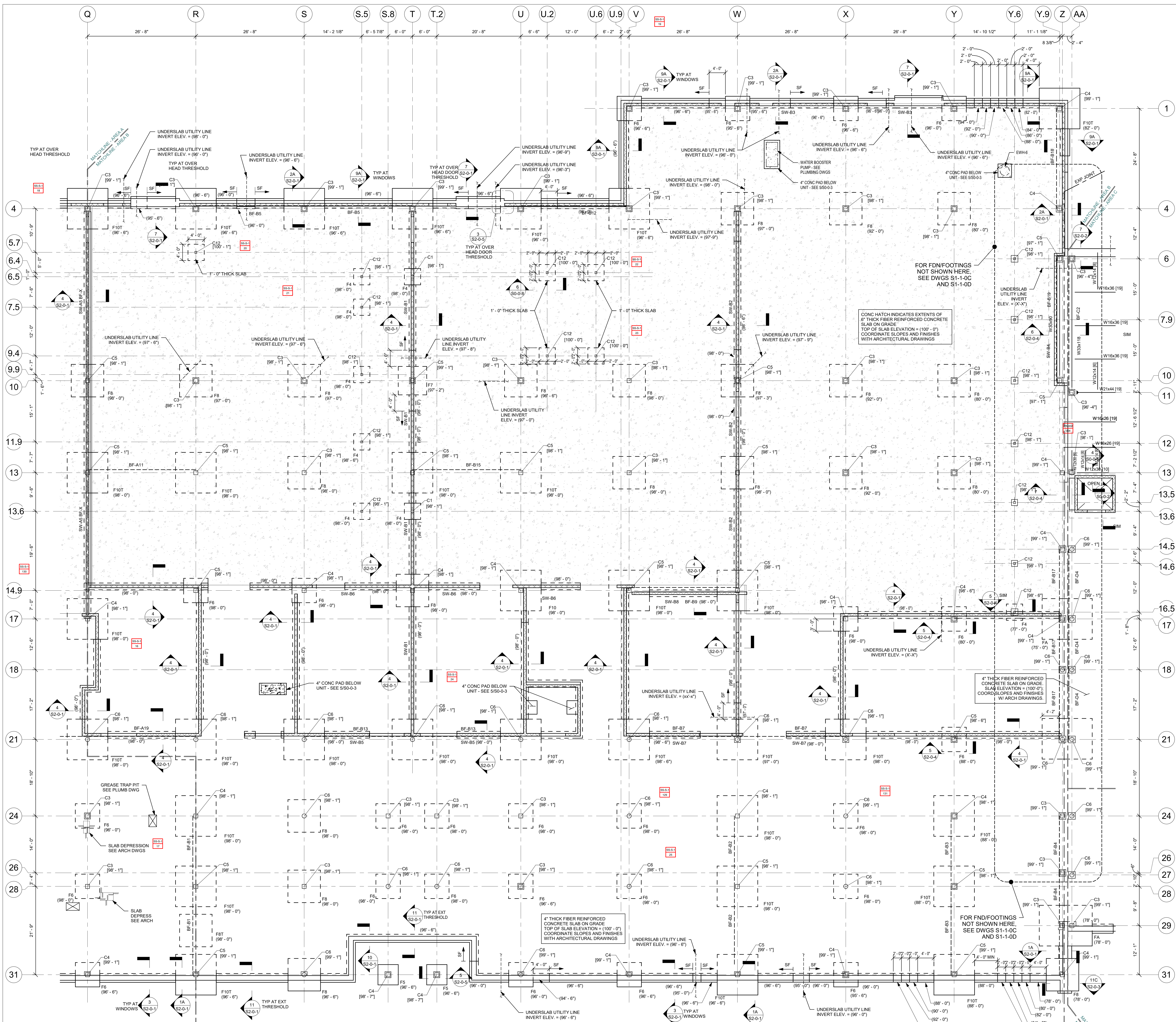


FIREPROOFING NOTES:

1. STEEL COLUMNS, SHOWN ON THIS DRAWING, SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING UNLESS INDICATED AS A ROUND HSS COLUMN. EXPOSED TO VIEW PORTION OF ROUND HSS COLUMNS SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING. CONCEALED FROM VIEW PORTION OF ROUND HSS COLUMNS SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
2. NO FIREPROOFING REQUIRED AT COLUMNS AT GRIDS Y10-6.5, Y11-6.5, Y10-3.2, AND Y11-3.2 OR AT THE HSS TUBES FRAMING INTO THEM.
3. STEEL COLUMNS SUPPORTING THE PLYWOOD MEZZANINES ALONG COLUMN GRIDS S.5 AND T.2 AND GRIDS Y.8 SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING.
4. CONCEALED FROM VIEW BRACED FRAMES SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
5. EXPOSED TO VIEW BRACED FRAMES SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING.

FIREPROOFING NOTES (cont.):

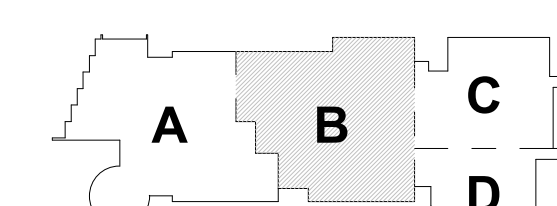
6. ALONG GRIDS Z AND AA ROUND COLUMNS TRANSITIONING FROM CEMENTITIOUS FIREPROOFING (FIRST FLOOR) TO INTUMESCENT MASTIC FIREPROOFING, TRANSITION SHALL OCCUR AT ELEVATION 118'-0".
7. NO FIREPROOFING REQUIRED AT SUPPLEMENTAL ANGLES SUPPORTING WINDOWS AND CURTAIN WALL.
8. COORDINATE FIREPROOFING REQUIREMENTS WITH THE SPECIFICATIONS AND ARCHITECTURAL DRAWINGS.



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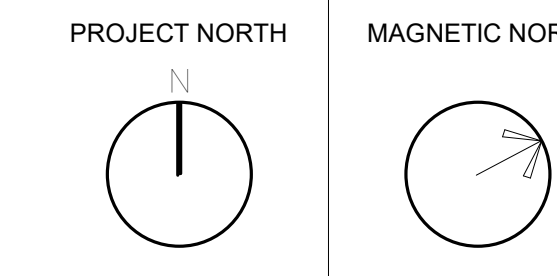
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August 28th, 2023



KEY PLAN

PROJECT NORTH MAGNETIC NORTH



FIRST FLOOR FOUNDATION PLAN - AREA B

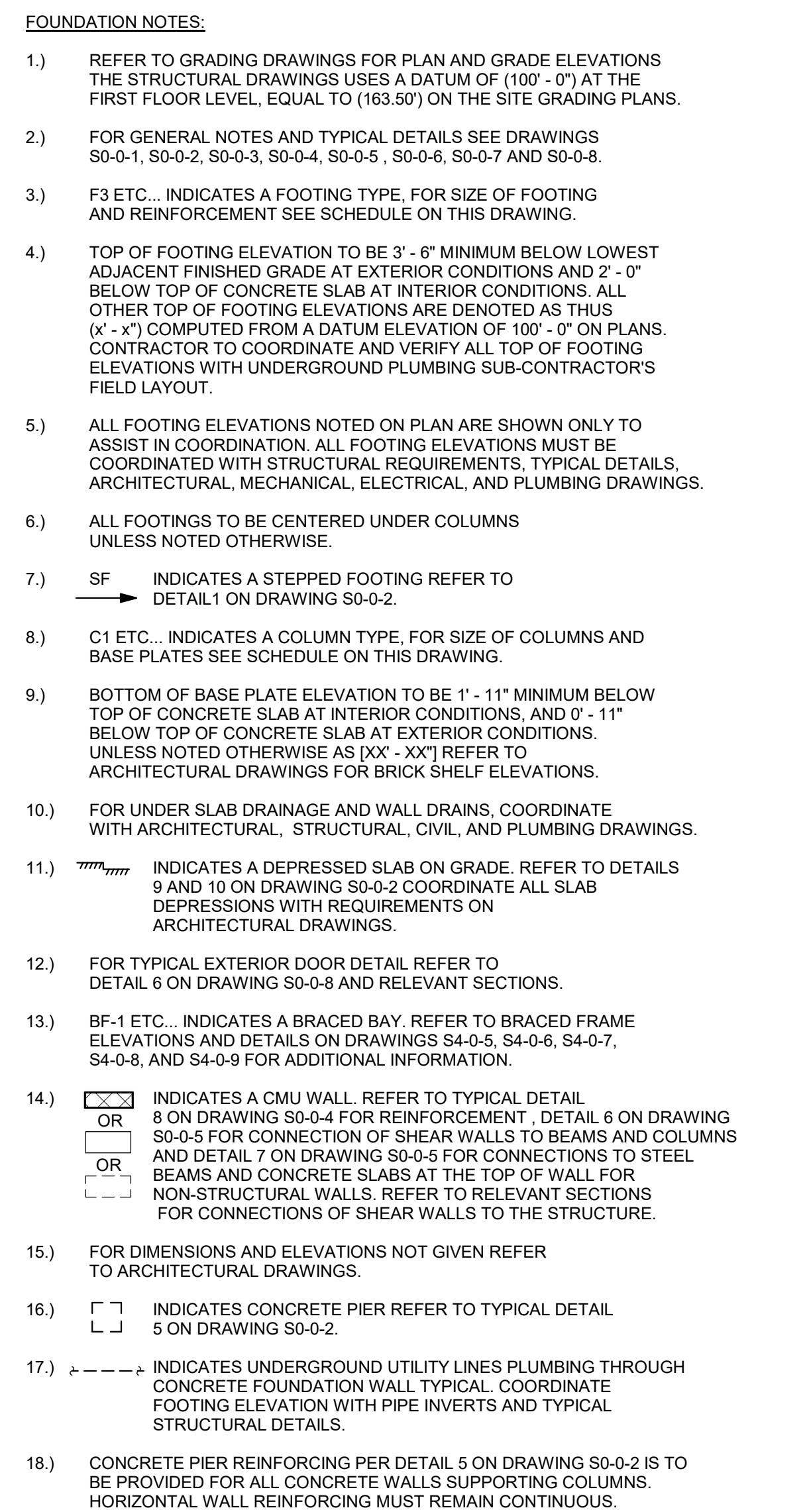
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Job No.: 20202

Drawn By: EDG

Date: August 28th, 2023

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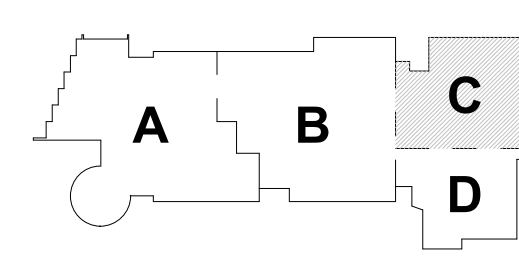
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SS-S-2	4/21/2023 STRUCTURAL STEEL ADDENDUM 2
PR-002	6/29/2023 MISCELLANEOUS STRUCTURAL REVISIONS

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August 28th, 2023



KEY PLAN

PROJECT NORTH


MAGNETIC NORTH


FIRST FLOOR
FOUNDATION
PLAN - AREA C

Scale: 1/8" = 1'-0"
Job No.: 20202
Drawn By: EDG
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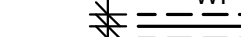
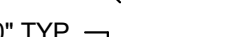
S1-1-1C

FOOTING SCHEDULE F SERIES		
MARK	SIZE	REINFORCEMENT
F4	4' - 0" x 4' - 0" x 2' - 0"	6 - #5 BOT EA WAY
F5	5' - 0" x 5' - 0" x 2' - 0"	7 - #5 BOT EA WAY
F6	6' - 0" x 6' - 0" x 2' - 6"	8 - #6 BOT EA WAY
F7	7' - 0" x 7' - 0" x 2' - 6"	9 - #6 BOT EA WAY
F8	8' - 0" x 8' - 0" x 3' - 0"	10 - #8 BOT EA WAY
F9	9' - 0" x 9' - 0" x 3' - 0"	11 - #9 BOT EA WAY
F10	10' - 0" x 10' - 0" x 3' - 6"	12 - #9 BOT EA WAY
F11	11' - 0" x 11' - 0" x 3' - 6"	13 - #10 BOT EA WAY
F12	12' - 0" x 12' - 0" x 4' - 0"	14 - #10 BOT EA WAY
FA	SEE PLAN X 2 - 0"	#8 @ 12" OC TOP AND BOT EA WAY

T INDICATES TOP REINFORCING TO MATCH BOTTOM REINFORCING

FIREPROOFING NOTES:		BEAM DESIGN SPEC. CONNECTION FOR FULL CAPACITY OF STEEL BEAM.	
1.	STEEL COLUMNS SHOWN ON THIS DRAWING SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING UNLESS INDICATED AS A ROUND HSS COLUMN OR BUILT-UP COLUMN ALONG COLUMN GRID B6.1. EXPOSED TO VIEW PORTION OF ROUND HSS COLUMNS SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING. CONCEALED FROM VIEW PORTION OF ROUND HSS COLUMN SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING. BUILT-UP COLUMNS ALONG GRID B6.1 SHALL RECEIVE 2-HOUR RATING BY INTUMESCENT MASTIC FIREPROOFING.	22.)	 INDICATES A SLAB DEPRESSION. REFER TO DETAILS 1 ON DRAWING S0-0-5.
2.	STEEL BEAMS AND KICKERS SHOWN ON THIS DRAWING, SUPPORTING UPPER LEVEL FLOORS SHALL RECEIVE 2-HOUR RATING BY CEMENTITIOUS FIREPROOFING.	23.)	 INDICATES SPAN DIRECTION OF 9/16" DEEP, 20 GAGE GALVANIZED STEEL ROOF DECK.
3.	STEEL BEAMS AND KICKERS SHOWN ON THIS DRAWING, SUPPORTING ROOF SHALL RECEIVE 1-HOUR RATINGS BY CEMENTITIOUS FIREPROOFING. ROOF DECK TO RECEIVE 1-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.	24.)	 INDICATES OPENING IN WEB OF STEEL BEAM FOR MECH/ELECT/PLUMB/FIRE PROTECT/ETC COORDINATE WITH APPLICABLE DRAWING FOR OPENING SIZE. SEE TYPICAL DETAIL 5 AND 6 ON S0-0-5 FOR ADDITIONAL INFORMATION.
4.	CONCEALED FROM VIEW BRACED FRAMES SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.	25.)	 INDICATES SPAN DIRECTION OF 3" DEEP, 20 GAGE TYPE N, GALVANIZED STEEL ROOF DECK.
5.	EXPOSED TO VIEW BRACED FRAMES SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING.	26.)	STEEL BEAMS ARE EQUALLY SPACED BETWEEN COLUMN GRID LINES, UNLESS NOTED OTHERWISE.
6.	AT STAIRS 4, 6, AND 7, ALL EXPOSED TO VIEW BEAMS AND HSS TUBES SUPPORTING STAIR LANDINGS AND STRINGERS SHALL RECEIVE 2-HOUR RATING BY INTUMESCENT MASTIC FIREPROOFING. NO FIREPROOFING REQUIRED AT SUPPLEMENTAL HSS TUBES SUPPORTING ONLY CURTAINWALL.		
7.	ALONG GROSS 2 AND A ROUND COLUMNS TRANSITIONING FROM CEMENTITIOUS FIREPROOFING (FIRST FLOOR) TO INTUMESCENT MASTIC FIREPROOFING, TRANSITION SHALL OCCUR AT BEAM OR COLUMN.		
8.	NO FIREPROOFING REQUIRED AT SUPPLEMENTAL HSS TUBES LOCATED AT THE VOCATIONAL OVERHEAD DOORS.		
9.	NO FIREPROOFING REQUIRED AT SUPPLEMENTAL HSS TUBES LOCATED AT STOREFRONT, CURTAINWALL, AND FIBERGLASS PANEL HEADS.		
10.	COORDINATE FIREPROOFING REQUIREMENTS WITH THE SPECIFICATIONS AND ARCHITECTURAL DRAWINGS.		

BRACE FRAME KEY

0" TYP		INDICATES A BRACE FRAME ABOVE AND BELOW THE BEAM
0" TYP		INDICATES A BRACE FRAME ABOVE THE BEAM
0" TYP		INDICATES A BRACE FRAME BELOW THE BEAM

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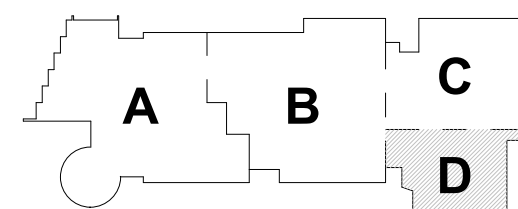
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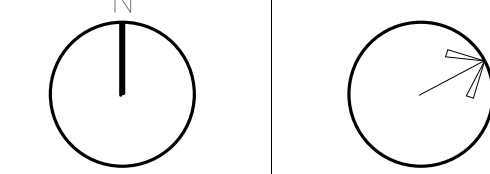
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KEY PLAN

PROJECT NORTH MAGNETIC NORTH



FIRST FLOOR FOUNDATION PLAN - AREA D

Scale: 1/8" = 1'-0"

Job No.: 20202

Drawn By: EDG

Date: August 28th, 2023

S1-1-1D

FRAMING NOTES:

- FOR GENERAL NOTES AND TYPICAL DETAILS SEE DRAWINGS S0-0-1, S0-0-2, S0-0-3, S0-0-4, S0-0-5, S0-0-6, S0-0-7 AND S0-0-8.
- REFER TO ARCHITECTURAL DRAWINGS FOR ELEVATIONS AND VERTICAL DIMENSIONS. PITCH ALL STEEL UNIFORMLY TO LOW POINTS AT THE COLUMNS AND BENT BEAMS AS SHOWN ON THE ARCHITECTURAL DRAWINGS.
- BF-1 ETC. INDICATES A BRACED BAY. REFER TO BRACED FRAME ELEVATIONS AND DETAILS ON DRAWINGS S4-0-1, S4-0-2, S4-0-3, S4-0-4, S4-0-5, S4-0-6, S4-0-7 AND S4-0-8 FOR ADDITIONAL INFORMATION.
- [XX] INDICATES THE NUMBER OF 3/4" DIAMETER X 4 1/4" LONG HEADED STUDS WELDED TO THE TOP FLANGE OF THE BEAM. SPACE STUDS EVENLY ALONG THE BEAM UNLESS NOTED OTHERWISE.
- INDICATES A MOMENT CONNECTION TO DEVELOP THE FULL CAPACITY OF THE MEMBER, REFER TO TYPICAL DETAILS 7, 8 AND 9 ON DRAWING S0-0-6.
- INDICATES A 5/16" FILLET WELD ALL AROUND (HSS BEAM TO HSS COLUMN) WHERE BEAM DIMENSIONS EXCEED COLUMN DIMENSIONS PROVIDE 1/2" THICK STEEL CAP PLATE TO ACHIEVE ALL AROUND WELD. REFER TO TYPICAL DETAIL 2 ON DRAWING S0-0-7.
- INDICATES UPWARD CAMBER AT THE MID-SPAN OF THE MEMBER.
- INDICATES SPAN DIRECTION OF 2" DEEP, 20 GAGE GALVANIZED COMPOSITE STEEL DECK WITH 3 1/4" LIGHT WEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 5 1/4". REINFORCE WITH 6x6 - W2 1xW2 1 WWR.
- INDICATES SPAN DIRECTION OF 1 1/2" DEEP, 20 GAGE TYPE B, GALVANIZED STEEL ROOF DECK.
- INDICATES SPAN DIRECTION OF 3" DEEP, 20 GAGE TYPE NA, GALVANIZED ACUSTIC STEEL ROOF DECK.
- INDICATES SPAN DIRECTION OF 1 1/2" DEEP, 20 GAGE GALVANIZED COMPOSITE STEEL DECK WITH 2 1/2" LIGHT WEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 4". REINFORCE WITH 6x6 - W2 1xW2 1 WWR.
- FOR EXACT NUMBER, SIZE, AND LOCATION OF OPENING IN STEEL DECKING REFERS TO MECHANICAL AND ARCHITECTURAL DRAWINGS FOR FRAMING INFORMATION. REFER TO DETAILS 1, 2 AND 3 ON DRAWING S0-0-8.
- HATCHED AREA INDICATES LOCATION OF CONCRETE SLAB WITH 3" DEEP, 16 GAGE GALVANIZED COMPOSITE STEEL DECK WITH 4" NORMAL WEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 6". REINFORCE WITH 6x6 - W2 1xW2 1 WWR. REFER TO TYPICAL DETAIL 5 ON DRAWING S0-0-7 FOR ADDITIONAL INFORMATION. USE 3/4" DIA X 7" LONG HEADED STUDS.
- INDICATES A ROOF DRAIN. REFER TO TYPICAL STRUCTURAL DETAILS 1 AND 2 ON DRAWING S0-0-8 AND DETAIL 1 ON DRAWING S0-0-8 FOR DECKING SUPPORT. REFER TO DETAIL 5 ON DRAWING S0-0-7 FOR ADDITIONAL INFORMATION.
- INDICATES A COLUMN TERMINATES AT THIS LEVEL.
- INDICATES A BEND IN THE STEEL BEAM. REFER TO TYPICAL DETAIL 9 ON DRAWING S0-0-7.
- INDICATES A CMU WALL. REFER TO TYPICAL DETAIL 8 ON DRAWING S0-0-4 FOR REINFORCEMENT. DETAIL 6 ON DRAWING S0-0-4 FOR CONNECTION OF SHEAR WALLS TO BEAMS AND COLUMNS, AND DETAIL 7 ON DRAWING S0-0-5 FOR CONNECTIONS TO STEEL BEAMS AND CONCRETE SLABS AT THE TOP OF WALL FOR NON-STRUCTURAL WALLS. REFER TO RELEVANT SECTIONS FOR CONNECTIONS OF SHEAR WALLS TO THE STRUCTURE.
- PC PLANK INDICATES SPAN OF 8" DEEP PRESTRESSED, PRECAST CONCRETE HOLLOW CORE PLANK WITH A MINIMUM 2" OF NORMAL WEIGHT CONCRETE TOPPING. PLANK AND CONCRETE TOPPING SLAB TO BE DESIGNED TO SUPPORT A MINIMUM LIVE LOAD CAPACITY OF 150 PSF. PRECAST CONCRETE PLANK TO BE DESIGNED FOR MINIMUM OF 2 HOUR FIRE RATING.
- PC PLANK INDICATES SPAN OF 10" DEEP PRESTRESSED, PRECAST CONCRETE HOLLOW CORE PLANK WITH A MINIMUM 2" OF NORMAL WEIGHT CONCRETE TOPPING. PLANK AND CONCRETE TOPPING SLAB TO BE DESIGNED TO SUPPORT A MINIMUM LIVE LOAD CAPACITY OF 150 PSF. PRECAST CONCRETE PLANK TO BE DESIGNED FOR MINIMUM OF 2 HOUR FIRE RATING.
- FOR DIMENSIONS AND ELEVATIONS NOT GIVEN REFER TO ARCHITECTURAL DRAWINGS.
- INDICATES A SPLICE CONNECTION ALONG A CONTINUOUS STEEL BEAM.
- INDICATES A SLAB DEPRESSION. REFER TO DETAILS 1 ON DRAWING S0-0-6.
- INDICATES SPAN DIRECTION OF 9/16" DEEP, 20 GAGE GALVANIZED STEEL ROOF DECK.
- INDICATES OPENING IN WEB OF STEEL BEAM FOR MECH/ELECT/PLUMB/ FIRE PROTECT/ ETC COORDINATE WITH APPLICABLE DRAWING FOR OPENING SIZE. SEE TYPICAL DETAIL 5 AND 6 ON S0-0-6 FOR ADDITIONAL INFORMATION.
- INDICATES SPAN DIRECTION OF 3" DEEP, 20 GAGE TYPE N, GALVANIZED STEEL ROOF DECK.
- STEEL BEAMS ARE EQUALLY SPACED BETWEEN COLUMN GRIDLINES UNLESS NOTED OTHERWISE.

COLUMN SCHEDULE *		
MARK	SIZE	BASE PLATE SIZE
C1	HSS8x8x3/8	1" x 16" x 1' - 4"
C2	HSS8x8x1/2	1" x 16" x 1' - 4"
C3	HSS12x12x3/8	1" x 20" x 1' - 8"
C4	HSS12x12x1/2	1" x 20" x 1' - 8"
C5	HSS12x12x5/8	1" x 20" x 1' - 8"
C6	HSS12 75x500	1" x 20" x 1' - 8"
C7	HSS20x12x1/2	1 1/2" x 20" x 2' - 4"
C8	HSS8x4x3/8	1" x 16" x 1' - 0"
C9	HSS16x500	1 1/2" x 24" x 2' - 0"
C10	HSS12x6x1/2	1 1/2" x 20" x 1' - 2"
C11	HSS10x500	1" x 18" x 1' - 8"
C12	HSS6x6x3/8	1" x 14" x 1' - 2"
C13	HSS6x4x3/8	1" x 12" x 1' - 2"

* BASE PLATE LENGTH AND WIDTH SPECIFIED IN SCHEDULE IS THE MINIMUM SIZE FOR A COLUMN THAT IS PART OF A BRACED FRAME. SEE FOUNDATION NOTE ABOVE AND REFER TO DETAILS ON DRAWING S4-0-8 AND S4-0-9 FOR ADDITIONAL INFORMATION.

* PROVIDE 4 - 1" DIA #1654-6551 ANCHOR RODS TYPICALLY REFER TO DETAILS ON DRAWING S4-0-8 AND S4-0-9 FOR ADDITIONAL ANCHOR RODS FOR COLUMN RECEIVING BRACING.

FOOTING SCHEDULE F SERIES		
MARK	SIZE	REINFORCEMENT
F4	4' - 0" x 4' - 0" x 2' - 0"	6 - #5 BOT EA WAY
F5	5' - 0" x 5' - 0" x 2' - 0"	7 - #5 BOT EA WAY
F6	6' - 0" x 6' - 0" x 2' - 0"	8 - #6 BOT EA WAY
F7	7' - 0" x 7' - 0" x 2' - 0"	9 - #6 BOT EA WAY
F8	8' - 0" x 8' - 0" x 3' - 0"	10 - #8 BOT EA WAY
F9	9' - 0" x 9' - 0" x 3' - 0"	11 - #9 BOT EA WAY
F10	10' - 0" x 10' - 0" x 3' - 0"	12 - #9 BOT EA WAY
F11	11' - 0" x 11' - 0" x 3' - 0"	13 - #10 BOT EA WAY
F12	12' - 0" x 12' - 0" x 3' - 0"	14 - #10 BOT EA WAY
FA	SEE PLAN x 2' - 0"	#8 @ 12" OC TOP AND BOT EA WAY

BRACE FRAME KEY		
0" TYP	WF	BF-X
0" TYP	WF	BF-X
0" TYP	WF	BF-X

INDICATES A BRACE FRAME ABOVE AND BELOW LEVEL

INDICATES A BRACE FRAME ABOVE LEVEL

INDICATES A BRACE FRAME BELOW LEVEL

FIREPROOFING NOTES:

- STEEL COLUMNS SHOWN ON THIS DRAWING SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING UNLESS INDICATED AS A ROUND HSS COLUMN OR BUILT-UP COLUMN ALONG COLUMN GRID BB.1. EXPOSED TO VIEW PORTION OF ROUND HSS COLUMNS SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING. CONCEALED FROM VIEW PORTION OF ROUND HSS COLUMNS SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING. BUILT-UP COLUMNS ALONG GRID BB.1 SHALL RECEIVE 2-HOUR RATING BY INTUMESCENT MASTIC FIREPROOFING.
- STEEL BEAMS AND KICKERS SHOWN ON THIS DRAWING, SUPPORTING UPPER LEVEL FLOORS SHALL RECEIVE 2-HOUR RATING BY CEMENTITIOUS FIREPROOFING.
- CONCEALED FROM VIEW BRACED FRAMES SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING.
- EXPOSED TO VIEW BRACED FRAMES SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
- AT STAIRS 3, 5, AND 6 ALL EXPOSED TO VIEW BEAMS AND HSS TUBES SUPPORTING STAIR LANDINGS AND STRINGERS SHALL RECEIVE 2-HOUR RATING BY INTUMESCENT MASTIC FIREPROOFING. NO FIREPROOFING REQUIRED AT SUPPLEMENTAL HSS TUBES SUPPORTING ONLY CURTAINWALL.
- ALONG GRID Z AND AA ROUND COLUMNS TRANSITIONING FROM CEMENTITIOUS FIREPROOFING (FIRST FLOOR) TO INTUMESCENT MASTIC FIREPROOFING, TRANSITION SHALL OCCUR AT ELEVATION 118'-0".
- NO FIREPROOFING REQUIRED AT SUPPLEMENTAL HSS TUBES LOCATED AT THE VOCATIONAL OVERHEAD DOORS.
- NO FIREPROOFING REQUIRED AT SUPPLEMENTAL HSS TUBES LOCATED AT STOREFRONT, CURTAINWALL, AND FIBERGLASS PANEL HEADS.
- COORDINATE FIREPROOFING REQUIREMENTS WITH THE SPECIFICATIONS AND ARCHITECTURAL DRAWINGS.

FRAMING ELEVATION NOTES:

- TYPICAL UNDERSIDE OF DECK ELEVATION = (89' - 6 3/4") AT THE FIRST FLOOR LEVEL. IN THE AREA BOUNDED BY GRIDS (BB) - (MM) AND (22) - (27), UNLESS NOTED OTHERWISE AS (X' - X"), (+/-)X' - X"), OR H/LO.
- FRAMING ELEVATIONS ARE BASED ON TOP-OF-FLOOR ELEVATION = (100' - 0"). SHOWN ON THE ARCHITECTURAL DRAWINGS.
- COORDINATE ALL ELEVATIONS WITH ARCHITECTURAL DRAWINGS.
- SLOPE STEEL UNIFORMLY TO COLUMN ELEVATIONS SHOWN ON PLANS.

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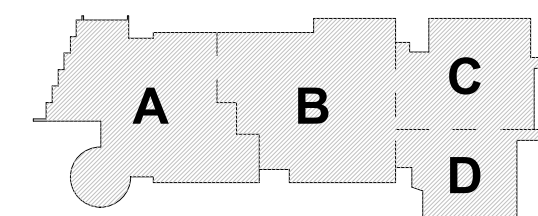
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03/31/2023	EARLY STRUCTURAL BID PACKAGE
REVISION LIST	
SS-S-1	4/14/2023 [STRUCTURAL STEEL ADDENDUM 1]
SS-S-2	4/21/2023 [STRUCTURAL STEEL ADDENDUM 2]

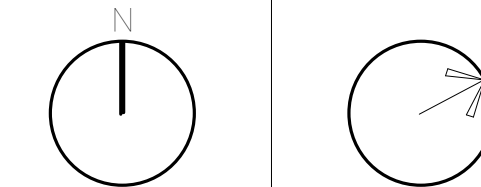
BID SET

August 28th, 2023



KEY PLAN

PROJECT NORTH MAGNETIC NORTH

MEZZANINE
FLOOR FRAMING
- AREA A

Scale: 1/8" = 1'-0"

Job No.: 20202

Drawn By: EDG

Date: August 28th, 2023

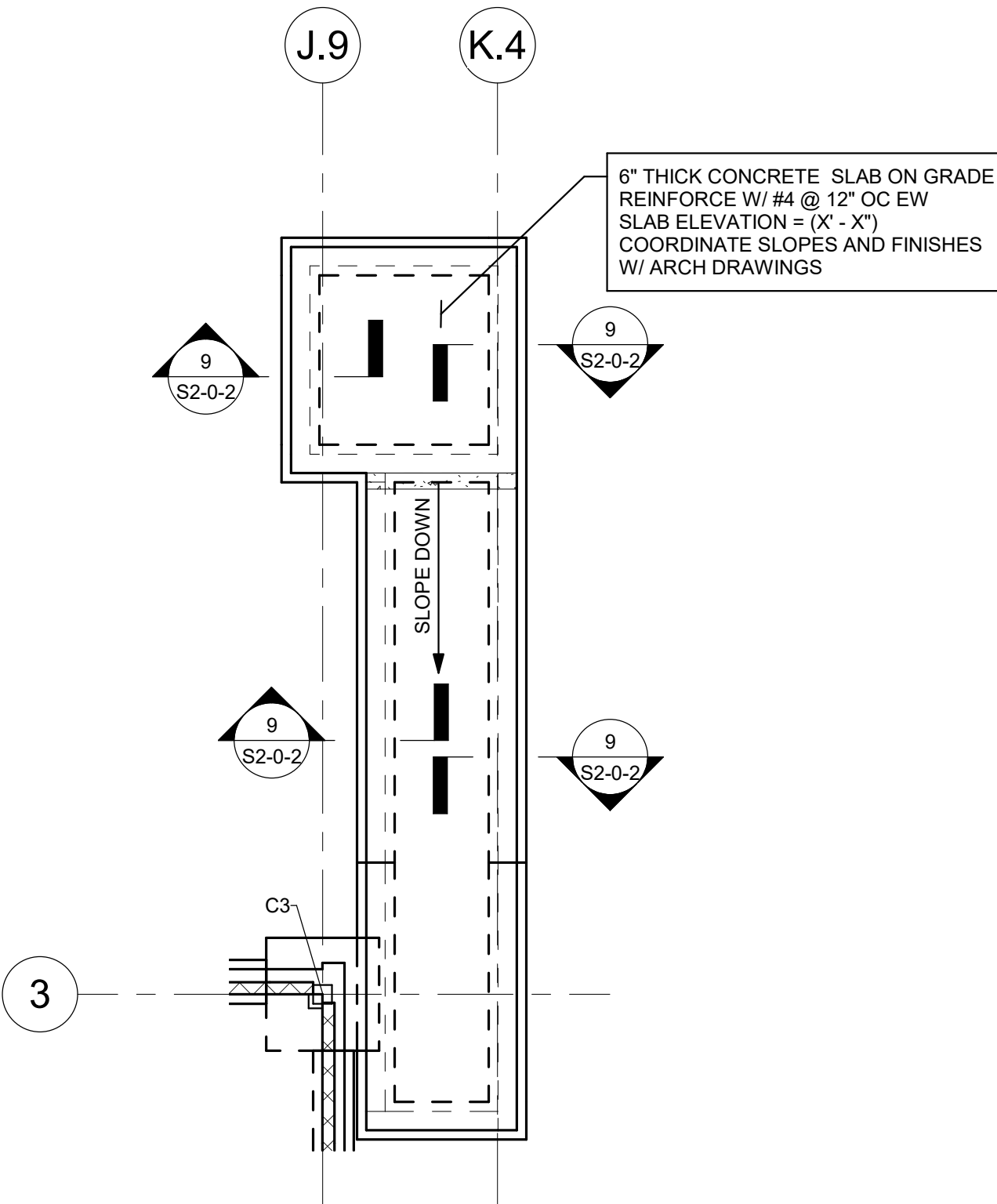
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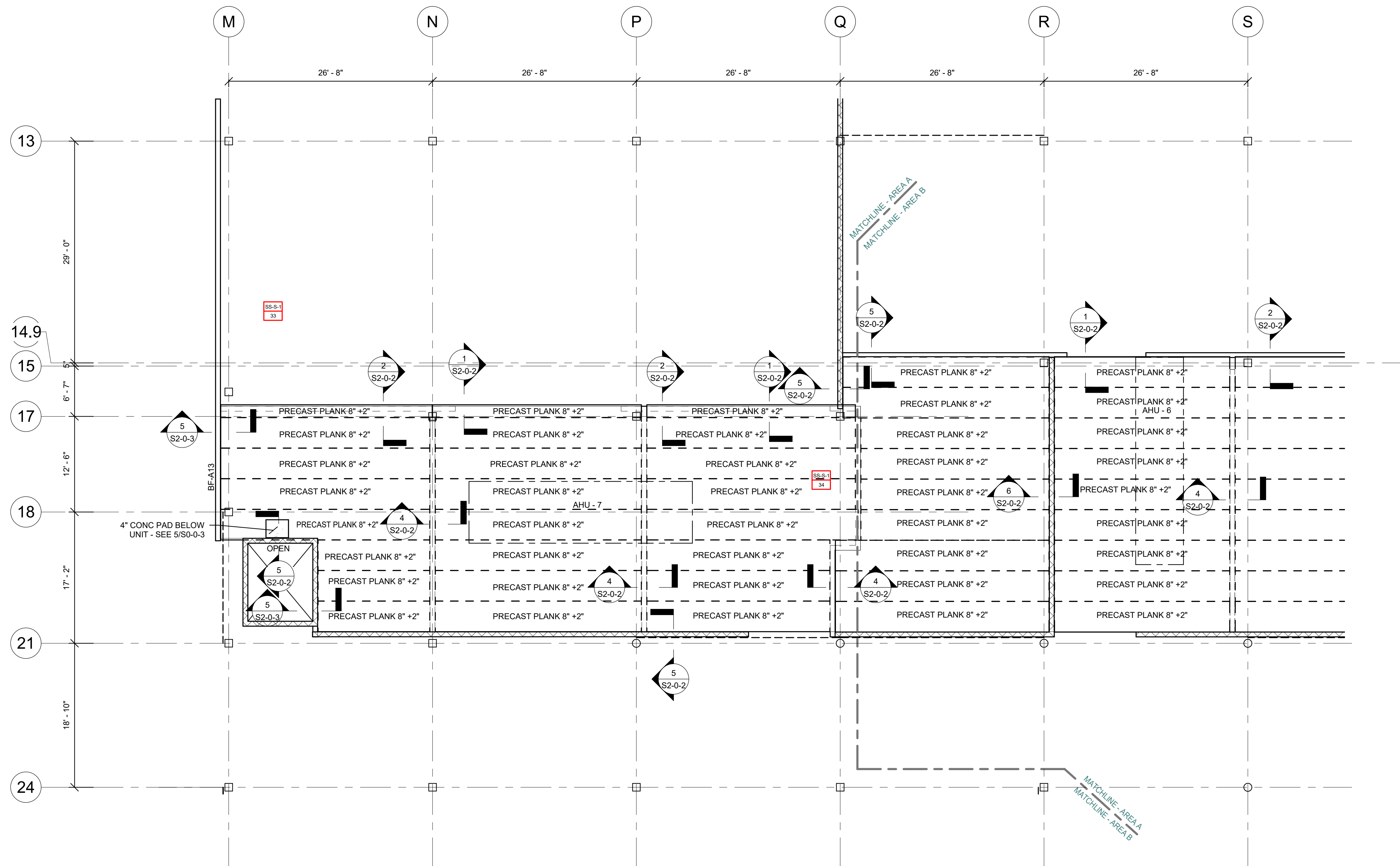
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- REFER TO ARCHITECTURAL DRAWINGS FOR ELEVATIONS AND VERTICAL DIMENSIONS. FITCH ALL STEEL UNIFORMLY TO LOW POINTS AT THE COLUMNS AND BENT BEAMS AS SHOWN ON THE ARCHITECTURAL DRAWINGS.
- 8" ETC. INDICATES A BRACED BAY. REFER TO BRACED FRAME ELEVATIONS AND DETAILS ON DRAWINGS S4-0-1, S4-0-2, S4-0-3, S4-0-4, S4-0-5, S4-0-6, S4-0-7 AND S4-0-8 FOR ADDITIONAL INFORMATION.
- [X] INDICATES THE NUMBER OF 3/4" DIAMETER x 4 1/4" LONG HEADED STUDS WELDED TO THE TOP FLANGE OF THE BEAM. SPACE STUDS EVENLY ALONG THE BEAM UNLESS NOTED OTHERWISE.
- INDICATES A MOMENT CONNECTION TO DEVELOP THE FULL CAPACITY OF THE MEMBER. REFER TO TYPICAL DETAILS 7, 8 AND 9 ON DRAWING S0-0-6.
- INDICATES A 5/16" FILLET WELD ALL AROUND. (HSS BEAM TO HSS COLUMN) WHERE BEAM DIMENSIONS EXCEED COLUMN DIMENSIONS PROVIDE 1/2" THICK STEEL CAP PLATE TO ACHIEVE ALL AROUND WELD. REFER TO TYPICAL DETAIL 2 ON DRAWING S0-0-7.
- < X' > INDICATES UPWARD CAMBER AT THE MID-SPAN OF THE MEMBER.
- 5-1/4" INDICATES SPAN DIRECTION OF 2" DEEP, 20 GAGE GALVANIZED COMPOSITE STEEL DECK WITH 3 1/4" LIGHT WEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 5 1/4". REINFORCE WITH 6x6 - W2.1xW2.1 WWR.
- 1 1/2" INDICATES SPAN DIRECTION OF 1 1/2" DEEP, 20 GAGE TYPE B GALVANIZED STEEL ROOF DECK.
- 3" NA INDICATES SPAN DIRECTION OF 3" DEEP, 20 GAGE TYPE NA, GALVANIZED ACOUSTIC STEEL ROOF DECK.
- 4" LW INDICATES SPAN DIRECTION OF 1 1/2" DEEP, 20 GAGE GALVANIZED COMPOSITE STEEL DECK WITH 2 1/2" LIGHT WEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 4". REINFORCE WITH 6x6 - W2.1xW2.1 WWR.
- FOR EXACT NUMBER, SIZE, AND LOCATION OF OPENING IN STEEL DECKING REFER TO MECHANICAL AND ARCHITECTURAL DRAWINGS FOR FRAMING INFORMATION. REFER TO DETAILS 1, 2 AND 3 ON DRAWING S0-0-8.
- 6" HATCHED AREA INDICATES LOCATION OF CONCRETE SLAB WITH 3" DEEP, 18 GAGE GALVANIZED COMPOSITE STEEL DECK WITH 4" NORMAL WEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 6". REINFORCE WITH 6x6 - W2.1xW2.1 WWR. REFER TO TYPICAL DETAIL 5 ON DRAWING S0-0-7 FOR ADDITIONAL INFORMATION. USE 3/4" DIA x 7' LONG HOLLOW STUDS.
- INDICATES A ROOF DRAIN. REFER TO TYPICAL STRUCTURAL DETAILS 1 AND 2 ON DRAWING S0-0-8 AND DETAIL 1 ON DRAWING S0-0-8. FOR DECKING SUPPORT, REFER TO DETAIL 6 ON DRAWING S0-0-7. REFER TO PLUMBING AND ARCHITECTURAL DRAWINGS FOR OPENING SIZES AND LOCATIONS.
- CT INDICATES A COLUMN TERMINATES AT THIS LEVEL.
- WF INDICATES A BEND IN THE STEEL BEAM. REFER TO TYPICAL DETAIL 9 ON DRAWING S0-0-7.
- INDICATES A CMU WALL. REFER TO TYPICAL DETAIL 8 ON DRAWING S0-0-4 FOR REINFORCEMENT. DETAIL 6 ON DRAWING S0-0-5 FOR CONNECTION OF SHEAR WALLS TO BEAMS AND COLUMNS. AND DETAIL 7 ON DRAWING S0-0-5 FOR CONNECTIONS TO STEEL BEAMS AND CONCRETE SLABS AT THE TOP OF WALL FOR NON-STRUCTURAL WALLS. REFER TO RELEVANT SECTIONS FOR CONNECTIONS OF SHEAR WALLS TO THE STRUCTURE.
- 8" x 2" PC PLANK INDICATES SPAN OF 8" DEEP PRESTRESSED, PRECAST CONCRETE HOLLOW CORE PLANK WITH A MINIMUM 2" OF NORMAL WEIGHT CONCRETE TOPPING. PLANK AND CONCRETE TOPPING SLAB TO BE DESIGNED TO SUPPORT A MINIMUM LIVE LOAD CAPACITY OF 150 PSF. PRECAST CONCRETE PLANK TO BE DESIGNED FOR MINIMUM OF 2 HOUR FIRE RATING.
- 10" x 2" PC PLANK INDICATES SPAN OF 10" DEEP PRESTRESSED, PRECAST CONCRETE HOLLOW CORE PLANK WITH A MINIMUM 2" OF NORMAL WEIGHT CONCRETE TOPPING. PLANK AND CONCRETE TOPPING SLAB TO BE DESIGNED TO SUPPORT A MINIMUM LIVE LOAD CAPACITY OF 150 PSF. PRECAST CONCRETE PLANK TO BE DESIGNED FOR MINIMUM OF 2 HOUR FIRE RATING.
- FOR DIMENSIONS AND ELEVATIONS NOT GIVEN REFER TO ARCHITECTURAL DRAWINGS.
- WF INDICATES A SPLICE CONNECTION ALONG A CONTINUOUS STEEL BEAM. DESIGN SPLICE CONNECTION FOR FULL CAPACITY OF STEEL BEAM.
- INDICATES A SLAB DEPRESSION. REFER TO DETAILS 1 ON DRAWING S0-0-6.
- 9/16" INDICATES SPAN DIRECTION OF 9/16" DEEP, 20 GAGE GALVANIZED STEEL ROOF DECK.
- X'X'X'X' INDICATES OPENING IN WEB OF STEEL BEAM FOR MECH/ELECT/PLUMB/FIRE PROTECT ETC. COORDINATE WITH APPLICABLE DRAWING FOR OPENING SIZE. SEE TYPICAL DETAIL 5 AND 9 ON S0-0-6. FOR ADDITIONAL INFORMATION.
- 3" INDICATES SPAN DIRECTION OF 3" DEEP, 20 GAGE TYPE N, GALVANIZED STEEL ROOF DECK.
- STEEL BEAMS ARE EQUALLY SPACED BETWEEN COLUMN GRIDLINES. UNLESS NOTED OTHERWISE.

FIREPROOFING NOTES:

- STEEL COLUMNS, SHOWN ON THIS DRAWING, SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING UNLESS INDICATED AS A ROUND HSS COLUMN. EXPOSED TO VIEW PORTION OF ROUND HSS COLUMNS SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING. CONCEALED FROM VIEW PORTION OF ROUND HSS COLUMN SHALL RECEIVE SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
- EXPOSED TO VIEW BRACED FRAMES SHOWN ON THIS DRAWING, SHALL RECEIVE 2-HOUR RATING BY INTUMESCENT MASTIC FIREPROOFING.
- COORDINATE FIREPROOFING REQUIREMENTS WITH THE SPECIFICATIONS AND ARCHITECTURAL DRAWINGS.



LOADING DOCK PART PLAN



FRAMING ELEVATION NOTES:

- TYPICAL TOP-OF-CONCRETE TOPPING ELEVATION = (109'-8\") AT THE MEZZANINE FLOOR LEVEL. IN THE AREA BOUNDED BY GRID (M) - (Q) AND (17) - (21).
- FRAMING ELEVATIONS ARE BASED ON TYPICAL BOTTOM-OF-PLANK ELEVATION = (108' - 8\"), SHOWN ON THE ARCHITECTURAL DRAWINGS.
- COORDINATE ALL ELEVATIONS WITH ARCHITECTURAL DRAWINGS.

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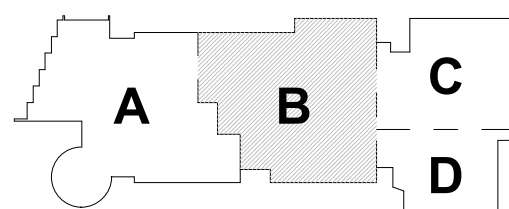
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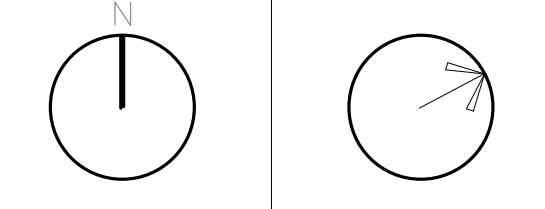
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BID SET

August 28th, 2023



PROJECT NORTH MAGNETIC NORTH



**MEZZANINE
FLOOR
FRAMING- AREA
B**

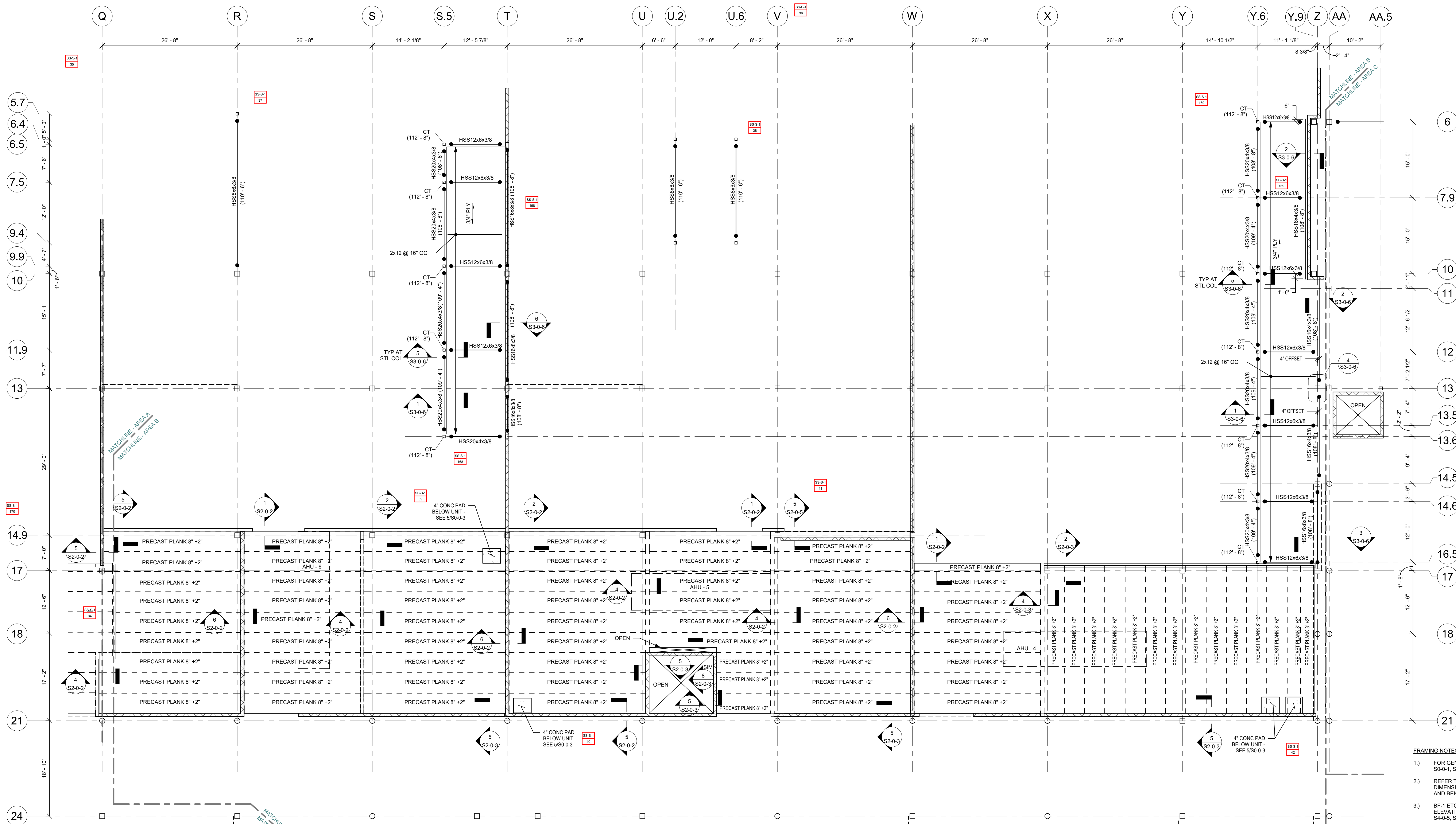
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S1-1-1MB



FRAMING ELEVATION NOTES:

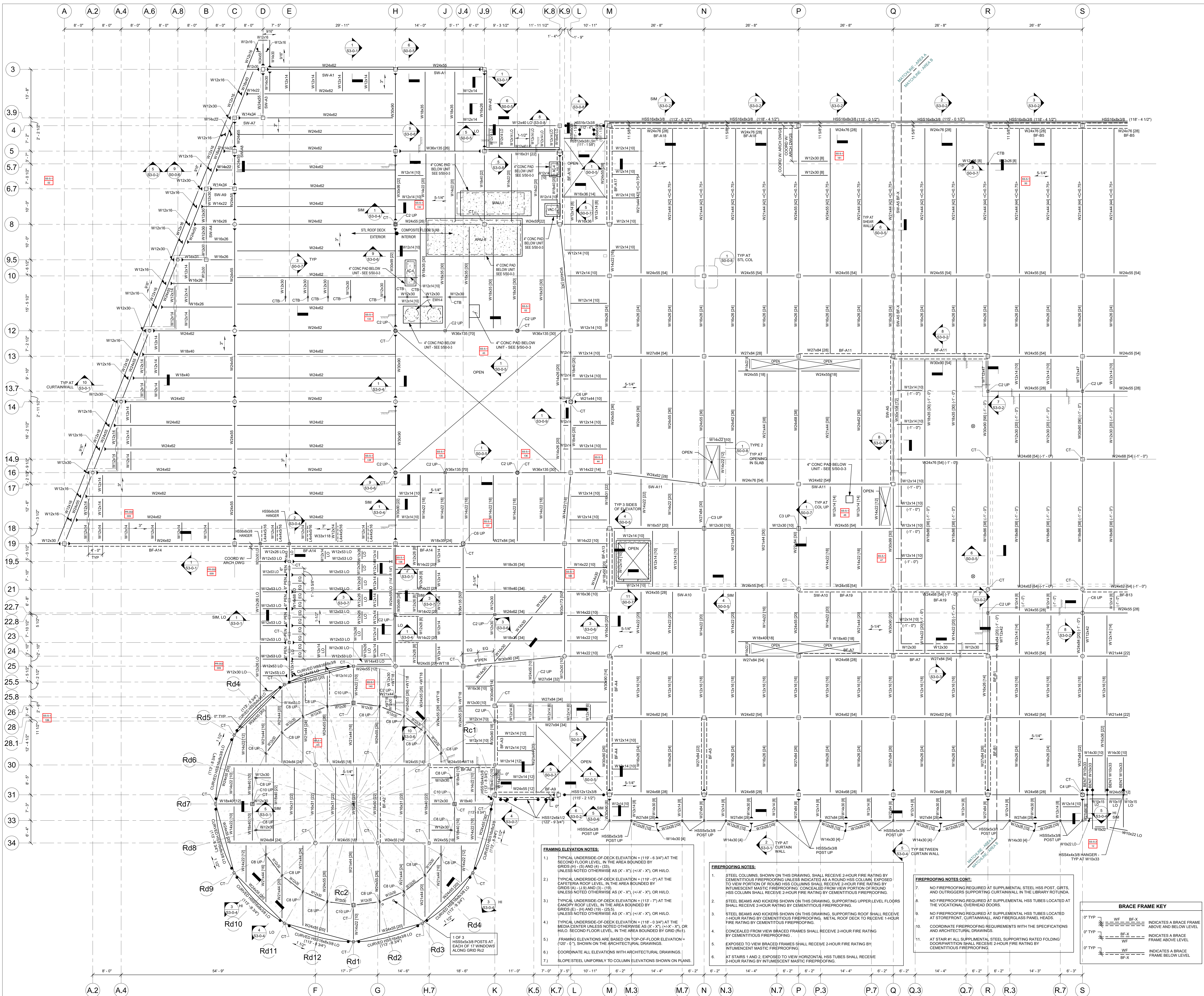
- 1.) TYPICAL TOP-OF-CONCRETE TOPPING ELEVATION = (109'-6") AT THE MEZZANINE FLOOR LEVEL. IN THE AREA BOUNDED BY GRIDS (Q) - (Z) AND (15) - (21).
- 2.) TYPICAL TOP-OF-STEEL ELEVATION = (108'-9") AT THE MEZZANINE FLOOR LEVEL. IN THE AREA BOUNDED BY GRIDS (S) - (T) AND (6) - (13.6), UNLESS NOTED OTHERWISE AS (X-X'), (Y-Y'), OR HILO.
- 3.) TYPICAL TOP-OF-STEEL ELEVATION = (108'-9") AT THE MEZZANINE FLOOR LEVEL. IN THE AREA BOUNDED BY GRIDS (Y) - (Z) AND (6) - (16.5), UNLESS NOTED OTHERWISE AS (X-X'), (Y-Y'), OR HILO.
- 4.) FRAMING ELEVATIONS ARE BASED ON TYPICAL BOTTOM-OF-PLANK ELEVATION = (108'-8"), SHOWN ON THE ARCHITECTURAL DRAWINGS.
- 5.) COORDINATE ALL ELEVATIONS WITH ARCHITECTURAL DRAWINGS.

FIREPROOFING NOTES:

1. STEEL COLUMNS, SHOWN ON THIS DRAWING, SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING UNLESS INDICATED AS A ROUND HSS COLUMN. EXPOSED TO VIEW PORTION OF ROUND HSS COLUMNS SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING. CONCEALED FROM VIEW PORTION OF ROUND HSS COLUMN SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
2. STEEL COLUMNS, HSS BEAMS, AND ANGLES SUPPORTING THE PLYWOOD MEZZANINES BETWEEN COLUMN GRIDS S.5 AND T AND GRIDS Y.6 AND Z SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING.
3. EXPOSED TO VIEW BRACED FRAMES SHOWN ON THIS DRAWING, SHALL RECEIVE 2-HOUR RATING BY INTUMESCENT MASTIC FIREPROOFING.
4. NO FIREPROOFING REQUIRED AT COLUMNS AT GRIDS Y.10-6.5, Y.11-6.5, Y.10-9.2, AND Y.11-9.2 OR AT THE HIBTUBES FRAMING INTO THEM.
5. COORDINATE FIREPROOFING REQUIREMENTS WITH THE SPECIFICATIONS AND ARCHITECTURAL DRAWINGS.

FRAMING NOTES:

- 1.) FOR GENERAL NOTES AND TYPICAL DETAILS SEE DRAWINGS S0-0-1, S0-0-2, S0-0-3, S0-0-4, S0-0-5, S0-0-6, S0-0-7 AND S0-0-8.
- 2.) REFER TO ARCHITECTURAL DRAWINGS FOR ELEVATIONS AND VERTICAL DIMENSIONS. PITCH ALL STEEL UNIFORMLY TO LOW POINTS AT THE COLUMNS AND BENT BEAMS AS SHOWN ON THE ARCHITECTURAL DRAWINGS.
- 3.) BF-1 ETC., INDICATES A BRACED BAY. REFER TO BRACED FRAME ELEVATIONS AND DETAILS ON DRAWINGS S4-0-1, S4-0-2, S4-0-3, S4-0-4, S4-0-5, S4-0-6, S4-0-7 AND S4-0-8 FOR ADDITIONAL INFORMATION.
- 4.) [XX] INDICATES THE NUMBER OF 3/4" DIAMETER x 4 1/4" LONG HEADED STUDS WELDED TO THE TOP FLANGE OF THE BEAM. SPACE STUDS EVENLY ALONG THE BEAM UNLESS NOTED OTHERWISE.
- 5.) INDICATES A MOMENT CONNECTION TO DEVELOP THE FULL CAPACITY OF THE MEMBER. REFER TO TYPICAL DETAILS 7, 8 AND 9 ON DRAWING S0-0-6.
- 6.) INDICATES A 5/16" FILLET WELD ALL AROUND. (HSS BEAM TO HSS COLUMN) WHERE BEAM DIMENSIONS EXCEED COLUMN DIMENSIONS PROVIDE 1/2" THICK STEEL CAP PLATE TO ACHIEVE ALL AROUND WELD. REFER TO TYPICAL DETAIL 2 ON DRAWING S0-0-7.
- 7.) < X' > INDICATES UPWARD CAMBER AT THE MID-SPAN OF THE MEMBER.
- 8.) 5-1/4" INDICATES SPAN DIRECTION OF 2" DEEP, 20 GAGE GALVANIZED COMPOSITE STEEL DECK WITH 5 1/4" LIGHT WEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 5 1/4". REINFORCE WITH 6x6 - W2.1xW2.1 WWR.
- 9.) 1 1/2" INDICATES SPAN DIRECTION OF 1 1/2" DEEP, 20 GAGE TYPE B, GALVANIZED STEEL ROOF DECK.
- 10.) 3" NA INDICATES SPAN DIRECTION OF 3" DEEP, 20 GAGE TYPE NA, GALVANIZED ACOUSTIC STEEL ROOF DECK.
- 11.) 4" LW INDICATES SPAN DIRECTION OF 1 1/2" DEEP, 20 GAGE GALVANIZED COMPOSITE STEEL DECK WITH 2 1/2" LIGHT WEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 4". REINFORCE WITH 6x6 - W2.1xW2.1 WWR.
- 12.) FOR EXACT NUMBER, SIZE, AND LOCATION OF OPENING IN STEEL DECKING REFER TO MECHANICAL AND ARCHITECTURAL DRAWINGS. FOR FRAMING INFORMATION, REFER TO DETAILS 1, 2 AND 3 ON DRAWING S0-0-6.
- 13.) 6" HATCHED AREA INDICATES LOCATION OF CONCRETE SLAB WITH 3" DEEP, 18 GAGE GALVANIZED COMPOSITE STEEL DECK WITH 4" NORMAL WEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 6". REINFORCE WITH 6x6 - W2.1xW2.1 WWR. REFER TO TYPICAL DETAIL 5 ON DRAWING S0-0-7 FOR ADDITIONAL INFORMATION. USE 3/4" DIA x 5' LONG HEADED STUDS.
- 14.) INDICATES A ROOF DRAIN. REFER TO TYPICAL STRUCTURAL DETAILS 1 AND 2 ON DRAWING S0-0-8 AND DETAIL 1 ON DRAWING S0-0-8 FOR DECKING SUPPORT. REFER TO DETAIL 6 ON DRAWING S0-0-7. REFER TO PLUMBING AND ARCHITECTURAL DRAWINGS FOR OPENING SIZES AND LOCATIONS.
- 15.) CT INDICATES A COLUMN TERMINATES AT THIS LEVEL.
- 16.) W INDICATES A BEND IN THE STEEL BEAM. REFER TO TYPICAL DETAIL 9 ON DRAWING S0-0-7.
- 17.) OR OR INDICATES A CMU WALL. REFER TO TYPICAL DETAIL 8 ON DRAWING S0-0-4 FOR REINFORCEMENT. DETAIL 6 ON DRAWING S0-0-5 FOR CONNECTION OF SHEAR WALLS TO BEAMS AND COLUMNS, AND DETAIL 7 ON DRAWING S0-0-5 FOR CONNECTIONS TO STEEL BEAMS AND CONCRETE SLABS AT THE TOP OF WALL FOR NON-STRUCTURAL WALLS. REFER TO RELEVANT SECTIONS FOR CONNECTIONS OF SHEAR WALLS TO THE STRUCTURE.
- 18.) 8' x 2" PC PLANK INDICATES SPAN OF 8' DEEP PRESTRESSED, PRECAST CONCRETE HOLLOW CORE PLANK WITH A MINIMUM 2" OF NORMAL WEIGHT CONCRETE TOPPING. PLANK AND CONCRETE TOPPING SLAB TO BE DESIGNED TO SUPPORT A MINIMUM LIVE LOAD CAPACITY OF 150 PSF. PRECAST CONCRETE PLANK TO BE DESIGNED FOR MINIMUM OF 2 HOUR FIRE RATING.
- 19.) 10' x 2" PC PLANK INDICATES SPAN OF 10' DEEP PRESTRESSED, PRECAST CONCRETE HOLLOW CORE PLANK WITH A MINIMUM 2" OF NORMAL WEIGHT CONCRETE TOPPING. PLANK AND CONCRETE TOPPING SLAB TO BE DESIGNED TO SUPPORT A MINIMUM LIVE LOAD CAPACITY OF 150 PSF. PRECAST CONCRETE PLANK TO BE DESIGNED FOR MINIMUM OF 2 HOUR FIRE RATING.
- 20.) FOR DIMENSIONS AND ELEVATIONS NOT GIVEN REFER TO ARCHITECTURAL DRAWINGS.
- 21.) W INDICATES A SPLICE CONNECTION ALONG A CONTINUOUS STEEL BEAM. DESIGN SPLICE CONNECTION FOR FULL CAPACITY OF STEEL BEAM.
- 22.) mmmm INDICATES A SLAB DEPRESSION. REFER TO DETAILS 1 ON DRAWING S0-0-6.
- 23.) 6/16" GALV INDICATES SPAN DIRECTION OF 9/16" DEEP, 20 GAGE GALVANIZED STEEL ROOF DECK.
- 24.) XX'XXX' INDICATES OPENING IN WEB OF STEEL BEAM FOR MECH/ELECT/PLUMB/FIRE PROTECT/ETC. COORDINATE WITH APPLICABLE DRAWING FOR OPENING SIZE. SEE TYPICAL DETAIL 5 AND 6 ON S0-0-6 FOR ADDITIONAL INFORMATION.
- 25.) 3" INDICATES SPAN DIRECTION OF 3" DEEP, 20 GAGE TYPE N, GALVANIZED STEEL ROOF DECK.
- 26.) STEEL BEAMS ARE EQUALLY SPACED BETWEEN COLUMN GRIDLINES, UNLESS NOTED OTHERWISE.



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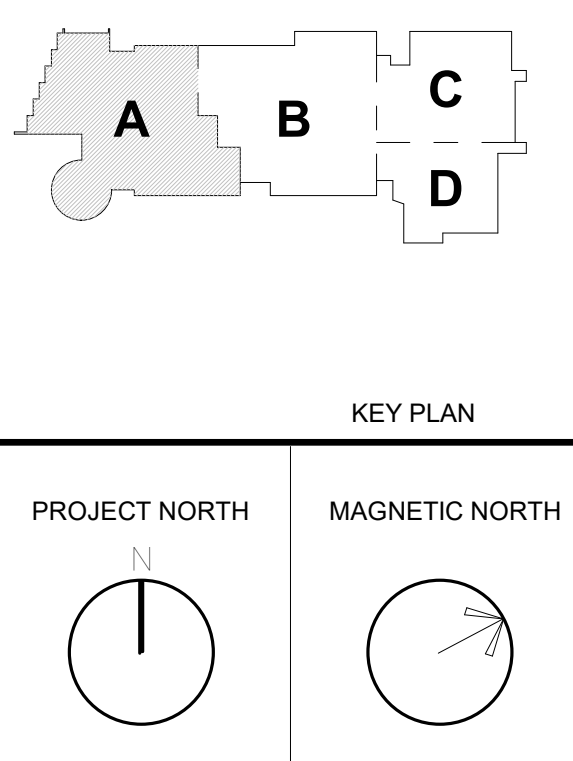
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03/12/2023	EARLY STRUCTURAL BID PACKAGE
REVISION LIST	
SS-S-1	4/14/2023 STRUCTURAL STEEL ADDENDUM 1
SS-S-2	4/21/2023 STRUCTURAL STEEL ADDENDUM 2
PR-002	6/29/2023 MISCELLANEOUS STRUCTURAL REVISIONS

BID SET

August 28th, 2023



**SECOND FLOOR
FRAMING PLAN -
AREA A**

Scale: 1/8" = 1'-0"
Job No.: 20202
Drawn By: EDG
Date: August 28th, 2023

S1-1-2A



FRAMING NOTES:

- FOR GENERAL NOTES AND TYPICAL DETAILS SEE DRAWINGS S0-0.1, S0-0.2, S0-0.3, S0-0.4, S0-0.5, S0-0.6, S0-0.7 AND S0-0.8.
- REFER TO ARCHITECTURAL DRAWINGS FOR ELEVATIONS AND VERTICAL DIMENSIONS. PITCH ALL STEEL UNIFORMLY TO LOW POINTS AT THE COLUMNS AND BENT BEAMS AS SHOWN ON THE ARCHITECTURAL DRAWINGS.
- BF-1 ETC., INDICATES A BRACED BAY. REFER TO BRACED FRAME ELEVATIONS AND DETAILS ON DRAWINGS S4-0.1, S4-0.2, S4-0.3, S4-0.4, S4-0.5, S4-0.6, S4-0.7 AND S4-0.8 FOR ADDITIONAL INFORMATION.
- [X] INDICATES THE NUMBER OF 3/4" DIAMETER x 4 1/4" LONG HEADED STUDS WELDED TO THE TOP FLANGE OF THE BEAM. SPACE STUDS EVENLY ALONG THE BEAM. UNLESS NOTED OTHERWISE.
- INDICATES A MOMENT CONNECTION TO DEVELOP THE FULL CAPACITY OF THE MEMBER. REFER TO TYPICAL DETAILS 7, 8 AND 9 ON DRAWING S0-0.6.
- INDICATES A 5/16" FILLET WELD ALL AROUND (HSS BEAM TO HSS COLUMN) WHERE BEAM DIMENSIONS EXCEED COLUMN DIMENSIONS PROVIDE 1/2" THICK STEEL CAP PLATE TO ACHIEVE ALL AROUND WELD. REFER TO TYPICAL DETAIL 2 ON DRAWING S0-0.7.
- <X> INDICATES UPWARD CAMBER AT THE MID-SPAN OF THE MEMBER.
- 5'-1/4" INDICATES SPAN DIRECTION OF 2" DEEP, 20 GAGE GALVANIZED COMPOSITE STEEL DECK WITH 3 1/2" LIGHT WEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 5'-1/4" REINFORCE WITH 6#6 - W2.1W2.1 WWF.
- 1 1/2" INDICATES SPAN DIRECTION OF 1 1/2" DEEP, 20 GAGE TYPE B GALVANIZED STEEL ROOF DECK.
- 3" NA INDICATES SPAN DIRECTION OF 3" DEEP, 20 GAGE TYPE NA GALVANIZED ACUSTIC STEEL ROOF DECK.
- 4" LW INDICATES SPAN DIRECTION OF 1 1/2" DEEP, 20 GAGE GALVANIZED COMPOSITE STEEL DECK WITH 3 1/2" LIGHT WEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 4" REINFORCE WITH 6#6 - W2.1W2.1 WWF.
- FOR EXACT NUMBER, SIZE, AND LOCATION OF OPENING IN STEEL DECKING REFER TO MECHANICAL AND ARCHITECTURAL DRAWINGS. FOR FRAMING INFORMATION, REFER TO DETAILS 1, 2 AND 3 ON DRAWING S0-0.8.
- HATCHED AREA INDICATES LOCATION OF CONCRETE SLAB WITH 3" DEEP, 18 GAGE GALVANIZED COMPOSITE STEEL DECK WITH 4" NORMAL WEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 8" REINFORCE WITH 6#6 - W2.1W2.1 WWF. REFER TO TYPICAL DETAIL 5 ON DRAWING S0-0.7 FOR ADDITIONAL INFORMATION. USE 3/4" DIA x 5' LONG HEADED STUDS.
- INDICATES A ROOF DRAIN. REFER TO TYPICAL STRUCTURAL DETAILS 1 AND 2 ON DRAWING S0-0.4 AND DETAIL 1 ON DRAWING S0-0.8 FOR DECKING SUPPORT. REFER TO DETAIL 6 ON DRAWING S0-0.7. REFER TO PLUMBING AND ARCHITECTURAL DRAWINGS FOR OPENING SIZES AND LOCATIONS.
- CT INDICATES A COLUMN TERMINATES AT THIS LEVEL.
- INDICATES A BEND IN THE STEEL BEAM. REFER TO TYPICAL DETAIL 9 ON DRAWING S0-0.7.
- INDICATES A CMU WALL. REFER TO TYPICAL DETAIL 8 ON DRAWING S0-0.4 FOR REINFORCEMENT. DETAIL 1 ON DRAWING S0-0.8 FOR CONNECTION OF SHEAR WALLS TO BEAMS AND COLUMNS, AND DETAIL 7 ON DRAWING S0-0.3 FOR CONNECTIONS TO STEEL BEAMS AND CONCRETE SLABS AT NON-STRUCTURAL WALLS. REFER TO RELEVANT SECTIONS FOR CONNECTIONS OF SHEAR WALLS TO THE STRUCTURE.
- PC PLANK INDICATES SPAN OF 8' DEEP PRESTRESSED, PRECAST CONCRETE HOLLOW CORE PLANK WITH A MINIMUM 2" OF NORMAL WEIGHT CONCRETE TOPPING. PLANK AND CONCRETE TOPPING SLAB TO BE DESIGNED TO SUPPORT A MINIMUM LIVE LOAD CAPACITY OF 150 PSF. PRECAST CONCRETE PLANK TO BE DESIGNED FOR MINIMUM OF 2 HOUR FIRE RATINGS.
- 10' x 2" PC PLANK INDICATES SPAN OF 10' DEEP PRESTRESSED, PRECAST CONCRETE HOLLOW CORE PLANK WITH A MINIMUM 2" OF NORMAL WEIGHT CONCRETE TOPPING. PLANK AND CONCRETE TOPPING SLAB TO BE DESIGNED TO SUPPORT A MINIMUM LIVE LOAD CAPACITY OF 150 PSF. PRECAST CONCRETE PLANK TO BE DESIGNED FOR MINIMUM OF 2 HOUR FIRE RATINGS.
- FOR DIMENSIONS AND ELEVATIONS NOT GIVEN REFER TO ARCHITECTURAL DRAWINGS.
- WF INDICATES A SPLICE CONNECTION ALONG A CONTINUOUS STEEL BEAM. DESIGN SPLICE CONNECTION FOR FULL CAPACITY OF STEEL BEAM.
- INDICATES A SLAB DEPRESSION. REFER TO DETAILS 1 ON DRAWING S0-0.6.
- INDICATES SPAN DIRECTION OF 8 1/8" DEEP, 20 GAGE GALVANIZED STEEL ROOF DECK.
- XXXXX INDICATES OPENING IN WEB OF STEEL BEAM FOR MECH/ELECT/PLUMB/FIRE PROTECT. ETC. COORDINATE WITH APPLICABLE DRAWING FOR OPENING SIZE. SEE TYPICAL DETAIL 5 AND 6 ON S0-0.6 FOR ADDITIONAL INFORMATION.
- INDICATES SPAN DIRECTION OF 3" DEEP, 20 GAGE TYPE N GALVANIZED STEEL ROOF DECK.
- STEEL BEAMS ARE EQUALLY SPACED BETWEEN COLUMN GRIDLINES, UNLESS NOTED OTHERWISE.

FRAMING ELEVATION NOTES:

- TYPICAL ELEVATION OF DECK ELEVATION = (11'9" - 6'34") AT THE SECOND FLOOR LEVEL IN THE AREA BOUNDED BY GRIDS (Q) - (Z) AND (1) - (3). UNLESS NOTED OTHERWISE AS (X' - X''), ((X' - X'') - X''), OR HILO.
- TYPICAL UNDERSIDE OF DECK ELEVATION = (11'9" - 6'34") AT THE LOW ROOF LEVEL IN THE AREA BOUNDED BY GRIDS (V) - (U) AND (21) - (33). UNLESS NOTED OTHERWISE AS (X' - X''), ((X' - X'') - X''), OR HILO.
- TYPICAL UNDERSIDE OF DECK ELEVATION = (11'9" - 5'34") AT THE CANOPY ROOF LEVEL IN THE AREA BOUNDED BY GRIDS (S) - (U) AND (21) - (33). UNLESS NOTED OTHERWISE AS (X' - X''), ((X' - X'') - X''), OR HILO.
- FRAMING ELEVATIONS ARE BASED ON TOP-OF-FLOOR ELEVATION = (120' - 0"), SHOWN ON THE ARCHITECTURAL DRAWINGS.
- COORDINATE ALL ELEVATIONS WITH ARCHITECTURAL DRAWINGS.
- SLOPE STEEL UNIFORMLY TO COLUMN ELEVATIONS SHOWN ON PLANS.

BRACE FRAME KEY

- 0" TYP WF BF-X INDICATES A BRACE FRAME ABOVE AND BELOW LEVEL.
- 0" TYP BF-X WF INDICATES A BRACE FRAME ABOVE LEVEL.
- 0" TYP BF-X BF-X INDICATES A BRACE FRAME BELOW LEVEL.

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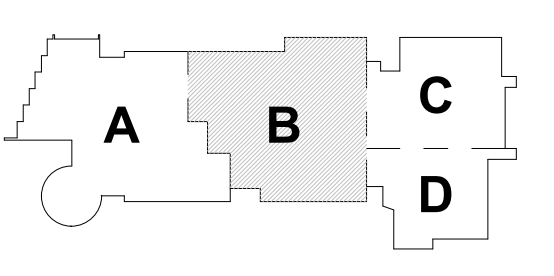
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03/31/2023	EARLY STRUCTURAL BID PACKAGE
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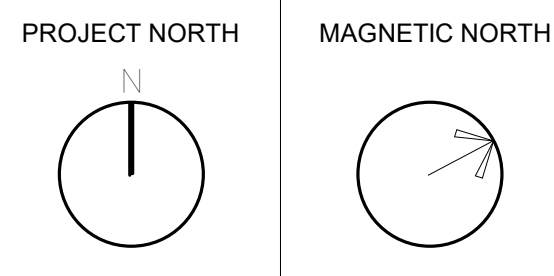
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BID SET	
August 28th, 2023	
KEY PLAN	
PROJECT NORTH	MAGNETIC NORTH
SECOND FLOOR FRAMING PLAN - AREA B	
Scale: 1/8" = 1'-0"	
Job No.: 20202	
Drawn By: EDG	
Date: August 28th, 2023	

BID SET

August 28th, 2023



KEY PLAN



SECOND FLOOR
FRAMING PLAN -
AREA B

Scale: 1/8" = 1'-0"
Job No.: 20202
Drawn By: EDG
Date: August 28th, 2023

S1-1-2B

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REVISION LIST	
03/31/2023	EARLY STRUCTURAL BID PACKAGE
SS-S-1	4/14/2023 STRUCTURAL STEEL ADDENDUM 1
SS-S-2	4/21/2023 STRUCTURAL STEEL ADDENDUM 2
PR-002	6/29/2023 MISCELLANEOUS STRUCTURAL REVISIONS

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BID SET

August 28th, 2023

KEY PLAN

PROJECT NORTH

MAGNETIC NORTH

Scale: 1/8" = 1'-0"

Job No.: 20202

Drawn By: EDG

Date: August 28th, 2023

S1-1-2C

FRAMING NOTES:

- FOR GENERAL NOTES AND TYPICAL DETAILS SEE DRAWINGS S0-0.1, S0-0.2, S0-0.3, S0-0.4, S0-0.5, S0-0.6, S0-0.7 AND S0-0.8.
- REFER TO ARCHITECTURAL DRAWINGS FOR ELEVATIONS AND VERTICAL DIMENSIONS. PITCH ALL STEEL UNIFORMLY TO LOW POINTS AT THE COLUMNS AND BENT BEAMS AS SHOWN ON THE ARCHITECTURAL DRAWINGS.
- BF-1 ETC. INDICATES A BRACED BAY. REFER TO BRACED FRAME ELEVATIONS AND DETAILS ON DRAWINGS S4-0.1, S4-0.2, S4-0.3, S4-0.4, S4-0.5, S4-0.6, S4-0.7 AND S4-0.8 FOR ADDITIONAL INFORMATION.
- [X] INDICATES THE NUMBER OF 3/4" DIAMETER x 4 1/4" LONG HEADED STUDS WELDED TO THE TOP FLANGE OF THE BEAM. SPACE STUDS EVENLY ALONG THE BEAM UNLESS NOTED OTHERWISE.
- INDICATES A MOMENT CONNECTION TO DEVELOP THE FULL CAPACITY OF THE MEMBER. REFER TO TYPICAL DETAILS 7, 8 AND 9 ON DRAWING S0-0.6.
- INDICATES A 5/16" FILLET WELD ALL AROUND. (HSS BEAM TO HSS COLUMN) WHERE BEAM DIMENSIONS EXCEED COLUMN DIMENSIONS PROVIDE 1/2" THICK STEEL CAP PLATE TO ACHIEVE ALL AROUND WELD. REFER TO TYPICAL DETAIL 2 ON DRAWING S0-0.7.
- < X > INDICATES UPWARD CAMBER AT THE MID-SPAN OF THE MEMBER.
- 5'-14" INDICATES SPAN DIRECTION OF 2" DEEP, 20 GAGE GALVANIZED COMPOSITE STEEL DECK WITH 3 1/4" LIGHT WEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 5 1/4". REINFORCE WITH 6#6 - W2, 1W2, 1 WWR.
- 1'-12" INDICATES SPAN DIRECTION OF 1 1/2" DEEP, 20 GAGE TYPE B, GALVANIZED STEEL ROOF DECK.
- 3" NA INDICATES SPAN DIRECTION OF 3" DEEP, 20 GAGE TYPE NA, GALVANIZED ACOUSTIC STEEL ROOF DECK.
- 4" LW INDICATES SPAN DIRECTION OF 1 1/2" DEEP, 20 GAGE GALVANIZED COMPOSITE STEEL DECK WITH 2 1/2" LIGHT WEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 4". REINFORCE WITH 6#6 - W2, 1W2, 1 WWR.
- FOR EXACT NUMBER, SIZE, AND LOCATION OF OPENING IN STEEL DECKING REFER TO MECHANICAL AND ARCHITECTURAL DRAWINGS. FOR FRAMING INFORMATION, REFER TO DETAILS 1, 2 AND 3 ON DRAWING S0-0.8.
- HATCHED AREA INDICATES LOCATION OF CONCRETE SLAB WITH 3" DEEP, 18 GAGE GALVANIZED COMPOSITE STEEL DECK WITH 4" NORMAL WEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 6". REINFORCE WITH 6#6 - W2, 1W2, 1 WWR. REFER TO TYPICAL DETAIL 5 ON DRAWING S0-0.7 FOR ADDITIONAL INFORMATION. USE 3/4" DIA x 5' LONG HEADED STUDS.
- INDICATES A ROOF DRAIN. REFER TO TYPICAL STRUCTURAL DETAILS 1 AND 2 ON DRAWING S0-0.8 AND DETAIL 1 ON DRAWING S0-0.8 FOR DECKING SUPPORT. REFER TO DETAIL 6 ON DRAWING S0-0.7 REFER TO PLUMBING AND ARCHITECTURAL DRAWINGS FOR OPENING SIZES AND LOCATIONS.
- CT INDICATES A COLUMN TERMINATES AT THIS LEVEL.
- WF INDICATES A BEND IN THE STEEL BEAM. REFER TO TYPICAL DETAIL 9 ON DRAWING S0-0.7.
- INDICATES A CMU WALL. REFER TO TYPICAL DETAIL 8 ON DRAWING S0-0.4 FOR REINFORCEMENT. DETAIL 6 ON DRAWING S0-0.4 FOR CONNECTION OF SHEAR WALLS TO BEAMS AND COLUMNS, AND DETAIL 7 ON DRAWING S0-0.5 FOR CONNECTIONS TO STEEL BEAMS AND CONCRETE SLABS AT THE TOP OF WALL FOR NON-STRUCTURAL WALLS. REFER TO RELEVANT SECTIONS FOR CONNECTIONS OF SHEAR WALLS TO THE STRUCTURE.
- 8" + 2" PC PLANK INDICATES SPAN OF 8" DEEP PRESTRESSED, PRECAST CONCRETE HOLLOW CORE PLANK WITH A MINIMUM 2" OF NORMAL WEIGHT CONCRETE TOPPING. PLANK AND CONCRETE TOPPING SLAB TO BE DESIGNED TO SUPPORT A MINIMUM LIVE LOAD CAPACITY OF 150 PSF. PRECAST CONCRETE PLANK TO BE DESIGNED FOR MINIMUM OF 2 HOUR FIRE RATING.
- 10" + 2" PC PLANK INDICATES SPAN OF 10" DEEP PRESTRESSED, PRECAST CONCRETE HOLLOW CORE PLANK WITH A MINIMUM 2" OF NORMAL WEIGHT CONCRETE TOPPING. PLANK AND CONCRETE TOPPING SLAB TO BE DESIGNED TO SUPPORT A MINIMUM LIVE LOAD CAPACITY OF 150 PSF. PRECAST CONCRETE PLANK TO BE DESIGNED FOR MINIMUM OF 2 HOUR FIRE RATING.
- FOR DIMENSIONS AND ELEVATIONS NOT GIVEN REFER TO ARCHITECTURAL DRAWINGS.
- WF INDICATES A SPICE CONNECTION ALONG A CONTINUOUS STEEL BEAM. DESIGN SPICE CONNECTION FOR FULL CAPACITY OF STEEL BEAM.
- INDICATES A SLAB DEPRESSION. REFER TO DETAILS 1 ON DRAWING S0-0.6.
- 9/16" INDICATES SPAN DIRECTION OF 9/16" DEEP, 20 GAGE GALVANIZED STEEL ROOF DECK.
- XXXXX INDICATES OPENING IN WEB OF STEEL BEAM FOR MECH/ELECT/PLUMB/FIRE PROTECT ETC COORDINATE WITH APPLICABLE DRAWING FOR OPENING SIZE. SEE TYPICAL DETAIL 5 AND 6 ON S0-0.6 FOR ADDITIONAL INFORMATION.
- 3" INDICATES SPAN DIRECTION OF 3" DEEP, 20 GAGE TYPE N, GALVANIZED STEEL ROOF DECK.
- STEEL BEAMS ARE EQUALLY SPACED BETWEEN COLUMN GRIDLINES, UNLESS NOTED OTHERWISE.

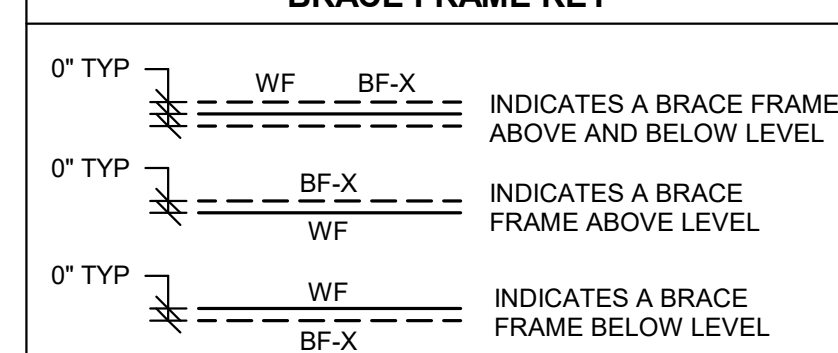
FRAMING ELEVATION NOTES:

- TYPICAL UNDERSIDE-OF-DECK ELEVATION = (119' - 6 3/4") AT THE SECOND FLOOR LEVEL, IN THE AREA BOUNDED BY GRID (AA) - (GG) AND (1) - (22) UNLESS NOTED OTHERWISE AS (X' - X'), (+X' - X'), OR HI/LO.
- TYPICAL UNDERSIDE-OF-DECK ELEVATION = (119' - 6 3/4") AT THE LOW ROOF LEVEL, IN THE AREA BOUNDED BY GRID (AA) - (FF) AND (2) - (11) UNLESS NOTED OTHERWISE AS (X' - X'), (+X' - X'), OR HI/LO.
- TYPICAL UNDERSIDE-OF-DECK ELEVATION = (112' - 8") AT THE CANOPY ROOF LEVEL, IN THE AREA BOUNDED BY GRID (AA) - (DD) AND (4) - (6).
- FRAMING ELEVATIONS ARE BASED ON TOP-OF-FLOOR ELEVATION = (120' - 0") SHOWN ON THE ARCHITECTURAL DRAWINGS.
- COORDINATE ALL ELEVATIONS WITH ARCHITECTURAL DRAWINGS.
- SLOPE STEEL UNIFORMLY TO COLUMN ELEVATIONS SHOWN ON PLANS.

FIREPROOFING NOTES:

- STEEL COLUMNS SHOWN ON THIS DRAWING SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING UNLESS INDICATED AS A ROUND HSS COLUMN OR BUILT-UP COLUMN ALONG COLUMN GRID BB.1. EXPOSED TO VIEW PORTION OF ROUND HSS COLUMNS SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING. CONCEALED FROM VIEW PORTION OF ROUND HSS COLUMN SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING. BUILT-UP COLUMNS ALONG GRID BB.1 SHALL RECEIVE 2-HOUR RATING BY INTUMESCENT MASTIC FIREPROOFING.
- STEEL BEAMS AND KICKERS SHOWN ON THIS DRAWING, SUPPORTING UPPER LEVEL FLOORS SHALL RECEIVE 2-HOUR RATING BY CEMENTITIOUS FIREPROOFING.
- STEEL BEAMS AND KICKERS SHOWN ON THIS DRAWING, SUPPORTING ROOF SHALL RECEIVE 1-HOUR RATING BY CEMENTITIOUS FIREPROOFING. METAL ROOF DECK TO RECEIVE 1-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
- CONCEALED FROM VIEW BRACED FRAMES SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING.
- EXPOSED TO VIEW BRACED FRAMES SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING.
- AT STAIRS 4 AND 7, ALL EXPOSED TO VIEW BEAMS AND HSS TUBES SUPPORTING STAIR LANDINGS AND STRINGERS SHALL RECEIVE 2-HOUR RATING BY INTUMESCENT MASTIC FIREPROOFING. NO FIREPROOFING REQUIRED AT SUPPLEMENTAL HSS TUBES SUPPORTING ONLY CURTAINWALL.
- NO FIREPROOFING REQUIRED AT SUPPLEMENTAL HSS TUBES SUPPORTING CURTAINWALL CW23.
- COORDINATE FIREPROOFING REQUIREMENTS WITH THE SPECIFICATIONS AND ARCHITECTURAL DRAWINGS.

BRACE FRAME KEY



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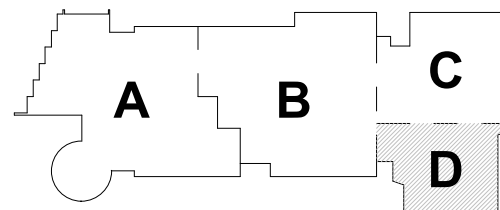


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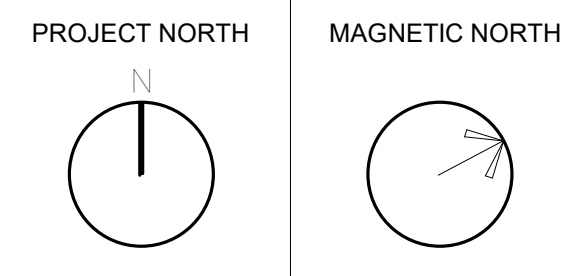
03/31/2023	EARLY STRUCTURAL BID PACKAGE
REVISION LIST	
SS-S-1	4/14/2023 STRUCTURAL STEEL ADDENDUM 1
SS-S-2	4/21/2023 STRUCTURAL STEEL ADDENDUM 2

BID SET

August 28th, 2023



KEY PLAN



SECOND FLOOR
FRAMING PLAN -
AREA D

Scale: 1/8" = 1'-0"
Job No.: 20202
Drawn By: EDG
Date: August 28th, 2023
S1-1-2D

FRAMING NOTES:

- FOR GENERAL NOTES AND TYPICAL DETAILS SEE DRAWINGS S0-0-1, S0-0-2, S0-0-3, S0-0-4, S0-0-5, S0-0-6, S0-0-7 AND S0-0-8.
- REFER TO ARCHITECTURAL DRAWINGS FOR ELEVATIONS AND VERTICAL DIMENSIONS. PITCH ALL STEEL UNIFORMLY TO LOW POINTS AT THE COLUMNS AND BENT BEAMS AS SHOWN ON THE ARCHITECTURAL DRAWINGS.
- BF-1 ETC., INDICATES A BRACED BAY. REFER TO BRACED FRAME ELEVATIONS AND DETAILS ON DRAWINGS S4-0-1, S4-0-2, S4-0-3, S4-0-4, S4-0-5, S4-0-6, S4-0-7 AND S4-0-8 FOR ADDITIONAL INFORMATION.
- [XX] INDICATES THE NUMBER OF 3/4" DIAMETER x 4 1/4" LONG HEADED STUDS WELDED TO THE TOP FLANGE OF THE BEAM. SPACE STUDS UNIFORMLY ALONG THE BEAM UNLESS NOTED OTHERWISE.
- INDICATES A MOMENT CONNECTION TO DEVELOP THE FULL CAPACITY OF THE MEMBER. REFER TO TYPICAL DETAILS 7, 8 AND 8 ON DRAWING S0-0-6.
- INDICATES A 5/16" FILLET WELD ALL AROUND. (HSS BEAM TO HSS COLUMN) WHERE BEAM DIMENSIONS EXCEED COLUMN DIMENSIONS PROVIDE 1/2" THICK STEEL CAP PLATE TO ACHIEVE ALL AROUND WELD. REFER TO TYPICAL DETAIL 2 ON DRAWING S0-0-7.
- < X > INDICATES UPWARD CAMBER AT THE MID-SPAN OF THE MEMBER.
- 5-1/4" INDICATES SPAN DIRECTION OF 2" DEEP, 20 GAUGE GALVANIZED COMPOSITE STEEL DECK WITH 3 1/4" LIGHT WEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 5 1/4". REINFORCE WITH 6x6 - W2.1xW2.1 WWR.
- 1 1/2" INDICATES SPAN DIRECTION OF 1 1/2" DEEP, 20 GAUGE TYPE B, GALVANIZED STEEL ROOF DECK.
- 3" INDICATES SPAN DIRECTION OF 3" DEEP, 20 GAUGE TYPE NA, GALVANIZED ACROUSTIC STEEL ROOF DECK.
- 4" LW INDICATES SPAN DIRECTION OF 1 1/2" DEEP, 20 GAUGE GALVANIZED COMPOSITE STEEL DECK WITH 2 1/2" LIGHT WEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 4". REINFORCE WITH 6x6 - W2.1xW2.1 WWR.
- FOR EXACT NUMBER, SIZE, AND LOCATION OF OPENING IN STEEL DECKING REFER TO MECHANICAL AND ARCHITECTURAL DRAWINGS. FOR FRAMING INFORMATION, REFER TO DETAILS 1, 2 AND 3 ON DRAWING S0-0-6.
- HATCHED AREA INDICATES LOCATION OF CONCRETE SLAB WITH 3" DEEP, 18 GAUGE GALVANIZED COMPOSITE STEEL DECK WITH 4" NORMAL WEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 6". REINFORCE WITH 6x6 - W2.1xW2.1 WWR. REFER TO TYPICAL DETAIL 5 ON DRAWING S0-0-7 FOR ADDITIONAL INFORMATION. USE 3/4" DIA x 5' LONG HEADED STUDS.
- INDICATES A ROOF DRAIN. REFER TO TYPICAL STRUCTURAL DETAILS 1 AND 2 ON DRAWING S0-0-6 AND DETAIL 1 ON DRAWING S0-0-8 FOR DECKING SUPPORT. REFER TO DETAIL 6 ON DRAWING S0-0-7. REFER TO PLUMBING AND ARCHITECTURAL DRAWINGS FOR OPENING SIZES AND LOCATIONS.
- CT INDICATES A COLUMN TERMINATES AT THIS LEVEL.
- WF INDICATES A BEND IN THE STEEL BEAM. REFER TO TYPICAL DETAILS ON DRAWING S0-0-7.
- INDICATES A CMU WALL. REFER TO TYPICAL DETAIL 8 ON DRAWING S0-0-4 FOR REINFORCEMENT. DETAIL 6 ON DRAWING S0-0-5 FOR CONNECTION OF SHEAR WALLS TO BEAMS AND COLUMNS, AND DETAIL 7 ON DRAWING S0-0-5 FOR CONNECTIONS TO STEEL BEAMS AND CONCRETE SLABS AT THE TOP OF WALL FOR NON-STRUCTURAL WALLS. REFER TO RELEVANT SECTIONS FOR CONNECTIONS OF SHEAR WALLS TO THE STRUCTURE.
- PC PLANK INDICATES SPAN OF 8" DEEP PRESTRESSED, PRECAST CONCRETE HOLLOW CORE PLANK WITH A MINIMUM 2" OF NORMAL WEIGHT CONCRETE TOPPING. PLANK AND CONCRETE TOPPING SLAB TO BE DESIGNED TO SUPPORT A MINIMUM LIVE LOAD CAPACITY OF 150 PSF. PRECAST CONCRETE PLANK TO BE DESIGNED FOR MINIMUM OF 2 HOUR FIRE RATING.
- 10" PC PLANK INDICATES SPAN OF 10" DEEP PRESTRESSED, PRECAST CONCRETE HOLLOW CORE PLANK WITH A MINIMUM 2" OF NORMAL WEIGHT CONCRETE TOPPING. PLANK AND CONCRETE TOPPING SLAB TO BE DESIGNED TO SUPPORT A MINIMUM LIVE LOAD CAPACITY OF 150 PSF. PRECAST CONCRETE PLANK TO BE DESIGNED FOR MINIMUM OF 2 HOUR FIRE RATING.
- FOR DIMENSIONS AND ELEVATIONS NOT GIVEN REFER TO ARCHITECTURAL DRAWINGS.
- WF INDICATES A SPICE CONNECTION ALONG A CONTINUOUS STEEL BEAM. DESIGN SPICE CONNECTION FOR FULL CAPACITY OF STEEL BEAM.
- INDICATES A SLAB DEPRESSION. REFER TO DETAILS 1 ON DRAWING S0-0-6.
- 9/16" GALVANIZED STEEL ROOF DECK.
- XX'XXX' INDICATES OPENING IN WEB OF STEEL BEAM FOR MECH/ELECT/PLUMB/FIRE PROTECT/ETC COORDINATE WITH APPLICABLE DRAWING FOR OPENING SIZE. SEE TYPICAL DETAIL 5 AND 6 ON S0-0-6 FOR ADDITIONAL INFORMATION.
- 9" INDICATES SPAN DIRECTION OF 3" DEEP, 20 GAUGE TYPE N, GALVANIZED STEEL ROOF DECK.

FRAMING ELEVATION NOTES:

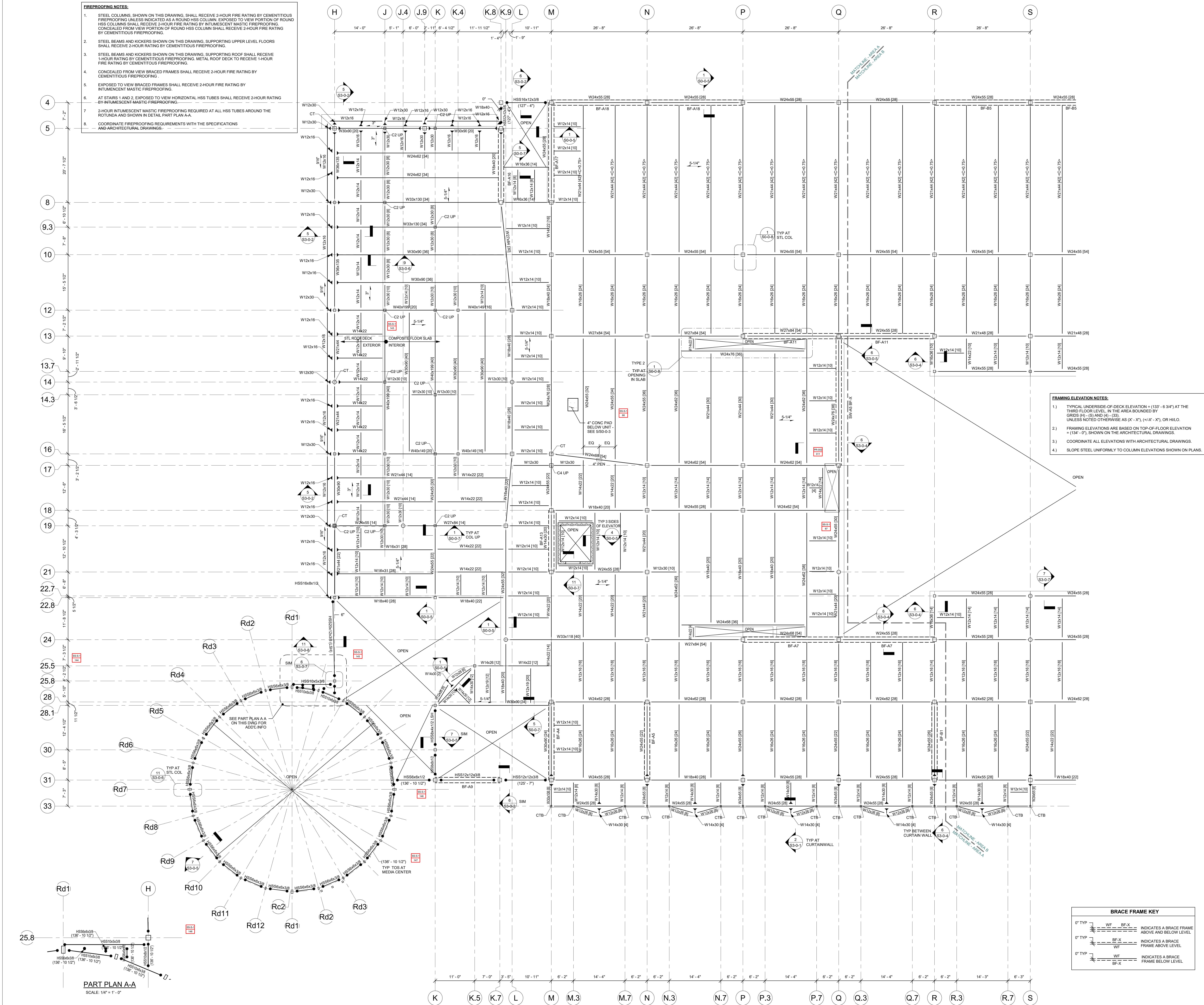
- TYPICAL UNDERSIDE-OF-DECK ELEVATION = (11' - 6 3/4") AT THE SECOND FLOOR LEVEL, IN THE AREA BOUNDED BY GRID (AA) - (DD) AND (22) - (S4.1) UNLESS NOTED OTHERWISE AS (X' - X'), (+/-X' - X'), OR HI/LO.
- TYPICAL UNDERSIDE-OF-DECK ELEVATION IS DECREASED BELOW (11' - 6 3/4") AT THE GYM FLOOR LEVEL, IN THE AREA BOUNDED BY GRID (DD) - (MM) AND (22) - (27). ELEVATION IS TO BE DETERMINED BASED ON THICKNESS OF GYM FLOORING SYSTEM, UNLESS NOTED OTHERWISE AS (X' - X'), (+/-X' - X'), OR HI/LO.
- FRAMING ELEVATIONS ARE BASED ON TOP-OF-FLOOR ELEVATION = (120' - 0"). SHOWN ON THE ARCHITECTURAL DRAWINGS.
- COORDINATE ALL ELEVATIONS WITH ARCHITECTURAL DRAWINGS.
- SLOPE STEEL UNIFORMLY TO COLUMN ELEVATIONS SHOWN ON PLANS, UNLESS NOTED OTHERWISE.

FIREPROOFING NOTES:

- STEEL COLUMNS SHOWN ON THIS DRAWING SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING UNLESS INDICATED AS A ROUND HSS COLUMN OR BUILT-UP COLUMN ALONG COLUMN GRID BB.1. EXPOSED TO VIEW PORTION OF ROUND HSS COLUMNS SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING. CONCEALED FROM VIEW PORTION OF ROUND HSS COLUMN SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING. BUILT-UP COLUMNS ALONG GRID BB.1 SHALL RECEIVE 2-HOUR RATING BY INTUMESCENT MASTIC FIREPROOFING.
- STEEL BEAMS AND KICKERS SHOWN ON THIS DRAWING, SUPPORTING UPPER LEVEL FLOORS SHALL RECEIVE 2-HOUR RATING BY CEMENTITIOUS FIREPROOFING.
- STEEL BEAMS AND KICKERS SHOWN ON THIS DRAWING, SUPPORTING ROOF SHALL RECEIVE 1-HOUR RATING BY CEMENTITIOUS FIREPROOFING. METAL ROOF DECK TO RECEIVE 1-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
- CONCEALED FROM VIEW BRACED FRAMES AND HORIZONTAL HSS GIRTS SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
- EXPOSED TO VIEW BRACED FRAMES AND HORIZONTAL HSS GIRTS SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING.
- AT STAIRS 3, 5, AND 6 ALL EXPOSED TO VIEW BEAMS AND HSS TUBES SUPPORTING STAIR LANDINGS AND STRINGERS SHALL RECEIVE 2-HOUR RATING BY INTUMESCENT MASTIC FIREPROOFING. NO FIREPROOFING REQUIRED AT SUPPLEMENTAL HSS TUBES SUPPORTING ONLY CURTAINWALL.
- COORDINATE FIREPROOFING REQUIREMENTS WITH THE SPECIFICATIONS AND ARCHITECTURAL DRAWINGS.

BRACE FRAME KEY

- 0" TYP
WF
BF-X
INDICATES A BRACE FRAME ABOVE AND BELOW LEVEL
- 0" TYP
BF-X
WF
INDICATES A BRACE FRAME ABOVE LEVEL
- 0" TYP
WF
BF-X
INDICATES A BRACE FRAME BELOW LEVEL



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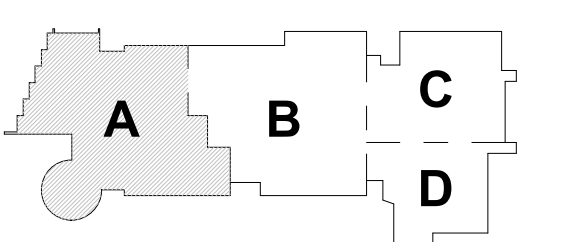
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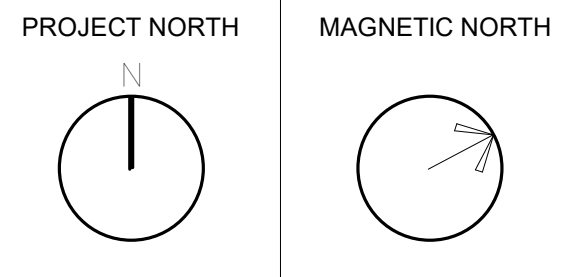
03/31/2023	EARLY STRUCTURAL BID PACKAGE
REVISION LIST	
SS-S-1	4/14/2023 STRUCTURAL STEEL ADDENDUM 1
PR-002	6/29/2023 MISCELLANEOUS STRUCTURAL REVISIONS

BID SET

August 28th, 2023



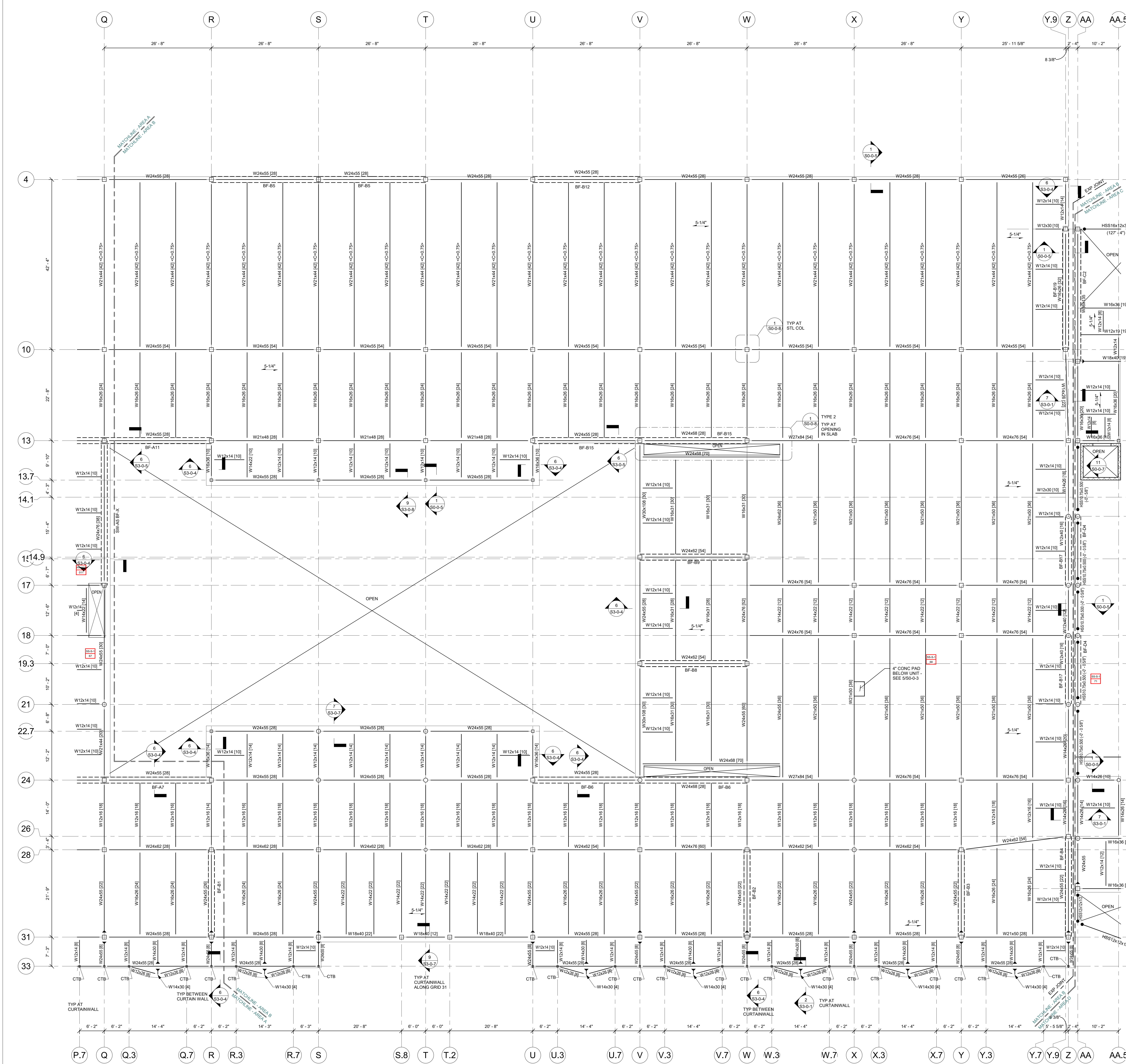
**THIRD FLOOR
FRAMING PLAN -
AREA A**



**THIRD FLOOR
FRAMING PLAN -
AREA A**

Scale: As Indicated
Job No.: 20202
Drawn By: EDG
Date: August 28th, 2023

S1-1-3A

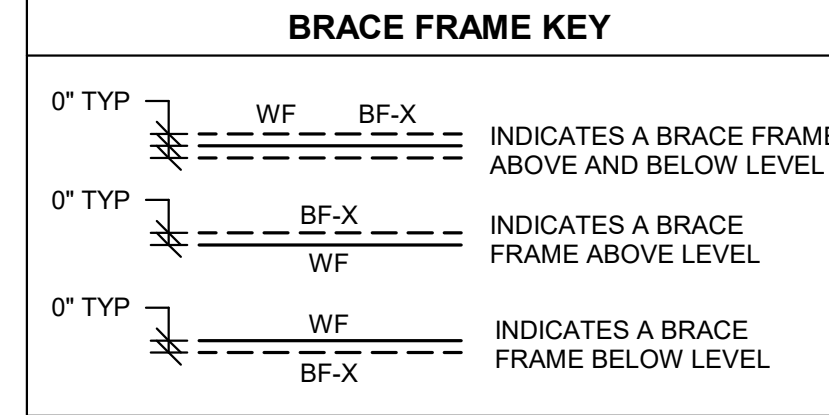


FIREPROOFING NOTES:

- STEEL COLUMNS, SHOWN ON THIS DRAWING, SHALL RECEIVE 2-HOUR FIRE RATING BY FIREPROOFING UNLESS INDICATED AS A ROUND HSS COLUMN. EXPOSED TO VIEW PORTION OF ROUND HSS COLUMNS SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING. CONCEALED FROM VIEW PORTION OF ROUND HSS COLUMN SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
- STEEL BEAMS AND KICKERS SHOWN ON THIS DRAWING, SUPPORTING UPPER LEVEL FLOORS SHALL RECEIVE 2-HOUR RATING BY CEMENTITIOUS FIREPROOFING.
- STEEL BEAMS AND KICKERS SHOWN ON THIS DRAWING, SUPPORTING ROOF SHALL RECEIVE 1-HOUR RATING BY CEMENTITIOUS FIREPROOFING. METAL ROOF DECK TO RECEIVE 1-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
- CONCEALED FROM VIEW BRACED FRAMES SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
- EXPOSED TO VIEW BRACED FRAMES SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING.
- COORDINATE FIREPROOFING REQUIREMENTS WITH THE SPECIFICATIONS AND ARCHITECTURAL DRAWINGS.

FRAMING ELEVATION NOTES:

- TYPICAL UNDERSIDE-OF-DECK ELEVATION = (13' - 6 3/4") AT THE THIRD FLOOR LEVEL IN THE AREA BOUNDED BY GRIDS (Q) - (Z) AND (4) - (33).
- FRAMING ELEVATIONS ARE BASED ON TOP-OF-FLOOR ELEVATION = (134' - 0"), SHOWN ON THE ARCHITECTURAL DRAWINGS.
- COORDINATE ALL ELEVATIONS WITH ARCHITECTURAL DRAWINGS.
- SLOPE STEEL UNIFORMLY TO COLUMN ELEVATIONS SHOWN ON PLANS.



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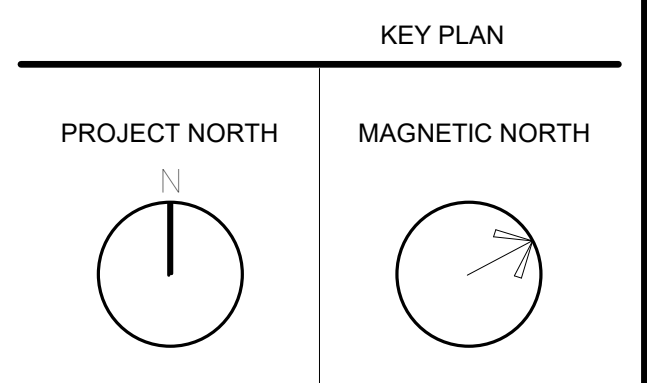
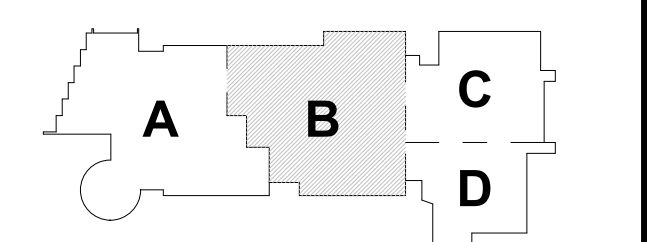
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03/31/2023	EARLY STRUCTURAL BID PACKAGE
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SS-S-1	4/14/2023 STRUCTURAL STEEL ADDENDUM 1
PR-002	6/29/2023 MISCELLANEOUS STRUCTURAL REVISIONS

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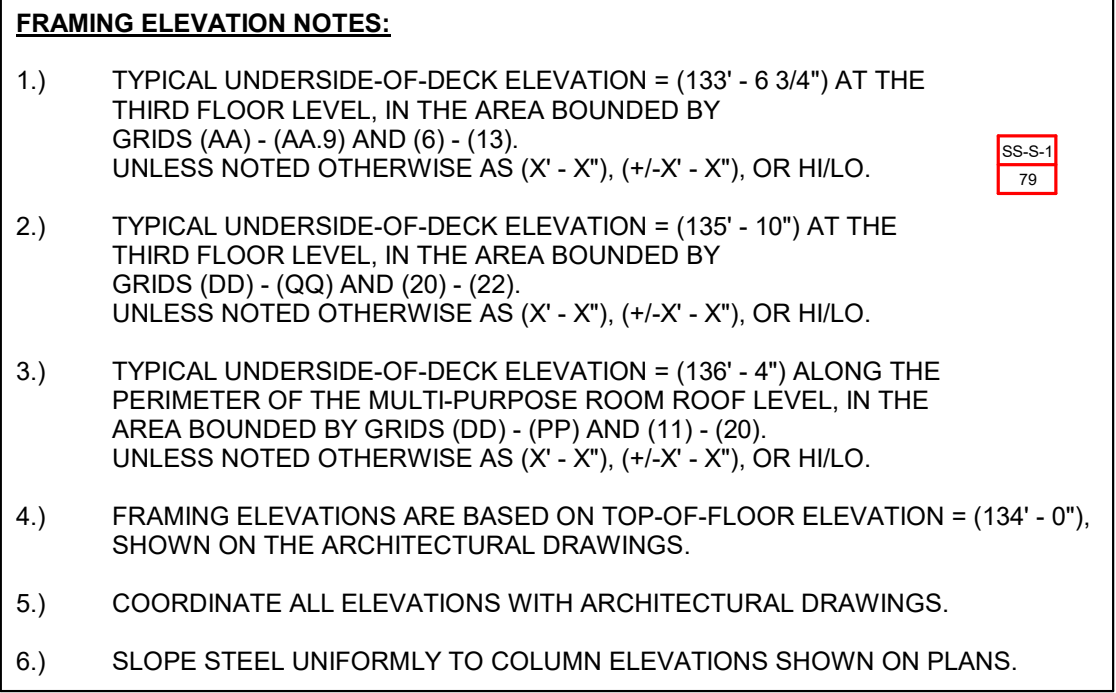
August 28th, 2023



THIRD FLOOR FRAMING PLAN - AREA B

Scale: 1/8" = 1'-0"
Job No.: 20202
Drawn By: EDG
Date: August 28th, 2023

S1-1-3B



1. STEEL COLUMNS SHOWN ON THIS DRAWING SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING UNLESS INDICATED AS A ROUND HSS COLUMN EXPOSED TO FIRE BY INTUMESCENT MASTIC FIREPROOFING. CONCEALED FROM VIEW PORTION OF ROUND HSS COLUMN SHALL RECEIVE 1-HOUR RATING BY CEMENTITIOUS FIREPROOFING.
2. STEEL BEAMS AND KICKERS SHOWN ON THIS DRAWING, SUPPORTING UPPER LEVEL FLOORS SHALL RECEIVE 2-HOUR RATING BY CEMENTITIOUS FIREPROOFING.
3. STEEL BEAMS AND KICKERS SHOWN ON THIS DRAWING, SUPPORTING ROOF SHALL RECEIVE 1-HOUR RATING BY CEMENTITIOUS FIREPROOFING. METAL ROOF DECK TO RECEIVE 1-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
4. IN MULTIPURPOSE JOISTS BETWEEN GRIDS DD AND MM (INCLUDING MM) SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING. REMAINING JOIST TO RECEIVE 1-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
5. CONCEALED FROM VIEW BRACED FRAMES AND HORIZONTAL HSS GIRTS SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
6. EXPOSED TO VIEW BRACED FRAMES AND HORIZONTAL HSS GIRTS SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING.
7. AT STAIRS, LANDINGS & ALL EXPOSED TO VIEW BEAMS AND HSS TUBES SUPPORTING FOUR LANSINGS AND STAIRS SHALL RECEIVE 2-HOUR RATING BY INTUMESCENT MASTIC FIREPROOFING. NO FIREPROOFING REQUIRED AT SUPPLEMENTAL HSS TUBES SUPPORTING ONLY CURTAIN WALLS.
8. COORDINATE FIREPROOFING REQUIREMENTS WITH THE SPECIFICATIONS AND ARCHITECTURAL DRAWINGS.

FOR GENERAL NOTES AND TYPICAL DETAILS SEE DRAWINGS 30-1, 30-2, 30-3, 30-4, 30-5, 30-6, 30-7 AND 30-8.

2) REFER TO ARCHITECTURAL DRAWINGS FOR ELEVATIONS AND VERTICAL DIMENSIONS. EACH STUD ENDING IN AN "R" POINTS AT THE COLUMNS AND BENT BEAMS AS SHOWN ON THE ARCHITECTURAL DRAWINGS.

3) BF-1 ETC. INDICATES A BRACED BAY. REFER TO BRACED FRAME ELEVATIONS AND DETAILS ON DRAWINGS SA-1, 30-2, SA-3, 30-4, 30-5, SA-6, 30-7 AND SA-8 FOR ADDITIONAL INFORMATION.

4) [DOX] INDICATES THE NUMBER OF 3/4" DIAMETER X 4' LONG HEADED STUDS WELDED TO THE TOP FLANGE OF THE BEAM. SPACE STUDS EVENLY ALONG THE BEAM UNLESS NOTED OTHERWISE.

5) [M] INDICATES A MOMENT CONNECTION TO DEVELOP THE FULL CAPACITY OF THE MEMBER. REFER TO BEAM DETAIL 1, 4 AND 5 ON DRAWING 30-6 FOR ADDITIONAL INFORMATION.

6) [●] INDICATES A 5/16" FILLET WELD ALL AROUND. (HSS BEAM TO HSS COLUMN) WHERE BEAM DIMENSIONS EXCEED COLUMN DIMENSIONS PROVIDE 1/2" THICK STEEL PLATE CLAMPS TO ACHIEVE ALL AROUND WELD. REFER TO TYPICAL DETAIL 2 ON DRAWING 30-6 FOR ADDITIONAL INFORMATION.

7) [X+X] INDICATES UPWARD CAMBER AT THE MID-SPAN OF THE MEMBER.

8) [↔] INDICATES SPAN DIRECTION OF 7" DEEP, 20 GAUGE GALVANIZED COMPOSITE STEEL DECK WITH 3 1/4" LIGHT WEIGHT CONCRETE TOPPING. THICKNESS + 5 1/4" REINFORCE WITH 6# @ W2 INWX2 1 WWR.

9) [↔] INDICATES SPAN DIRECTION OF 1 1/2" DEEP, 20 GAUGE TYPE B, GALVANIZED STEEL ROOF DECK.

10) [↔] INDICATES SPAN DIRECTION OF 3" DEEP, 20 GAUGE TYPE NA, GALVANIZED ACOUSTIC STEEL ROOF DECK.

11) [↔] INDICATES SPAN DIRECTION OF 1 1/2" DEEP, 20 GAUGE GALVANIZED COMPOSITE STEEL DECK WITH 1 1/2" LIGHT WEIGHT CONCRETE TOPPING. THICKNESS + 4" REINFORCE WITH 6# @ W2 INWX2 1 WWR.

12) FOR EXACT NUMBER, SIZE, AND LOCATION OF OPENING IN STEEL DECKING REFER TO MEMBERS AND DRAWING 30-6 FOR ADDITIONAL INFORMATION. REFER TO DETAILS 1, 2 AND 3 ON DRAWING 30-8.

13) [Hatched Area] HATCHED AREA INDICATES LOCATION OF CONCRETE SLAB WITH 3" DEEP, 18 GAUGE GALVANIZED COMPOSITE STEEL DECK WITH 4" NORMAL WEIGHT CONCRETE TOPPING. THICKNESS + 5" REINFORCE WITH 6# @ W2 INWX2 1 WWR. REFER TO TYPICAL DETAIL 5 ON DRAWING 30-7 FOR ADDITIONAL INFORMATION.

14) [○] INDICATES A ROOF DRAIN. REFER TO TYPICAL STRUCTURAL DETAILS 1 AND 2 ON DRAWING 30-8 AND DETAIL 1 ON DRAWING 30-9, FOR DECKING SUPPORT. REFER TO DETAIL 8 ON DRAWING 30-7, REFER TO PLUMBING AND MECHANICAL DRAWINGS FOR OPENING SIZES AND LOCATIONS.

15) [CT] INDICATES A COLUMN TERMINATES AT THIS LEVEL.

16) [Y] INDICATES A BEND IN THE STEEL BEAM. REFER TO TYPICAL DETAIL 9 ON DRAWING 30-7.

17) [OR] INDICATES A CMU WALL. REFER TO TYPICAL DETAIL 8 ON DRAWING 30-6 FOR REINFORCEMENT. DETAIL 6 ON DRAWING 30-6 FOR CONNECTION OF SHIM WALLS TO BEAM. DETAIL 7 ON DRAWING 30-6 FOR CONNECTIONS TO STEEL BEAMS AND CONCRETE SLABS. DETAIL 17 FOR OPENING WALLS AND NON-STRUCTURAL WALLS. REFER TO RELEVANT SECTIONS FOR CONNECTIONS OF STEEL PLANK WALLS TO THE STRUCTURE.

18) [P] PLANK INDICATES SPAN OF 8" DEEP PRESTRESSED, PRECAST CONCRETE TOPPING PLANK WITH A MINIMUM 2" OF NORMAL WEIGHT CONCRETE TOPPING. PLANK AND CONNECTIONS TO TOPPING SLAB TO BE DESIGNED TO SUPPORT A MINIMUM LIVE LOAD CAPACITY OF 150 PSF. PRECAST CONCRETE PLANK TO BE DESIGNED FOR MINIMUM OF 2 HOUR FIRE RATING.

19) [P+Z] PLANK INDICATES SPAN OF 8" DEEP PRESTRESSED, PRECAST CONCRETE TOPPING CORREL CONCRETE PLANK WITH A MINIMUM 2" OF NORMAL WEIGHT CONCRETE TOPPING. PLANK AND CONNECTIONS TO TOPPING SLAB TO BE DESIGNED TO SUPPORT A MINIMUM LIVE LOAD CAPACITY OF 150 PSF. PRECAST CONCRETE PLANK TO BE DESIGNED FOR MINIMUM OF 2 HOUR FIRE RATING.

20) FOR DIMENSIONS AND ELEVATIONS NOT GIVEN REFER TO ARCHITECTURAL DRAWINGS.

21) [YF] INDICATES A STEEL CONNECTION ALONG A CONTINUOUS STEEL BEAM. DESIGN STEEL CONNECTION FOR FULL CAPACITY OF STEEL BEAM.

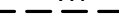
22) [mmmm] INDICATES A SLAB DEPRESSION. REFER TO DETAILS 1 ON DRAWING 30-6.

23) [↔] INDICATES SPAN DIRECTION OF 8" DEEP, 20 GAUGE GALVANIZED STEEL ROOF DECK.

24) [XOXOX] INDICATES OPENING IN WEB OF STEEL BEAM FOR MECH/ELECTR/PLUMB/FIRE PROTECT ETC COORDINATE WITH APPLICABLE DRAWING FOR OPENING SIZE. SEE TYPICAL DETAIL 5 AND 6 ON 30-4 FOR ADDITIONAL INFORMATION.

25) [↔] INDICATES SPAN DIRECTION OF 3" DEEP, 20 GAUGE TYPE N, GALVANIZED STEEL ROOF DECK.

26) STEEL BEAMS ARE EQUALLY SPACED BETWEEN COLUMN GRIDLINES. UNLESS NOTED OTHERWISE.

	INDICATES A BRACE FRAME ABOVE AND BELOW LEVEL
	INDICATES A BRACE FRAME ABOVE LEVEL
	INDICATES A BRACE FRAME BELOW LEVEL

S1-1-3C

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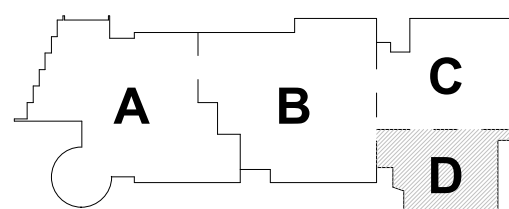


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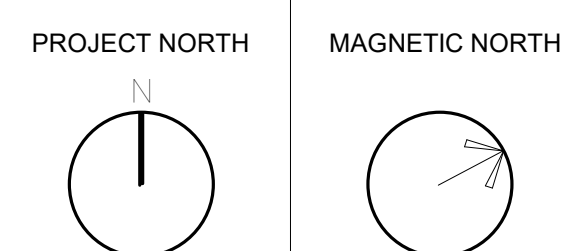
03/31/2023	EARLY STRUCTURAL BID PACKAGE
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SS-S-1	4/14/2023 STRUCTURAL STEEL ADDENDUM 1
SS-S-2	4/21/2023 STRUCTURAL STEEL ADDENDUM 2
PR-001	5/23/2023 DECK TYPE REVISION - MULTI-PURPOSE ROOM

BID SET

August 28th, 2023



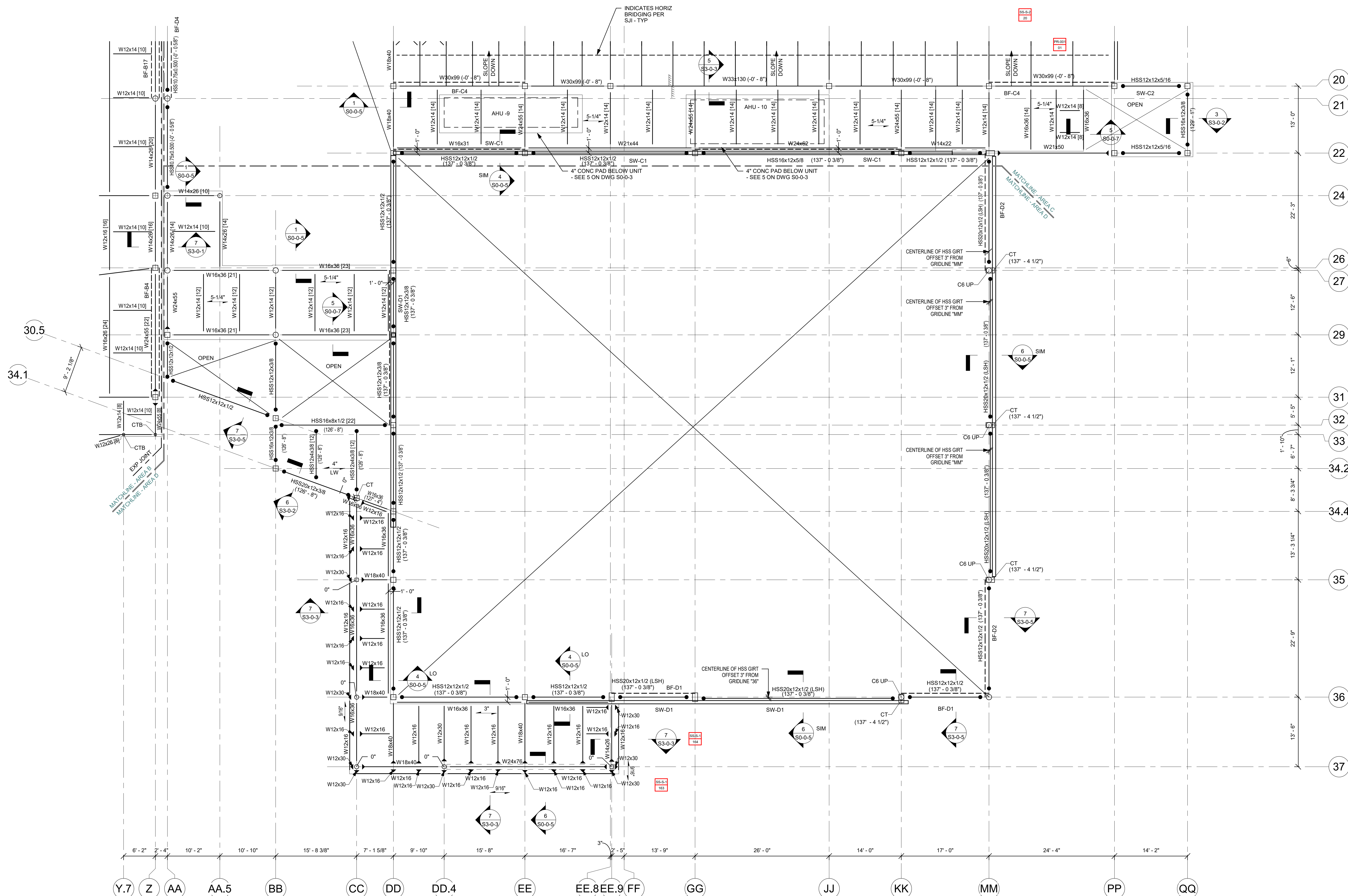
KEY PLAN



THIRD FLOOR FRAMING PLAN - AREA D

Scale: 1/8" = 1'-0"
Job No.: 20202
Drawn By: EDG
Date: August 28th, 2023

S1-1-3D



- FIREPROOFING NOTES:**
- STEEL COLUMNS SHOWN ON THIS DRAWING SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING UNLESS INDICATED AS A ROUND HSS COLUMN. EXPOSED TO VIEW PORTION OF ROUND HSS COLUMNS SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING. CONCEALED FROM VIEW PORTION OF ROUND HSS COLUMN SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
 - STEEL BEAMS AND KICKERS SHOWN ON THIS DRAWING, SUPPORTING UPPER LEVEL FLOORS SHALL RECEIVE 2-HOUR RATING BY CEMENTITIOUS FIREPROOFING.
 - STEEL BEAMS AND KICKERS SHOWN ON THIS DRAWING, SUPPORTING ROOF SHALL RECEIVE 1-HOUR RATING BY CEMENTITIOUS FIREPROOFING. METAL ROOF DECK TO RECEIVE 1-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
 - CONCEALED FROM VIEW BRACED FRAMES AND HORIZONTAL HSS GIRTS SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
 - EXPOSED TO VIEW BRACED FRAMES AND HORIZONTAL HSS GIRTS SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING.
 - AT STAIRS 3 AND 6, ALL EXPOSED TO VIEW BEAMS AND HSS TUBES SUPPORTING STAIR LANDINGS AND STRINGERS SHALL RECEIVE 2-HOUR RATING BY INTUMESCENT MASTIC FIREPROOFING. NO FIREPROOFING REQUIRED AT SUPPLEMENTAL HSS TUBES SUPPORTING ONLY CURTAINWALL.
 - EXPOSED TO VIEW COLUMNS AND HORIZONTAL HSS TUBES IN GYMNASIUM AND SUPPORTING CURTAINWALL CW2 SHALL RECEIVE 2-HOUR RATING BY INTUMESCENT MASTIC FIREPROOFING.
 - COORDINATE FIREPROOFING REQUIREMENTS WITH THE SPECIFICATIONS AND ARCHITECTURAL DRAWINGS.

- FRAMING ELEVATION NOTES:**
- TYPICAL UNDERSIDE-OF-DECK ELEVATION = (133' - 6 3/4") AT THE THIRD FLOOR LEVEL. IN THE AREA BOUNDED BY GRIDS (AA) - (DD) AND (24) - (34.4), OR HILO, UNLESS NOTED OTHERWISE AS (X' - X'), (Y' - Y'), OR HILO.
 - TYPICAL UNDERSIDE-OF-DECK ELEVATION = (129' - 0") AT THE LOW ROOF LEVEL. IN THE AREA BOUNDED BY GRIDS (CC) - (FF) AND (34.4) - (37), UNLESS NOTED OTHERWISE AS (X' - X'), (Y' - Y'), OR HILO.
 - FRAMING ELEVATIONS ARE BASED ON TOP-OF-FLOOR ELEVATION = (134' - 0"), SHOWN ON THE ARCHITECTURAL DRAWINGS.
 - COORDINATE ALL ELEVATIONS WITH ARCHITECTURAL DRAWINGS.
 - SLOPE STEEL UNIFORMLY TO COLUMN ELEVATIONS SHOWN ON PLANS.

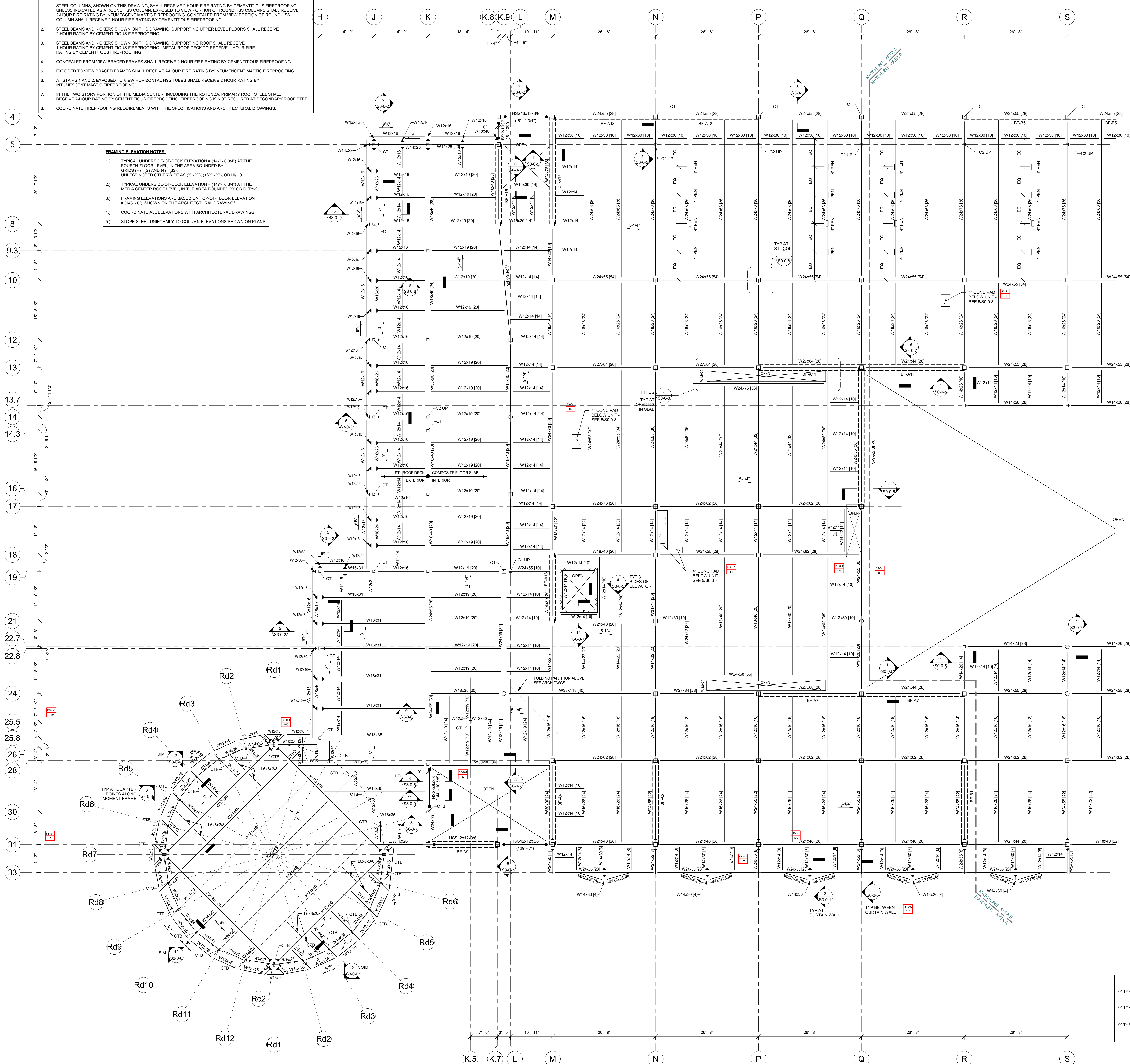
BRACE FRAME KEY	
0' TYP	INDICATES A BRACE FRAME ABOVE AND BELOW LEVEL
0' TYP	INDICATES A BRACE FRAME ABOVE LEVEL
0' TYP	INDICATES A BRACE FRAME BELOW LEVEL

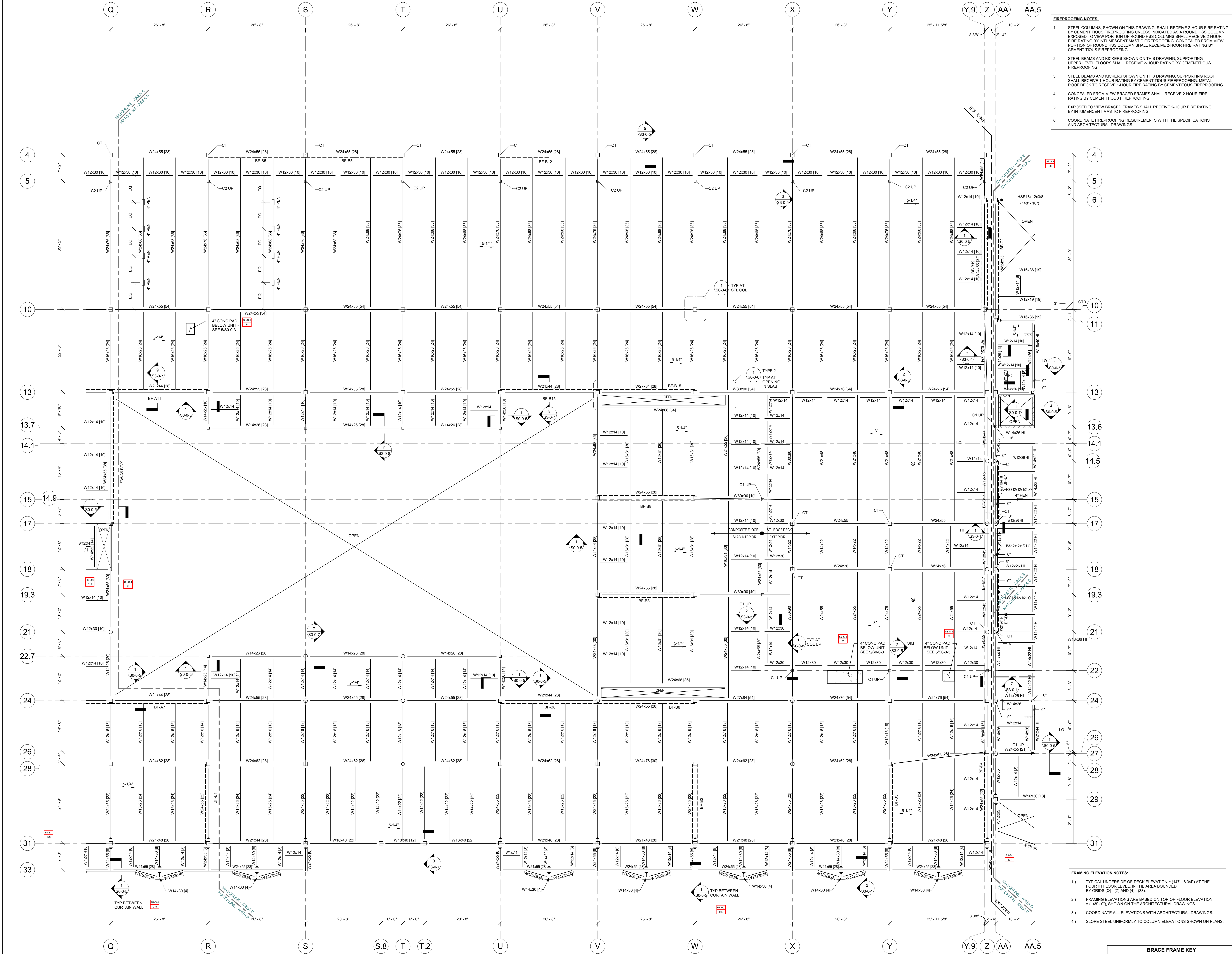
FIREPROOFING NOTES:

- STEEL COLUMNS SHOWN ON THIS DRAWING, SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING UNLESS INDICATED AS A ROUND HSS COLUMN. EXPOSED TO VIEW PORTION OF ROUND HSS COLUMNS SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING. CONCEALED FROM VIEW PORTION OF ROUND HSS COLUMN SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
- STEEL BEAMS AND KICKERS SHOWN ON THIS DRAWING, SUPPORTING UPPER LEVEL FLOORS SHALL RECEIVE 2-HOUR RATING BY CEMENTITIOUS FIREPROOFING.
- STEEL BEAMS AND KICKERS SHOWN ON THIS DRAWING, SUPPORTING ROOF SHALL RECEIVE 1-HOUR RATING BY CEMENTITIOUS FIREPROOFING. METAL ROOF DECK TO RECEIVE 1-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
- CONCEALED FROM VIEW BRACED FRAMES SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
- EXPOSED TO VIEW BRACED FRAMES SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING.
- AT STAIRS 1 AND 2, EXPOSED TO VIEW HORIZONTAL HSS TUBES SHALL RECEIVE 2-HOUR RATING BY INTUMESCENT MASTIC FIREPROOFING.
- IN THE TWO STORY PORTION OF THE MEDIA CENTER, INCLUDING THE ROTUNDA, PRIMARY ROOF STEEL SHALL RECEIVE 2-HOUR RATING BY CEMENTITIOUS FIREPROOFING. FIREPROOFING IS NOT REQUIRED AT SECONDARY ROOF STEEL.
- COORDINATE FIREPROOFING REQUIREMENTS WITH THE SPECIFICATIONS AND ARCHITECTURAL DRAWINGS.

FRAMING ELEVATION NOTES:

- TYPICAL UNDERSIDE-OF-DECK ELEVATION = (147' - 6 3/4") AT THE FOURTH FLOOR LEVEL. IN THE AREA BOUNDED BY GRIDS (H) - (S) AND (4) - (33), UNLESS NOTED OTHERWISE AS (X'-Y'), (+X'-Y'), OR (H/L).
- TYPICAL UNDERSIDE-OF-DECK ELEVATION = (147' - 6 3/4") AT THE MEDIA CENTER ROOF LEVEL. IN THE AREA BOUNDED BY GRID (R2).
- FRAMING ELEVATIONS ARE BASED ON TOP-OF-FLOOR ELEVATION = (148' - 0"), SHOWN ON THE ARCHITECTURAL DRAWINGS.
- COORDINATE ALL ELEVATIONS WITH ARCHITECTURAL DRAWINGS.
- SLOPE STEEL UNIFORMLY TO COLUMN ELEVATIONS SHOWN ON PLANS.





- FIREPROOFING NOTES:**
- STEEL COLUMNS, SHOWN ON THIS DRAWING, SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING UNLESS INDICATED AS A ROUND HSS COLUMN. EXPOSED TO VIEW PORTION OF ROUND HSS COLUMNS SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING. CONCEALED FROM VIEW PORTION OF ROUND HSS COLUMN SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
 - STEEL BEAMS AND KICKERS SHOWN ON THIS DRAWING, SUPPORTING UPPER LEVEL FLOORS SHALL RECEIVE 2-HOUR RATING BY CEMENTITIOUS FIREPROOFING.
 - STEEL BEAMS AND KICKERS SHOWN ON THIS DRAWING, SUPPORTING ROOF SHALL RECEIVE 1-HOUR RATING BY CEMENTITIOUS FIREPROOFING. METAL ROOF DECK TO RECEIVE 1-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
 - CONCEALED FROM VIEW BRACED FRAMES SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
 - EXPOSED TO VIEW BRACED FRAMES SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING.
 - COORDINATE FIREPROOFING REQUIREMENTS WITH THE SPECIFICATIONS AND ARCHITECTURAL DRAWINGS.

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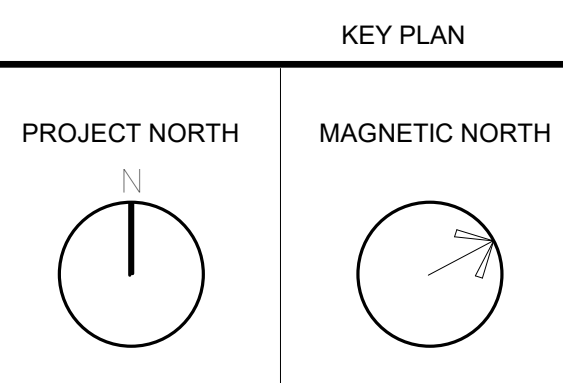
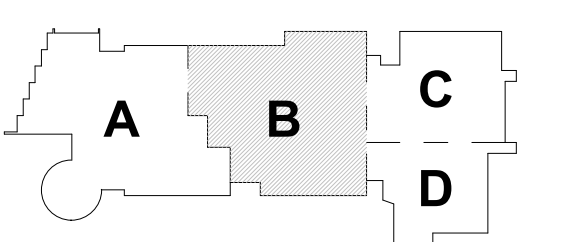
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PR-002	6/29/2023 MISCELLANEOUS STRUCTURAL REVISIONS

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BID SET

August 28th, 2023



**FOURTH FLOOR
FRAMING PLAN -
AREA B**

Scale: 1/8" = 1'-0"
Job No.: 20202
Drawn By: EDG
Date: August 28th, 2023

S1-1-4B

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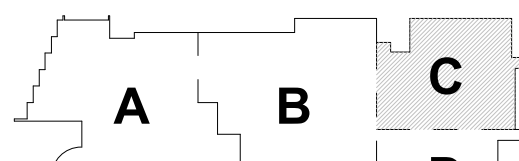


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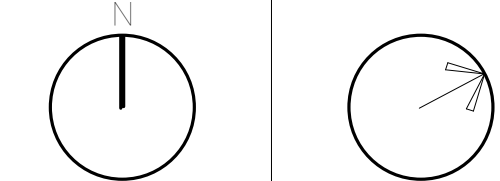
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August 28th, 2023



KEY PLAN

PROJECT NORTH MAGNETIC NORTH



FOURTH FLOOR FRAMING PLAN - AREA C

Scale: 1/8" = 1'-0"

Job No.: 20202

Drawn By: EDG

Date: August 28th, 2023

S1-1-4C

FIREPROOFING NOTES:

- STEEL COLUMNS SHOWN ON THIS DRAWING SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING UNLESS INDICATED AS A ROUND HSS COLUMN. EXPOSED TO VIEW PORTION OF ROUND HSS COLUMNS SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING. CONCEALED FROM VIEW PORTION OF ROUND HSS COLUMNS SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
- STEEL BEAMS AND KICKERS SHOWN ON THIS DRAWING, SUPPORTING UPPER LEVEL FLOORS SHALL RECEIVE 2-HOUR RATING BY CEMENTITIOUS FIREPROOFING.
- STEEL BEAMS AND KICKERS SHOWN ON THIS DRAWING, SUPPORTING ROOF SHALL RECEIVE 1-HOUR RATING BY CEMENTITIOUS FIREPROOFING. METAL ROOF DECK TO RECEIVE 1-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
- CONCEALED FROM VIEW BRACED FRAMES SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING.
- EXPOSED TO VIEW BRACED FRAMES SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING.
- AT STAIRS 4 AND 7, ALL EXPOSED TO VIEW STRUCTURE SHALL RECEIVE 2-HOUR RATING BY INTUMESCENT MASTIC FIREPROOFING.
- COORDINATE FIREPROOFING REQUIREMENTS WITH THE SPECIFICATIONS AND ARCHITECTURAL DRAWINGS.

FRAMING NOTES:

- FOR GENERAL NOTES AND TYPICAL DETAILS SEE DRAWINGS S0-0-1, S0-0-2, S0-0-3, S0-0-4, S0-0-5, S0-0-6, S0-0-7 AND S0-0-8.
- REFER TO ARCHITECTURAL DRAWINGS FOR ELEVATIONS AND VERTICAL DIMENSIONS. FITCH ALL STEEL UNIFORMLY TO LOW POINTS AT THE COLUMNS AND BENT BEAMS AS SHOWN ON THE ARCHITECTURAL DRAWINGS.
- BF-1 ETC., INDICATES A BRACED BAY. REFER TO BRACED FRAME ELEVATIONS AND DETAILS ON DRAWINGS S4-0-1, S4-0-2, S4-0-3, S4-0-4, S4-0-5, S4-0-6, S4-0-7 AND S4-0-8 FOR ADDITIONAL INFORMATION.
- [X] INDICATES THE NUMBER OF 3/4" DIAMETER x 4 1/4" LONG HEADED STUDS WELDED TO THE TOP FLANGE OF THE BEAM. SPACE STUDS EVENLY ALONG THE BEAM UNLESS NOTED OTHERWISE.
- INDICATES A MOMENT CONNECTION TO DEVELOP THE FULL CAPACITY OF THE MEMBER. REFER TO TYPICAL DETAILS 7, 8 AND 9 ON DRAWING S0-0-6.
- INDICATES A 9/16" FILLET WELD ALL AROUND (HSS BEAM TO HSS COLUMN) WHERE BEAM DIMENSIONS EXCEED COLUMN DIMENSIONS PROVIDE 1/2" THICK STEEL CAP PLATE TO ACHIEVE ALL AROUND WELD. REFER TO TYPICAL DETAIL 2 ON DRAWING S0-0-7.
- < X > INDICATES UPWARD CAMBER AT THE MID-SPAN OF THE MEMBER.
- INDICATES SPAN DIRECTION OF 2" DEEP, 20 GAGE GALVANIZED COMPOSITE STEEL DECK WITH 3 1/4" LIGHT WEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 5 1/4". REINFORCE WITH 6#6 - W2 1W2 1 WWR.
- INDICATES SPAN DIRECTION OF 1 1/2" DEEP, 20 GAGE TYPE B, GALVANIZED STEEL ROOF DECK.
- INDICATES SPAN DIRECTION OF 3" DEEP, 20 GAGE TYPE NA, GALVANIZED ACOUSTIC STEEL ROOF DECK.
- INDICATES SPAN DIRECTION OF 1 1/2" DEEP, 20 GAGE GALVANIZED COMPOSITE STEEL DECK WITH 1 1/2" LIGHT WEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 4". REINFORCE WITH 6#6 - W2 1W2 1 WWR.
- FOR EXACT NUMBER, SIZE, AND LOCATION OF OPENING IN STEEL DECKING REFER TO MECHANICAL AND ARCHITECTURAL DRAWINGS. FOR FRAMING INFORMATION, REFER TO DETAILS 1, 2 AND 3 ON DRAWING S0-0-8.
- HATCHED AREA INDICATES LOCATION OF CONCRETE SLAB WITH 3" DEEP, 18 GAGE GALVANIZED COMPOSITE STEEL DECK WITH 4" NORMAL WEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 6". REINFORCE WITH 6#6 - W2 1W2 1 WWR. REFER TO TYPICAL DETAIL 5 ON DRAWING S0-0-7 FOR ADDITIONAL INFORMATION. USE 3/4" DIA x 5' LONG HEADED STUDS.
- INDICATES A ROOF DRAIN. REFER TO TYPICAL STRUCTURAL DETAILS 1 AND 2 ON DRAWING S0-0-8 AND DETAIL 1 ON DRAWING S0-0-8. FOR DECKING SUPPORT, REFER TO DETAIL 6 ON DRAWING S0-0-7. REFER TO PLUMBING AND ARCHITECTURAL DRAWINGS FOR OPENING SIZES AND LOCATIONS.
- INDICATES A COLUMN TERMINATES AT THIS LEVEL.
- INDICATES A BEND IN THE STEEL BEAM. REFER TO TYPICAL DETAIL 9 ON DRAWING S0-0-7.
- INDICATES A CMU WALL. REFER TO TYPICAL DETAIL 6 ON DRAWING S0-0-4 FOR REINFORCEMENT. DETAIL 6 ON DRAWING S0-0-4 FOR CONNECTION OF SHEAR WALLS TO BEAMS AND COLUMNS, AND DETAIL 7 ON DRAWING S0-0-5 FOR CONNECTIONS TO STEEL BEAMS AND CONCRETE SLABS AT THE TOP OF WALL FOR NON-STRUCTURAL WALLS. REFER TO RELEVANT SECTIONS FOR CONNECTIONS OF SHEAR WALLS TO THE STRUCTURE.
- PC PLANK INDICATES SPAN OF 6" DEEP PRESTRESSED, PRECAST CONCRETE HOLLOW CORE PLANK WITH A MINIMUM 2" OF NORMAL WEIGHT CONCRETE TOPPING. PLANK AND CONCRETE TOPPING SLAB TO BE DESIGNED TO SUPPORT A MINIMUM LIVE LOAD CAPACITY OF 150 PSF. PRECAST CONCRETE PLANK TO BE DESIGNED FOR MINIMUM OF 2 HOUR FIRE RATING.
- PC PLANK INDICATES SPAN OF 10" DEEP PRESTRESSED, PRECAST CONCRETE HOLLOW CORE PLANK WITH A MINIMUM 2" OF NORMAL WEIGHT CONCRETE TOPPING. PLANK AND CONCRETE TOPPING SLAB TO BE DESIGNED TO SUPPORT A MINIMUM LIVE LOAD CAPACITY OF 150 PSF. PRECAST CONCRETE PLANK TO BE DESIGNED FOR MINIMUM OF 2 HOUR FIRE RATING.
- FOR DIMENSIONS AND ELEVATIONS NOT GIVEN REFER TO ARCHITECTURAL DRAWINGS.
- INDICATES A SPLICE CONNECTION ALONG A CONTINUOUS STEEL BEAM. DESIGN SPLICE CONNECTION FOR FULL CAPACITY OF STEEL BEAM.
- INDICATES A SLAB DEPRESSION. REFER TO DETAILS 1 ON DRAWING S0-0-6.
- INDICATES SPAN DIRECTION OF 9/16" DEEP, 20 GAGE GALVANIZED STEEL ROOF DECK.
- INDICATES OPENING IN WEB OF STEEL BEAM FOR MECH/ELECTR/PLUMB/FIRE PROTECT/ETC. COORDINATE WITH APPLICABLE DRAWING FOR OPENING SIZE. SEE TYPICAL DETAIL 5 AND 6 ON S0-0-6 FOR ADDITIONAL INFORMATION.
- INDICATES SPAN DIRECTION OF 3" DEEP, 20 GAGE TYPE N, GALVANIZED STEEL ROOF DECK.
- STEEL BEAMS ARE EQUALLY SPACED BETWEEN COLUMN GRIDLINES, UNLESS NOTED OTHERWISE.

FRAMING ELEVATION NOTES:

- TYPICAL UNDERSIDE-OF-DECK ELEVATION = (147'-6 3/4") AT THE FOURTH FLOOR LEVEL, IN THE AREA BOUNDED BY GRIDS (AA) - (AA.9) AND (8) - (13) UNLESS NOTED OTHERWISE AS (X' - X'), (+/-X' - X'), OR HI/LO.
- TYPICAL UNDERSIDE-OF-DECK ELEVATION = (152'-4") AT THE CLEAR-STORY ROOF LEVEL, IN THE AREA BOUNDED BY GRIDS (AA) - (QQ) AND (10) - (22) UNLESS NOTED OTHERWISE AS (X' - X'), (+/-X' - X'), OR HI/LO.
- FRAMING ELEVATIONS ARE BASED ON TOP-OF-FLOOR ELEVATION = (148'-0"), SHOWN ON THE ARCHITECTURAL DRAWINGS.
- COORDINATE ALL ELEVATIONS WITH ARCHITECTURAL DRAWINGS.
- SLOPE STEEL UNIFORMLY TO COLUMN ELEVATIONS SHOWN ON PLANS.

BRACE FRAME KEY

- 0" TYP. --- WF --- BF-X --- INDICATES A BRACE FRAME ABOVE AND BELOW LEVEL
- 0" TYP. --- BF-X --- INDICATES A BRACE FRAME ABOVE LEVEL
- 0" TYP. --- WF --- BF-X --- INDICATES A BRACE FRAME BELOW LEVEL

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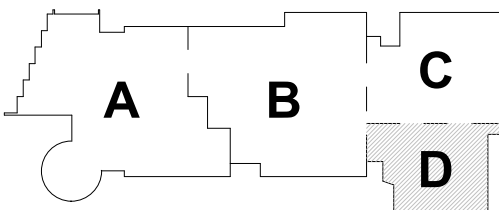
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August 28th, 2023



KEY PLAN

PROJECT NORTH

MAGNETIC NORTH

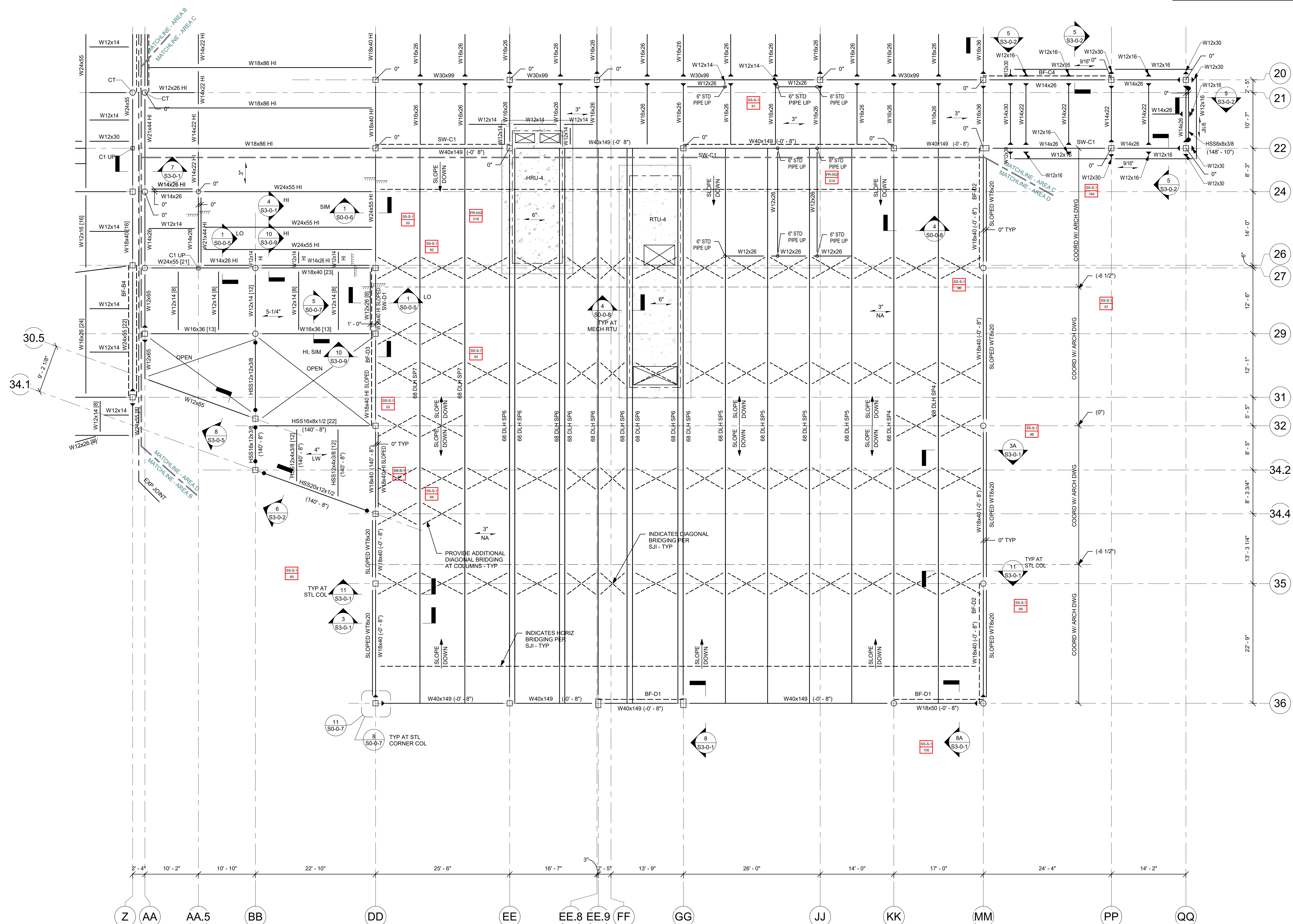
**FOURTH FLOOR
FRAMING PLAN -
AREA D**

Scale: $1/8" = 1'-0"$

Job No.: 20202

Drawn By: EDC

S1-1-4D



FRAMING ELEVATION NOTES

- 1.) TYPICAL UNDERSIDE-OF-DECK ELEVATION = $(147' - 6.34'')$ AT THE FOURTH FLOOR LEVEL, IN THE AREA BOUNDED BY GRIDS (AA), (DD) AND (24) - (34).
(UNLESS NOTED OTHERWISE AS $(X' - X'')$, $(+X' - X'')$, OR HILO.
- 2.) TYPICAL UNDERSIDE-OF-DECK ELEVATION = $(152' - 4'')$ ALONG THE PERIMETER OF THE GYM ROLL LEVEL, IN THE AREA BOUNDED BY GRIDS (D) - (MM) AND (35) - (38).
(UNLESS NOTED OTHERWISE AS $(X' - X'')$, $(+X' - X'')$, OR HILO.
- 3.) FRAMING ELEVATIONS ARE BASED ON TOP-OF-FLOOR ELEVATION = $(148' - 0'')$, SHOWN ON THE ARCHITECTURAL DRAWINGS.
- 4.) COORDINATE ALL ELEVATIONS WITH ARCHITECTURAL DRAWINGS.
- 5.) SLOPE STEEL UNIFORMLY TO COLUMN ELEVATIONS SHOWN ON PLANS

BRACE FRAME KEY

0" TYP		INDICATES A BRACE FRAME ABOVE AND BELOW LEVEL
0" TYP		INDICATES A BRACE FRAME ABOVE LEVEL
0" TYP		INDICATES A BRACE FRAME BELOW LEVEL

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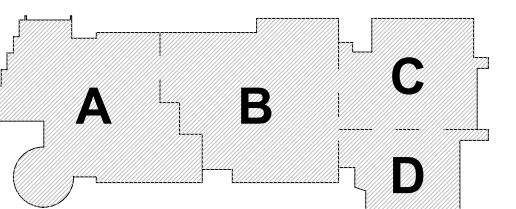
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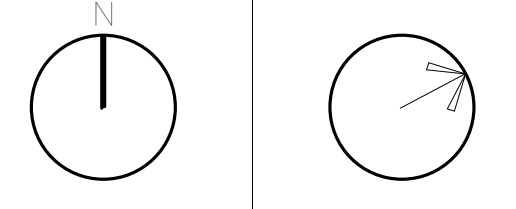
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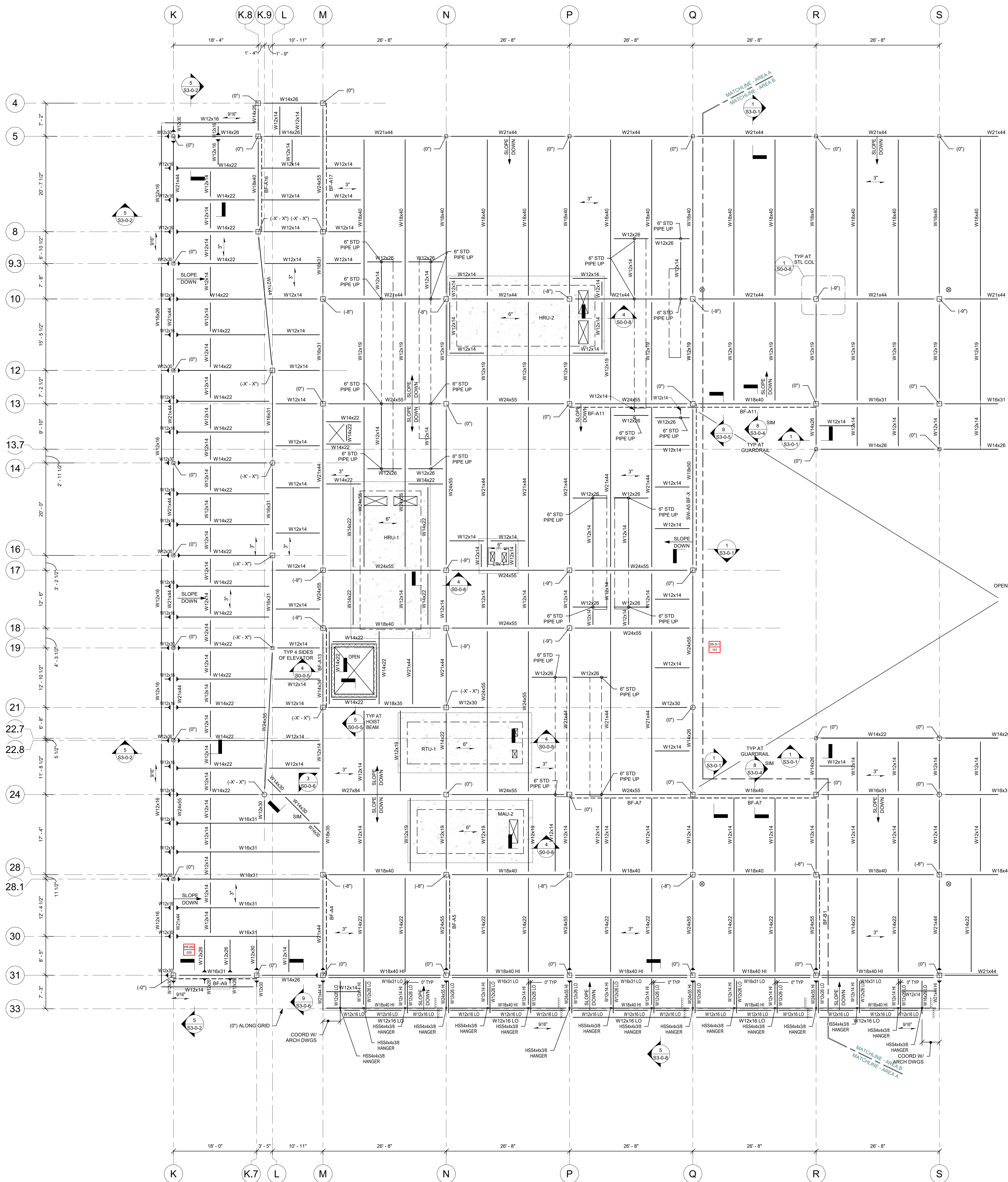
PROJECT NORTH MAGNETIC NORTH



ROOF FRAMING PLAN - AREA A

Scale: 1/8" = 1'-0"
Job No.: 20202
Drawn By: EDG
Date: August 28th, 2023

S1-1-5A



- FIREPROOFING NOTES:**
- STEEL COLUMNS, SHOWN ON THIS DRAWING, SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING UNLESS INDICATED AS A ROUND HSS COLUMN. EXPOSED TO VIEW PORTION OF ROUND HSS COLUMNS SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING. CONCEALED FROM VIEW PORTION OF ROUND HSS COLUMNS SHALL RECEIVE 1-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
 - STEEL BEAMS AND KICKERS SHOWN ON THIS DRAWING, SUPPORTING ROOF SHALL RECEIVE 1-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING. METAL ROOF DECK TO RECEIVE 1-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
 - CONCEALED FROM VIEW BRACED FRAMES SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
 - EXPOSED TO VIEW BRACED FRAMES SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING.
 - COORDINATE FIREPROOFING REQUIREMENTS WITH THE SPECIFICATIONS AND ARCHITECTURAL DRAWINGS.

FRAMING NOTES:

- FOR GENERAL NOTES AND TYPICAL DETAILS SEE DRAWINGS S0-1, S0-2, S0-3, S0-4, S0-5, S0-6, S0-7 AND S0-8.
- REFER TO ARCHITECTURAL DRAWINGS FOR ELEVATIONS AND VERTICAL DIMENSIONS. PITCH ALL STEEL UNIFORMLY TO LOW POINTS AT THE COLUMNS AND BENT BEAMS AS SHOWN ON THE ARCHITECTURAL DRAWINGS.
- BF-1 ETC. INDICATES A BRACED BAY. REFER TO BRACED FRAME ELEVATIONS AND DETAILS ON DRAWINGS S4-0-1, S4-0-2, S4-0-3, S4-0-4, S4-0-5, S4-0-6, S4-0-7 AND S4-0-8 FOR ADDITIONAL INFORMATION.
- [XX] INDICATES THE NUMBER OF 3/4" DIAMETER X 1 1/4" LONG HEADED STUDS WELDED TO THE TOP FLANGE OF THE BEAM. SPACE STUDS EVENLY ALONG THE BEAM UNLESS NOTED OTHERWISE.
- INDICATES A MOMENT CONNECTION TO DEVELOP THE FULL CAPACITY OF THE MEMBER. REFER TO TYPICAL DETAILS 7, 8 AND 9 ON DRAWING S0-6-6.
- INDICATES A 5/16" FILLET WELD ALL AROUND (HSS BEAM TO HSS COLUMN) WHERE BEAM DIMENSIONS EXCEED COLUMN DIMENSIONS. PROVIDE 1/2" THICK STEEL CAP PLATE TO ACHIEVE ALL AROUND WELD. REFER TO TYPICAL DETAIL 2 ON DRAWING S0-6-7.
- + X" INDICATES UPWARD CAMBER AT THE MID-SPAN OF THE MEMBER.
- S-1/4" INDICATES SPAN DIRECTION OF 2" DEEP, 20 GAGE GALVANIZED COMPOSITE STEEL DECK WITH 3 1/4" LIGHT WEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 5 1/4". REINFORCE WITH 6x6 - W2 1xW2 1 WWR.
- 1 1/2" INDICATES SPAN DIRECTION OF 1 1/2" DEEP, 20 GAGE TYPE B, GALVANIZED STEEL ROOF DECK.
- 3" INDICATES SPAN DIRECTION OF 3" DEEP, 20 GAGE TYPE NA, GALVANIZED ACOUSTIC STEEL ROOF DECK.
- 4" INDICATES SPAN DIRECTION OF 1 1/2" DEEP, 20 GAGE GALVANIZED COMPOSITE STEEL DECK WITH 2 1/2" LIGHT WEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 4". REINFORCE WITH 6x6 - W2 1xW2 1 WWR.
- FOR EXACT NUMBER, SIZE, AND LOCATION OF OPENING IN STEEL DECKING REFER TO MECHANICAL AND ARCHITECTURAL DRAWINGS FOR DECKING INFORMATION. REFER TO DETAILS 1, 2 AND 3 ON DRAWING S0-4-8.
- HATCHED AREA INDICATES LOCATION OF CONCRETE SLAB WITH 3" DEEP, 18 GAGE GALVANIZED COMPOSITE STEEL DECK WITH 4" NORMAL WEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 6". REINFORCE WITH 6x6 - W2 1xW2 1 WWR. REFER TO TYPICAL DETAIL 5 ON DRAWING S0-0-7 FOR ADDITIONAL INFORMATION. USE 3/4" DIA. 9" LONG HEADED STUDS.
- INDICATES A ROOF DRAIN. REFER TO TYPICAL STRUCTURAL DETAILS 1 AND 2 ON DRAWING S0-4-8 AND DETAIL 1 ON DRAWING S0-4-8. FOR DECKING SUPPORT, REFER TO DETAIL 8 ON DRAWING S0-4-7. REFER TO PLUMBING AND ARCHITECTURAL DRAWINGS FOR OPENING SIZES AND LOCATIONS.
- CT INDICATES A COLUMN TERMINATES AT THIS LEVEL.
- VF INDICATES A BEND IN THE STEEL BEAM. REFER TO TYPICAL DETAIL 9 ON DRAWING S0-0-7.
- INDICATES A CMU WALL. REFER TO TYPICAL DETAIL 8 ON DRAWING S0-4-8 FOR REINFORCEMENT. DETAIL 8 ON DRAWING S0-5-5 FOR CONNECTION OF SHEAR WALLS TO BEAMS AND COLUMNS, AND DETAIL 7 ON DRAWING S0-5-5 FOR CONNECTIONS TO STEEL BEAMS AND CONCRETE SLABS AT THE TOP OF WALL FOR NON-STRUCTURAL WALLS. REFER TO RELEVANT SECTIONS FOR CONNECTIONS OF SHEAR WALLS TO THE STRUCTURE.
- PC PLANK INDICATES SPAN OF 8" DEEP PRESTRESSED, PRECAST CONCRETE HOLLOW CORE PLANK WITH A MINIMUM 2" OF NORMAL WEIGHT CONCRETE TOPPING. PLANK AND CONCRETE TOPPING SLAB TO BE DESIGNED TO SUPPORT A MINIMUM LIVE LOAD CAPACITY OF 150 PSF. PRECAST CONCRETE PLANK TO BE DESIGNED FOR MINIMUM OF 2-HOUR FIRE RATING.
- 10" + 2" PC PLANK INDICATES SPAN OF 10" DEEP PRESTRESSED, PRECAST CONCRETE HOLLOW CORE PLANK WITH A MINIMUM 2" OF NORMAL WEIGHT CONCRETE TOPPING. PLANK AND CONCRETE TOPPING SLAB TO BE DESIGNED TO SUPPORT A MINIMUM LIVE LOAD CAPACITY OF 150 PSF. PRECAST CONCRETE PLANK TO BE DESIGNED FOR MINIMUM OF 2-HOUR FIRE RATING.
- FOR DIMENSIONS AND ELEVATIONS NOT GIVEN REFER TO ARCHITECTURAL DRAWINGS.
- WF INDICATES A SPICE CONNECTION ALONG A CONTINUOUS STEEL BEAM. DESIGN SPICE CONNECTION FOR FULL CAPACITY OF STEEL BEAM.
- INDICATES A SLAB DEPRESSION. REFER TO DETAILS 1 ON DRAWING S0-4-8.
- 9/16" INDICATES SPAN DIRECTION OF 9/16" DEEP, 20 GAGE GALVANIZED STEEL ROOF DECK.
- XX'XXX' INDICATES OPENING IN WEB OF STEEL BEAM FOR MECH/ELECT/PLUMB/FIRE PROTECT/ETC COORDINATE WITH APPLICABLE DRAWING FOR OPENING SIZE. SEE TYPICAL DETAIL 3 AND 6 ON S0-4-8 FOR ADDITIONAL INFORMATION.
- 3" INDICATES SPAN DIRECTION OF 3" DEEP, 20 GAGE TYPE N, GALVANIZED STEEL ROOF DECK.
- STEEL BEAMS ARE EQUALLY SPACED BETWEEN COLUMN GRIDLINES, UNLESS NOTED OTHERWISE.

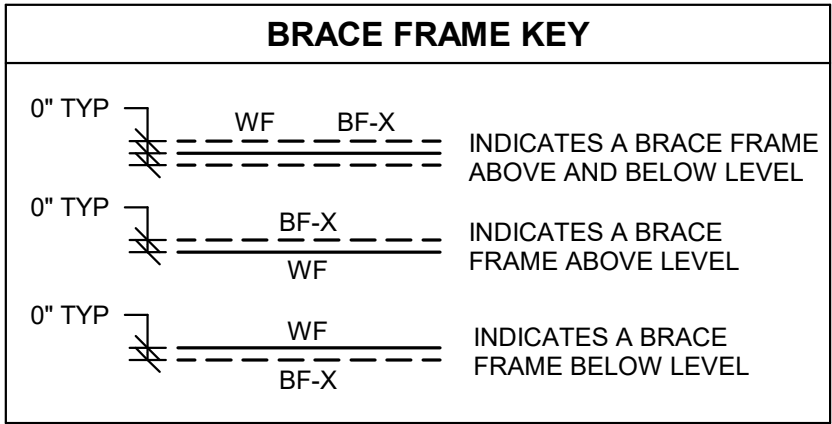
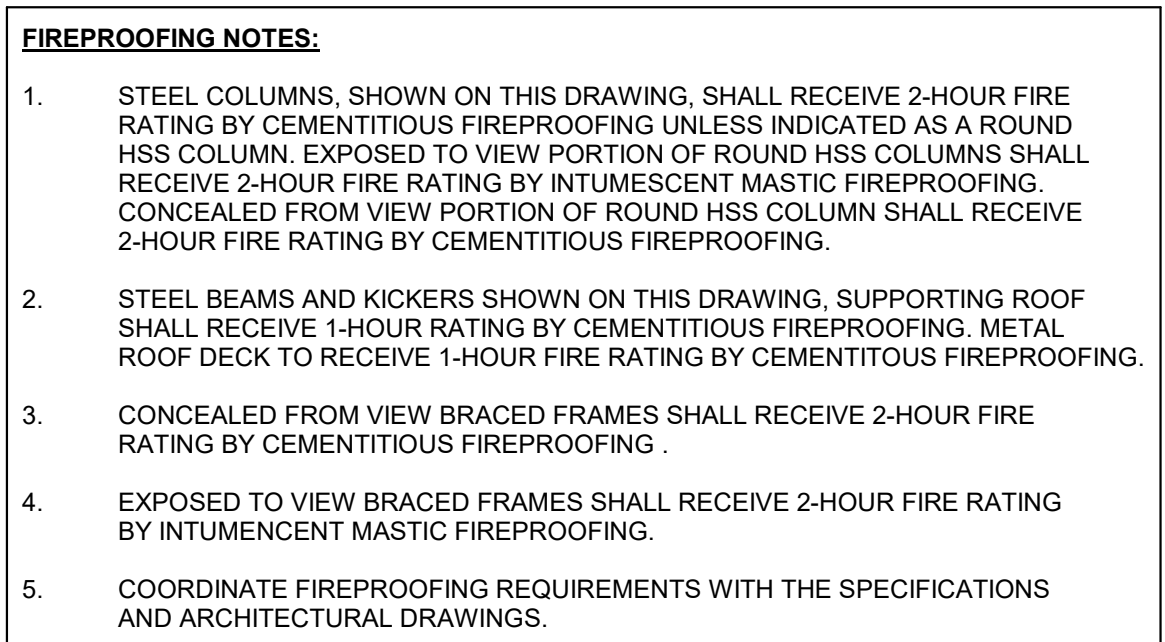
FRAMING ELEVATION NOTES:

- TYPICAL UNDERSIDE-OF-DECK ELEVATION = (162'-0") AT THE ROOF LEVEL, IN THE AREA BOUNDED BY GRIDS (K)-(S) AND (4)-(33). UNLESS NOTED OTHERWISE AS (X' - X'), (Y' - Y'), OR H/L.
- TYPICAL UNDERSIDE-OF-DECK ELEVATION = (163'-4") AT THE ELEVATOR ROOF - AREA A.
- COORDINATE ALL ELEVATIONS WITH ARCHITECTURAL DRAWINGS.
- SLOPE STEEL UNIFORMLY TO COLUMN ELEVATIONS SHOWN ON PLANS.

ELEVATOR ROOF FRAMING PART PLAN - AREA A

UD ELEV = 163' - 4"

BRACE FRAME KEY	
0' TYP	WF BF-X INDICATES A BRACE FRAME ABOVE AND BELOW LEVEL
0' TYP	BF-X WF INDICATES A BRACE FRAME ABOVE LEVEL
0' TYP	WF BF-X INDICATES A BRACE FRAME BELOW LEVEL



100 Hemlock Rd,
Wakefield, MA 01880



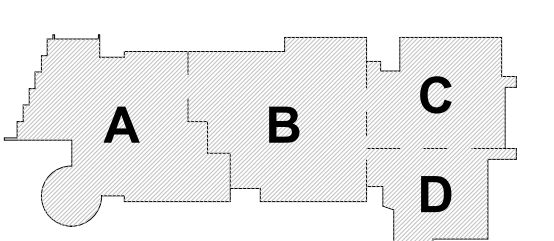
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Structural Engineers
89 Main Street, Suite 401
Malden, MA 02148
(781)396-9007
EDG@EDGINC.COM

03/31/2023	EARLY STRUCTURAL BID PACKAGE	
REVISION LIST		
SS-S-1	4/14/2023	STRUCTURAL STEEL ADDENDUM 1

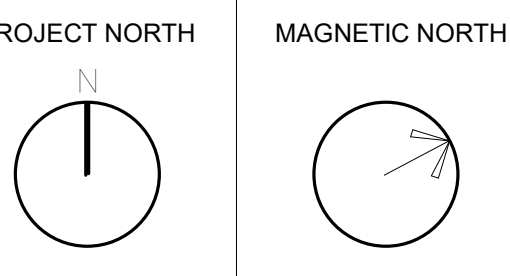
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BID SET

August 28th, 2023



KEY PLAN



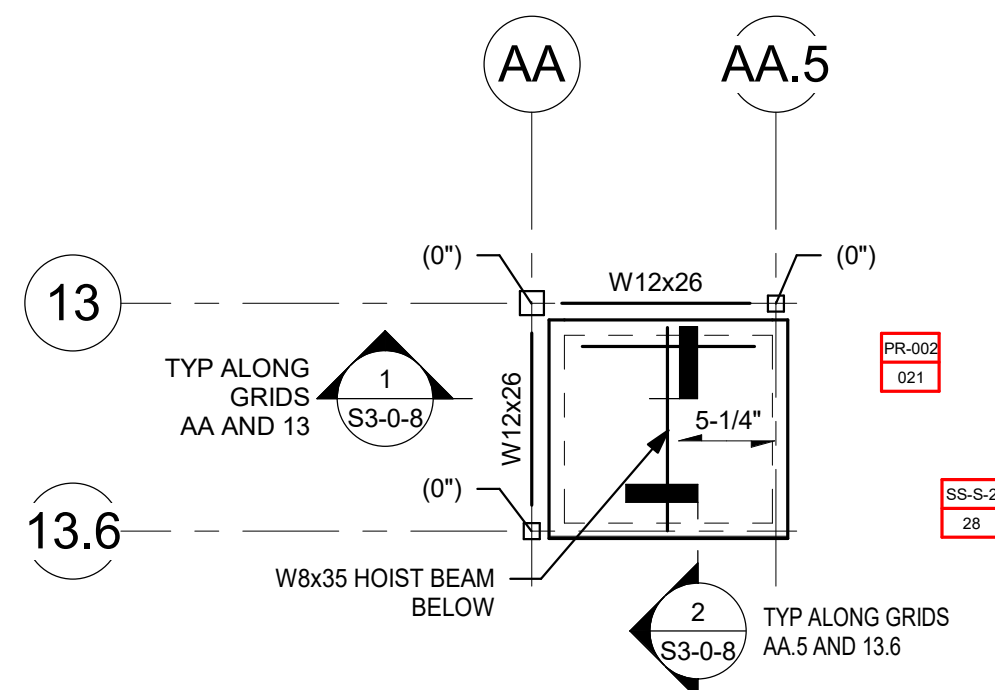
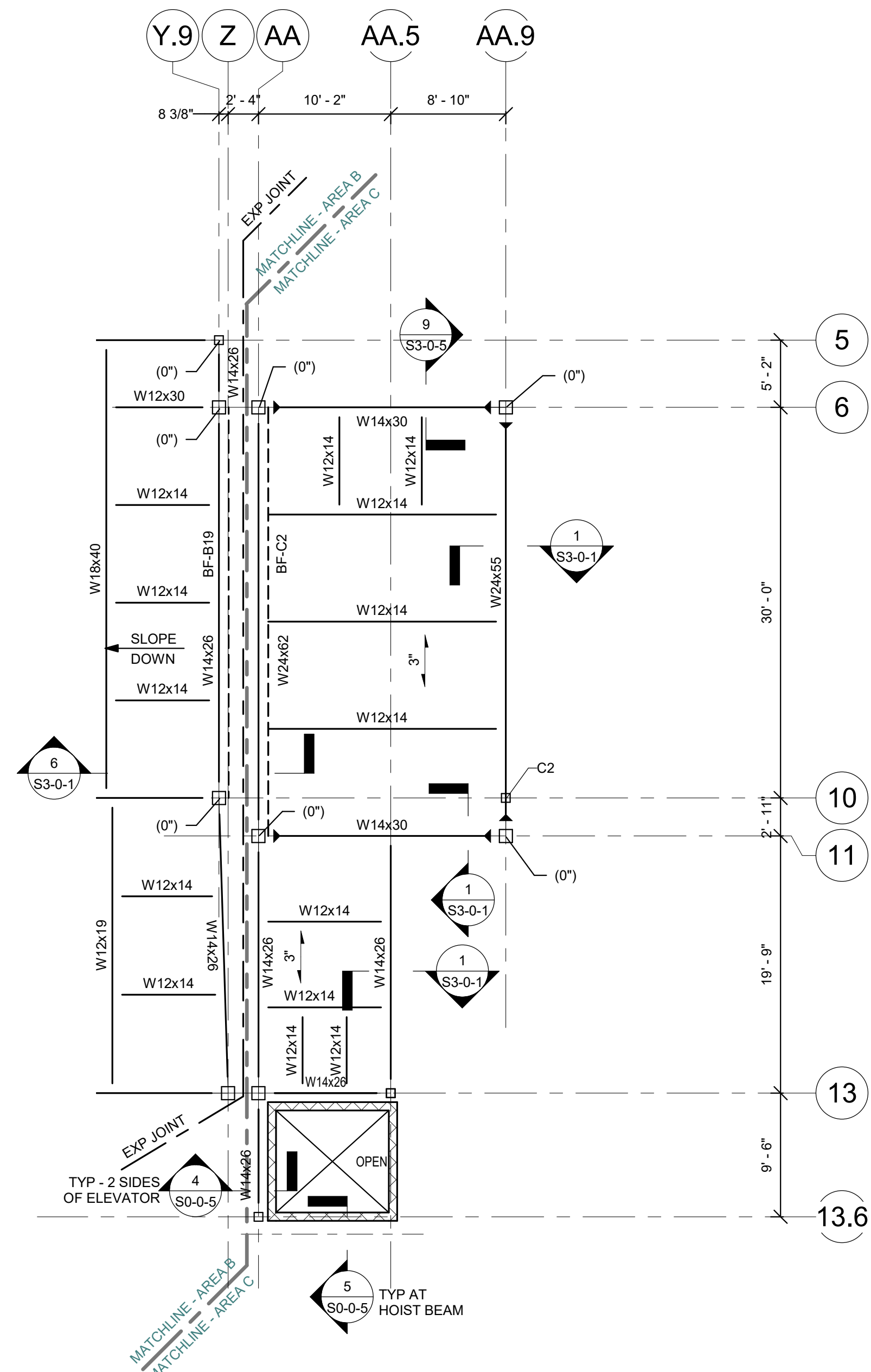
ROOF FRAMING PLAN - AREA B

FRAMING ELEVATION NOTES:

- 1.) TYPICAL UNDERSIDE-OF-DECK ELEVATION = $\{162' - 0''\}$ AT THE ROOF LEVEL, IN THE AREA BOUNDED BY GRIDS (Q) - (Z) AND (5) - (33). UNLESS NOTED OTHERWISE AS $(X' - X'')$, $(+/-X' - X'')$, OR H/L/O.
- 2.) COORDINATE ALL ELEVATIONS WITH ARCHITECTURAL DRAWINGS.
- 3.) SLOPE STEEL UNIFORMLY TO COLUMN ELEVATIONS SHOWN ON PLANS.

Scale: 1/8" = 1'-0"
Job No.: 20202
Drawn By: EDG
Date: August 28th, 2023

S1-1-5B



ELEVATOR ROOF FRAMING
PART PLAN - AREA C

U/D ELEV = 163' - 4"

FIREPROOFING NOTES

1. STEEL COLUMNS SHOWN ON THIS DRAWING SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING UNLESS INDICATED AS A ROUND HISS COLUMN. EXPOSED END PORTIONS OF ROUND HISS COLUMNS SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING. CONCEALED FROM VIEW PORTIONS OF ROUND HISS COLUMNS SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
2. STEEL BEAMS AND KICKERS SHOWN ON THIS DRAWING, SUPPORTING ROOF SHALL RECEIVE 1-HOUR RATING BY CEMENTITIOUS FIREPROOFING. METAL ROOF DECK TO RECEIVE 1-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
3. CONCEALED FROM VIEW BRACED FRAMES SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
4. EXPOSED TO VIEW BRACED FRAMES SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING.
5. COORDINATE FIREPROOFING REQUIREMENTS WITH THE SPECIFICATIONS AND ARCHITECTURAL DRAWINGS.

FRAMING NOTES:

- 1) FOR GENERAL NOTES AND TYPICAL DETAILS SEE DRAWINGS S0-0.1, S0-0.2, 0-0.3, S0-0.4, S0-0.5, S0-0.6, S0-0.7 AND S0-0.8.
- 2) REFER TO ARCHITECTURAL DRAWINGS FOR ELEVATIONS AND VERTICAL DIMENSIONS. FITCH ALL UNIFORMS AND NON UNIFORM DIMENSIONS TO THE COLUMNS AND BENT BEAMS AS SHOWN ON THE ARCHITECTURAL DRAWINGS.
- 3) BF-1 ETC. INDICATES A BRACED BAY. REFER TO BRACED FRAME ELEVATIONS AND DETAILS ON DRAWINGS S4-0.1, S4-0.2, S4-0.3, S4-0.4, S4-0.5, S4-0.6, S4-0.7 AND S4-0.8 FOR ADDITIONAL INFORMATION.
- 4) [XXX] INDICATES THE NUMBER OF 3/4" DIAMETER X 4' LONG HEADED STUDS USED TO THE TOP FLANGE OF THE BEAM. SPACE STUDS EVENLY ALONG THE BEAM UNLESS NOT OTHERWISE NOTED.
- 5)  INDICATES A MOMENT CONNECTION TO DEVELOP THE FULL CAPACITY OF THE MEMBER. REFER TO TYPICAL DETAILS 7, 8 AND 9 ON DRAWING S0-0.6.
- 6)  INDICATES A 5/16" FILLET WELD ALL AROUND (HSB BEAM TO HSS COLUMN) WHERE BEAM DIMENSIONS EXCEED 12" OR COLUMN DIMENSIONS PROVIDE 12" THICK STEEL PLATE TO ACHIEVE ALL AROUND WELD. TOPPING SHALL BE TYPICAL DETAIL 9 ON DRAWING S0-0.7 FOR ADDITIONAL INFORMATION.
- 7) < X > INDICATES UPWARD CAMBER AT THE MID-SPAN OF THE MEMBER.
- 8) 5'-11 1/2" INDICATES SPAN DIRECTION OF 7' DEEP, 20 GAUGE GALVANIZED COMPOSITE STEEL DECK WITH 3 1/4" LIGHT WEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 5' 11 1/2" REINFORCE WITH 6#6 - W2X12 1 WWR.
- 9) 1'-10 1/2" INDICATES SPAN DIRECTION OF 1' DEEP, 20 GAUGE TYPE B, GALVANIZED STEEL ROOF DECK.
- 10) 3' NA 4' LW INDICATES SPAN DIRECTION OF 3' DEEP, 20 GAUGE TYPE NA, GALVANIZED ACOUSTIC STEEL ROOF DECK.
- 11) 4' LW INDICATES SPAN DIRECTION OF 1' DEEP, 20 GAUGE GALVANIZED COMPOSITE STEEL DECK WITH 3 1/4" LIGHT WEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 4' REINFORCE WITH 6#6 - W2X12 1 WWR.
- 12) FOR EXACT NUMBER, SIZE, AND LOCATION OF OPENING IN STEEL DECKING REFER TO MECHANICAL AND ARCHITECTURAL DRAWINGS FOR TRACING INFORMATION. REFER TO DETAILS 1, 2 AND 3 ON DRAWING S0-0.8.
- 13)  6" HATCHED AREA INDICATES LOCATION OF CONCRETE SLAB WITH 3' DEEP, 18 GAUGE GALVANIZED COMPOSITE STEEL DECK WITH 4" NORMAL WEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 6" REINFORCE WITH 6#6 - W2X12 1 WWR. REFER TO TYPICAL DETAIL 5 ON DRAWING S0.07 FOR ADDITIONAL INFORMATION.
- 14)  INDICATES A ROOF DRAIN. REFER TO TYPICAL STRUCTURAL DETAILS 1 AND 2 ON DRAWING S0-0.8 AND DETAIL 1 ON DRAWING S0-0.9. FOR DECKING SUPPORT, REFER TO DETAIL 6 ON DRAWING S0-0.7. REFER TO PLUMBING AND ARCHITECTURAL DRAWINGS FOR OPENING SIZES AND LOCATIONS.
- 15) CT INDICATES A COLUMN TERMINATES AT THIS LEVEL.
- 16) WF DETAIL 9 ON DRAWING S0-0.7 INDICATES A BEND IN DRAWING S0-0.7.
- 17)  OR  INDICATES A CMU WALL. REFER TO TYPICAL DETAIL 8 ON DRAWING S0-0.4 FOR REINFORCEMENT. DETAIL 6 ON DRAWINGS S0-0.5 FOR CONNECTION OF SHEAR WALL TO COLUMN. DETAIL 7 ON DRAWING S0-0.4 FOR CONNECTIONS TO STEEL BEAMS AND CONCRETE SLABS. REFER TO ARCHITECTURAL DRAWINGS FOR RELEVANT SECTIONS FOR CONNECTIONS OF STEEL WALLS TO THE STRUCTURE.
- 18) 6" x 2" PC PLANK INDICATES SPAN OF 6" DEEP PRESTRESSED, PRECAST CONCRETE HOLLOW CORE PLANK WITH A MINIMUM 2" OF NORMAL WEIGHT CONCRETE TOPPING. PLANK AND CONNECTION TOPPING SLAB TO BE DESIGNED TO SUPPORT A MINIMUM LIVE LOAD CAPACITY OF 150 PSF. PRECAST CONCRETE PLANK TO BE DESIGNED FOR MINIMUM OF 2 HOURS FIRE RATING.
- 19) 10" x 2" PC PLANK INDICATES SPAN OF 10" DEEP PRESTRESSED, PRECAST CONCRETE HOLLOW CORE PLANK WITH A MINIMUM 2" OF NORMAL WEIGHT CONCRETE TOPPING. PLANK AND CONNECTION TOPPING SLAB TO BE DESIGNED TO SUPPORT A MINIMUM LIVE LOAD CAPACITY OF 150 PSF. PRECAST CONCRETE PLANK TO BE DESIGNED FOR MINIMUM OF 2 HOURS FIRE RATING.
- 20) FOR DIMENSIONS AND ELEVATIONS NOT GIVEN REFER TO ARCHITECTURAL DRAWINGS.
- 21) WF INDICATES A STEEL CONNECTION ALONG A CONTINUOUS STEEL BEAM. DESIGN SPEL CONNECTION FOR FULL CAPACITY OF STEEL BEAM.
- 22)  INDICATES A SLAB DEPRESSION. REFER TO DETAILS 1 ON DRAWING S0-0.6.
- 23) 9/16" INDICATES SPAN DIRECTION OF 9' DEEP, 20 GAUGE GALVANIZED STEEL ROOF DECK.
- 24)  INDICATES OPENING IN WEB OF STEEL BEAM FOR MCH/ELECT/PLUMB/FIRE PROTECT ETC. COORDINATE WITH ARCHITECTURAL DRAWINGS FOR OPENING SIZE. SEE TYPICAL DETAIL 6 AND 5 ON S0-0.6 FOR ADDITIONAL INFORMATION.
- 25) 3' WF INDICATES SPAN DIRECTION OF 3' DEEP, 20 GAUGE TYPE N, GALVANIZED STEEL ROOF DECK.
- 26)  OR  STEEL BEAMS ARE EQUALLY SPACED BETWEEN COLUMN GRIDLINES, UNLESS NOT OTHERWISE NOTED.

FRAMING ELEVATION NOTES:

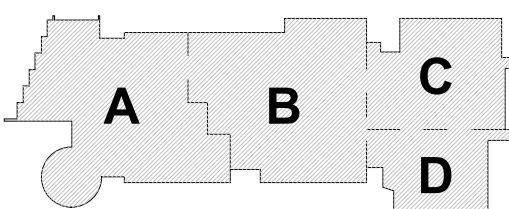
- 1.) TYPICAL UNDERSIDE-OF-DECK ELEVATION = ($162^{\circ} - 0'$) AT THE ROOF LEVEL, IN THE AREA BOUNDED BY GRIDS (AA) - (AA.9) AND (6) - (13.9) UNLESS NOTED OTHERWISE AS ($X^{\circ} - X'$), ($+X^{\circ} - X'$), OR HILO.
- 2.) TYPICAL UNDERSIDE-OF-DECK ELEVATION = ($163^{\circ} - 4'$) AT THE ELEVATOR ROOF - AREA C.
- 3.) COORDINATE ALL ELEVATIONS WITH ARCHITECTURAL DRAWINGS.
- 4.) SLOPE STEEL UNIFORMLY TO COLUMN ELEVATIONS SHOWN ON PLANS.

03/31/2023	EARLY STRUCTURAL BID PACKAGE	
REVISION LIST		
SS-S-2	4/21/2023	STRUCTURAL STEEL ADDENDUM 2
PR-002	6/29/2023	MISCELLANEOUS STRUCTURAL REVISIONS

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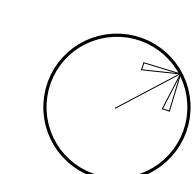
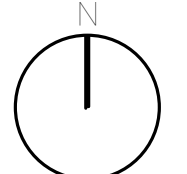
August 28th, 2023



KEY PLAN

PROJECT NORTH

MAGNETIC NORTH



ROOF FRAMING PLAN - AREA C

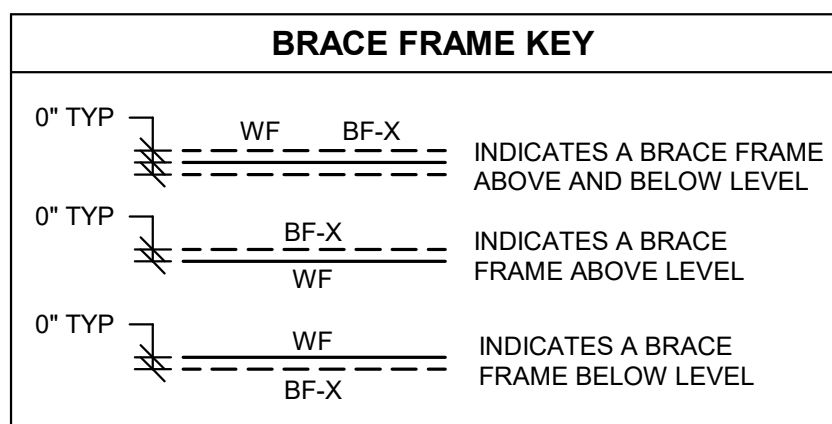
Scale: $1/8" = 1'-0"$

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Return Due 500

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S1-1-5C



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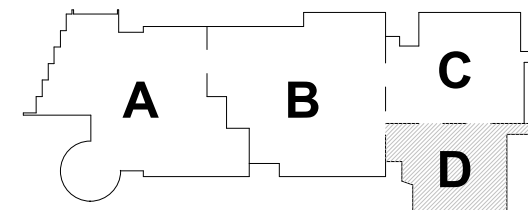


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REVISION LIST		
03/31/2023	EARLY STRUCTURAL BID PACKAGE	
SS-S-1	4/14/2023	STRUCTURAL STEEL ADDENDUM 1
SS-S-2	4/21/2023	STRUCTURAL STEEL ADDENDUM 2
PR-002	6/29/2023	MISCELLANEOUS STRUCTURAL REVISIONS

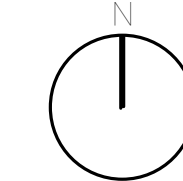
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August 28th, 2023

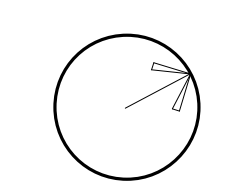


KEY PLAN

PROJECT NORTH



MAGNETIC NORTH



ROOF FRAMING
PLAN - AREA D

Scale: 1/8" = 1'-0"

Job No.: 20202

Drawn By: EDG

Date: August 28th, 2023

S1-1-5D

FIREPROOFING NOTES:

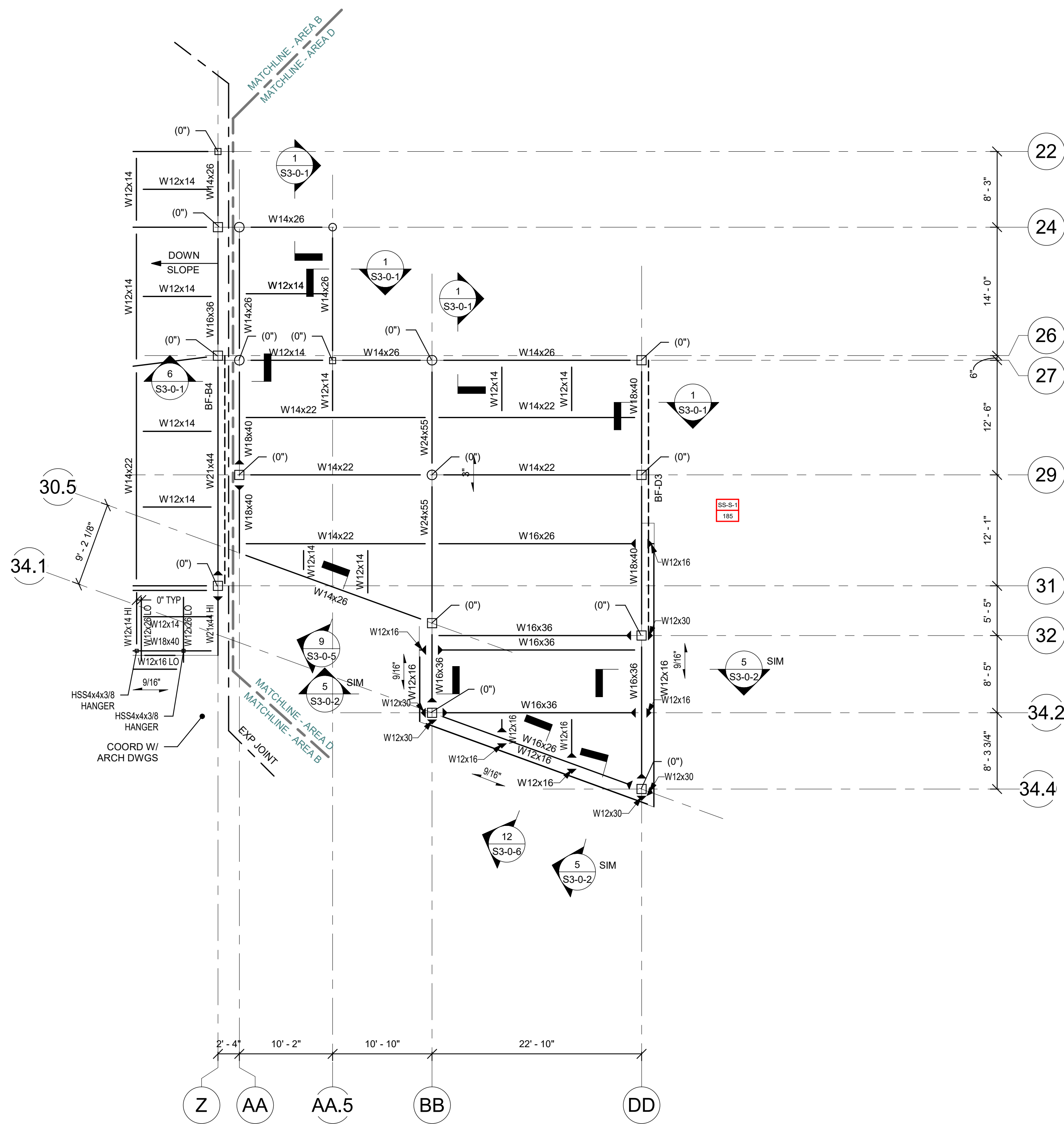
- STEEL COLUMNS SHOWN ON THIS DRAWING SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING UNLESS INDICATED AS A ROUND HSS COLUMN. EXPOSED TO VIEW PORTION OF ROUND HSS COLUMNS SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING. CONCEALED FIRE VIEW PORTION OF ROUND HSS COLUMN SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
- STEEL BEAMS AND KICKERS SHOWN ON THIS DRAWING, SUPPORTING ROOF SHALL RECEIVE 1-HOUR RATING BY CEMENTITIOUS FIREPROOFING. METAL ROOF DECK TO RECEIVE 1-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
- CONCEALED FROM VIEW BRACED FRAMES GIRTS SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
- EXPOSED TO VIEW BRACED FRAMES SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING.
- EXPOSED TO VIEW HORIZONTAL HSS TUBES SUPPORTING CURTAINWALL CW2 SHALL RECEIVE 2-HOUR RATING BY INTUMESCENT MASTIC FIREPROOFING.
- COORDINATE FIREPROOFING REQUIREMENTS WITH THE SPECIFICATIONS AND ARCHITECTURAL DRAWINGS.

FRAMING NOTES:

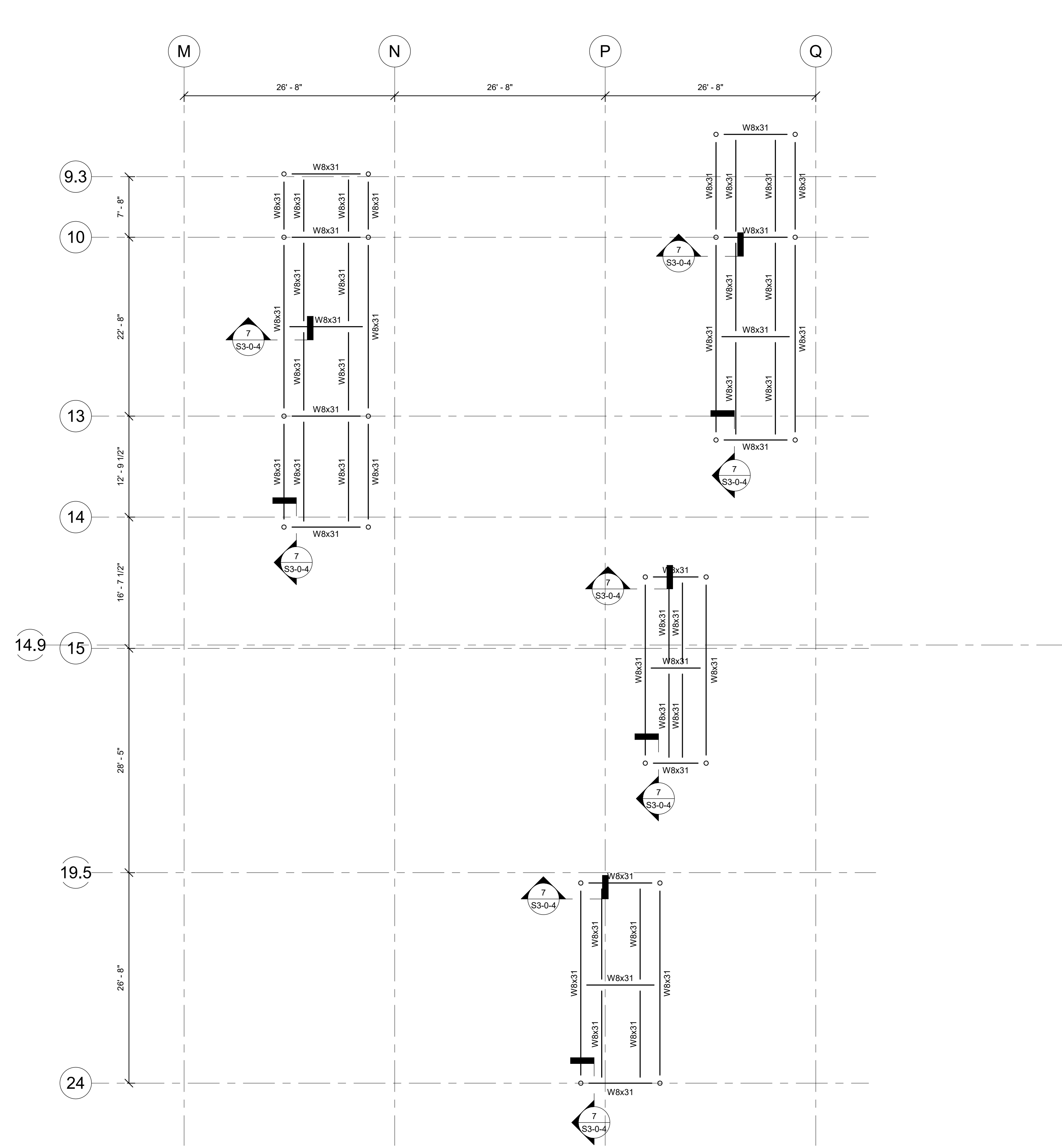
- FOR GENERAL NOTES AND TYPICAL DETAILS SEE DRAWINGS S0-0-1, S0-0-2, S0-0-3, S0-0-4, S0-0-5, S0-0-6, S0-0-7 AND S0-0-8.
- REFER TO ARCHITECTURAL DRAWINGS FOR ELEVATIONS AND VERTICAL DIMENSIONS. PITCH ALL STEEL UNIFORMLY TO LOW POINTS AT THE COLUMNS AND RENT BEAMS AS SHOWN ON THE ARCHITECTURAL DRAWINGS.
- BF-1 ETC., INDICATES A BRACED BAY. REFER TO BRACED FRAME ELEVATIONS AND DETAILS ON DRAWINGS S4-0-1, S4-0-2, S4-0-3, S4-0-4, S4-0-5, S4-0-6, S4-0-7 AND S4-0-8 FOR ADDITIONAL INFORMATION.
- [XX] INDICATES THE NUMBER OF 3/4" DIAMETER x 4 1/4" LONG HEADED STUDS WELDED TO THE TOP FLANGE OF THE BEAM. SPACE STUDS EVENLY ALONG THE BEAM UNLESS NOTED OTHERWISE.
- INDICATES A MOMENT CONNECTION TO DEVELOP THE FULL CAPACITY OF THE MEMBER. REFER TO TYPICAL DETAILS 7, 8 AND 9 ON DRAWING S0-0-4.
- INDICATES A 5/16" FILLET WELD ALL AROUND. (HSS BEAM TO HSS COLUMN) WHERE BEAM DIMENSIONS EXCEED COLUMN DIMENSIONS PROVIDE 1/2" THICK STEEL CAP PLATE TO ACHIEVE ALL AROUND WELD. REFER TO TYPICAL DETAIL 2 ON DRAWING S0-0-7.
- < X > INDICATES UPWARD CAMBER AT THE MID-SPAN OF THE MEMBER.
- 5'-1/4" INDICATES SPAN DIRECTION OF 2" DEEP, 20 GAGE GALVANIZED COMPOSITE STEEL DECK WITH 3 1/4" LIGHT WEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 5 1/4". REINFORCE WITH 6#5 - W2, 1W2, 1 WWR.
- 1'-1/2" INDICATES SPAN DIRECTION OF 1 1/2" DEEP, 20 GAGE TYPE B, GALVANIZED STEEL ROOF DECK.
- 3" NA INDICATES SPAN DIRECTION OF 3" DEEP, 20 GAGE TYPE NA, GALVANIZED ACOUSTIC STEEL ROOF DECK.
- 4" LW INDICATES SPAN DIRECTION OF 1 1/2" DEEP, 20 GAGE GALVANIZED COMPOSITE STEEL DECK WITH 2 1/2" LIGHT WEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 4". REINFORCE WITH 6#5 - W2, 1W2, 1 WWR.
- FOR EXACT NUMBER, SIZE, AND LOCATION OF OPENING IN STEEL DECKING REFER TO MECHANICAL AND ARCHITECTURAL DRAWINGS. FOR FRAMING INFORMATION, REFER TO DETAILS 1, 2 AND 3 ON DRAWING S0-0-8.
- HATCHED AREA INDICATES LOCATION OF CONCRETE SLAB WITH 3" DEEP, 18 GAGE GALVANIZED COMPOSITE STEEL DECK WITH 4" NORMAL WEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 6". REINFORCE WITH 6#5 - W2, 1W2, 1 WWR. REFER TO TYPICAL DETAIL 5 ON DRAWING S0-0-7 FOR ADDITIONAL INFORMATION. USE 3/4" DIA x 5' LONG HEADED STUDS.
- INDICATES A ROOF DRAIN. REFER TO TYPICAL STRUCTURAL DETAILS 1 AND 2 ON DRAWING S0-0-8 AND DETAIL 1 ON DRAWING S0-0-8. FOR DECKING SUPPORT, REFER TO DETAIL 6 ON DRAWING S0-0-7. REFER TO PLUMBING AND ARCHITECTURAL DRAWINGS FOR OPENING SIZES AND LOCATIONS.
- CT INDICATES A COLUMN TERMINATES AT THIS LEVEL.
- WF INDICATES A BEND IN THE STEEL BEAM. REFER TO TYPICAL DETAIL 9 ON DRAWING S0-0-7.
- INDICATES A CMU WALL. REFER TO TYPICAL DETAIL 8 ON DRAWING S0-0-4 FOR REINFORCEMENT. DETAIL 10 ON DRAWING S0-0-5 FOR CONNECTION OF SHEAR WALLS TO BEAMS AND COLUMNS, AND DETAIL 7 ON DRAWING S0-0-5 FOR CONNECTIONS TO STEEL BEAMS AND CONCRETE SLABS AT THE TOP OF WALL FOR NON-STRUCTURAL WALLS. REFER TO RELEVANT SECTIONS FOR CONNECTIONS OF SHEAR WALLS TO THE STRUCTURE.
- PC PLANK INDICATES SPAN OF 8" DEEP PRESTRESSED, PRECAST CONCRETE HOLLOW CORE PLANK WITH A MINIMUM 2" OF NORMAL WEIGHT CONCRETE TOPPING. PLANK AND CONCRETE TOPPING SLAB TO BE DESIGNED TO SUPPORT A MINIMUM LIVE LOAD CAPACITY OF 150 PSF. PRECAST CONCRETE PLANK TO BE DESIGNED FOR MINIMUM OF 2 HOUR FIRE RATING.
- PC PLANK INDICATES SPAN OF 10" DEEP PRESTRESSED, PRECAST CONCRETE HOLLOW CORE PLANK WITH A MINIMUM 2" OF NORMAL WEIGHT CONCRETE TOPPING. PLANK AND CONCRETE TOPPING SLAB TO BE DESIGNED TO SUPPORT A MINIMUM LIVE LOAD CAPACITY OF 150 PSF. PRECAST CONCRETE PLANK TO BE DESIGNED FOR MINIMUM OF 2 HOUR FIRE RATING.
- FOR DIMENSIONS AND ELEVATIONS NOT GIVEN REFER TO ARCHITECTURAL DRAWINGS.
- WF INDICATES A SPLICE CONNECTION ALONG A CONTINUOUS STEEL BEAM. DESIGN SPLICE CONNECTION FOR FULL CAPACITY OF STEEL BEAM.
- INDICATES A SLAB DEPRESSION. REFER TO DETAILS 1 ON DRAWING S0-0-6.
- 9/16" INDICATES SPAN DIRECTION OF 9/16" DEEP, 20 GAGE GALVANIZED STEEL ROOF DECK.
- XX'XX" INDICATES OPENING IN WEB OF STEEL BEAM FOR MECH/ELECT/PLUMB/FIRE PROTECT/ETC COORDINATE WITH APPLICABLE DRAWING FOR OPENING SIZE. SEE TYPICAL DETAIL 5 AND 6 ON S0-0-4 FOR ADDITIONAL INFORMATION.
- 3" INDICATES SPAN DIRECTION OF 3" DEEP, 20 GAGE TYPE N, GALVANIZED STEEL ROOF DECK.
- STEEL BEAMS ARE EQUALLY SPACED BETWEEN COLUMN GRIDLINES, UNLESS NOTED OTHERWISE.

FRAMING ELEVATION NOTES:

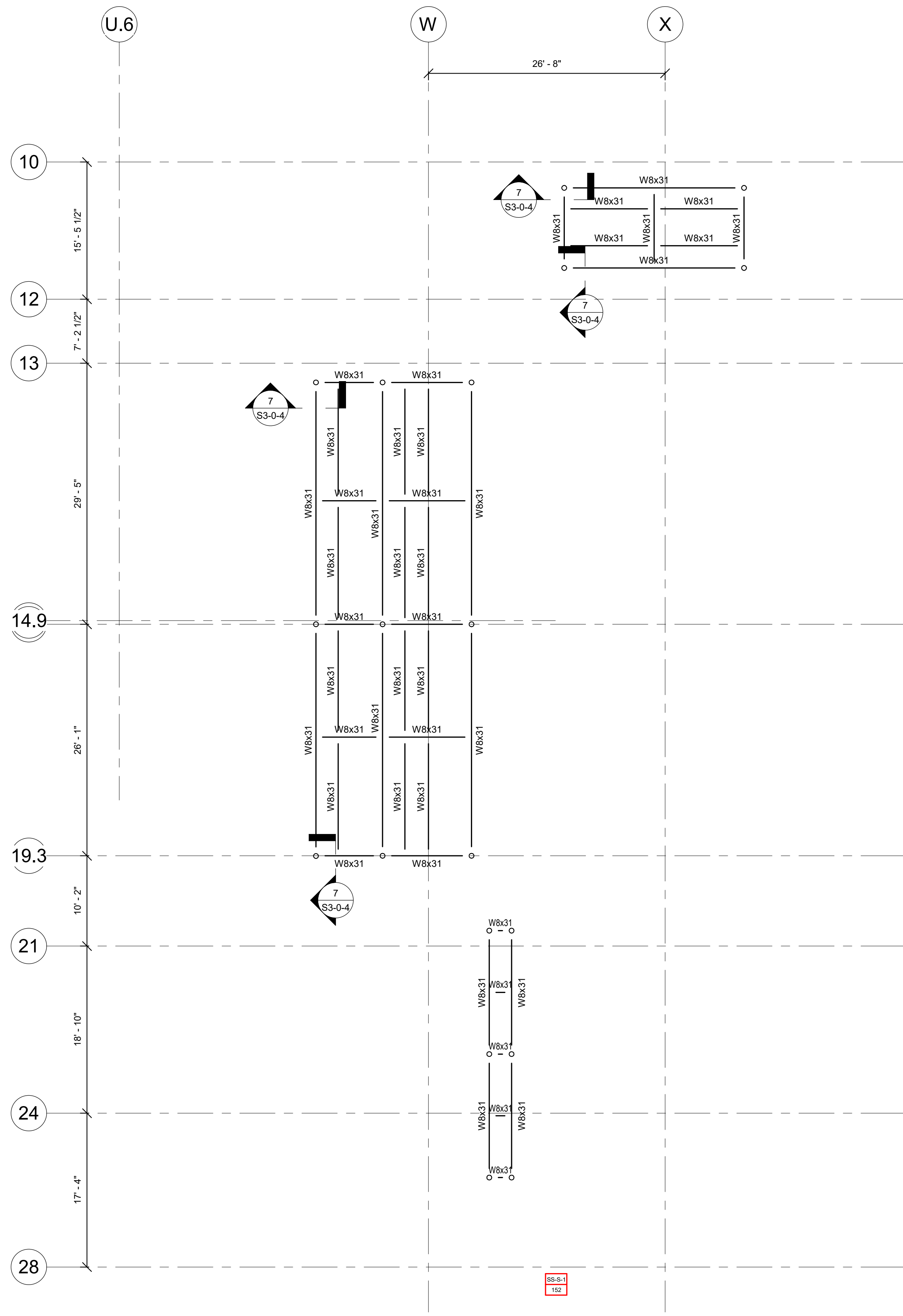
- TYPICAL UNDERSIDE-OF-DECK ELEVATION = (162' - 0") AT THE ROOF LEVEL, IN THE AREA BOUNDED BY GRIDS (AA) - (DD) AND (24) - (34.4), UNLESS NOTED OTHERWISE AS (X' - Y'), (X' - Y'), (X' - Y'), OR (H/O).
- COORDINATE ALL ELEVATIONS WITH ARCHITECTURAL DRAWINGS.
- SLOPE STEEL UNIFORMLY TO COLUMN ELEVATIONS SHOWN ON PLANS.



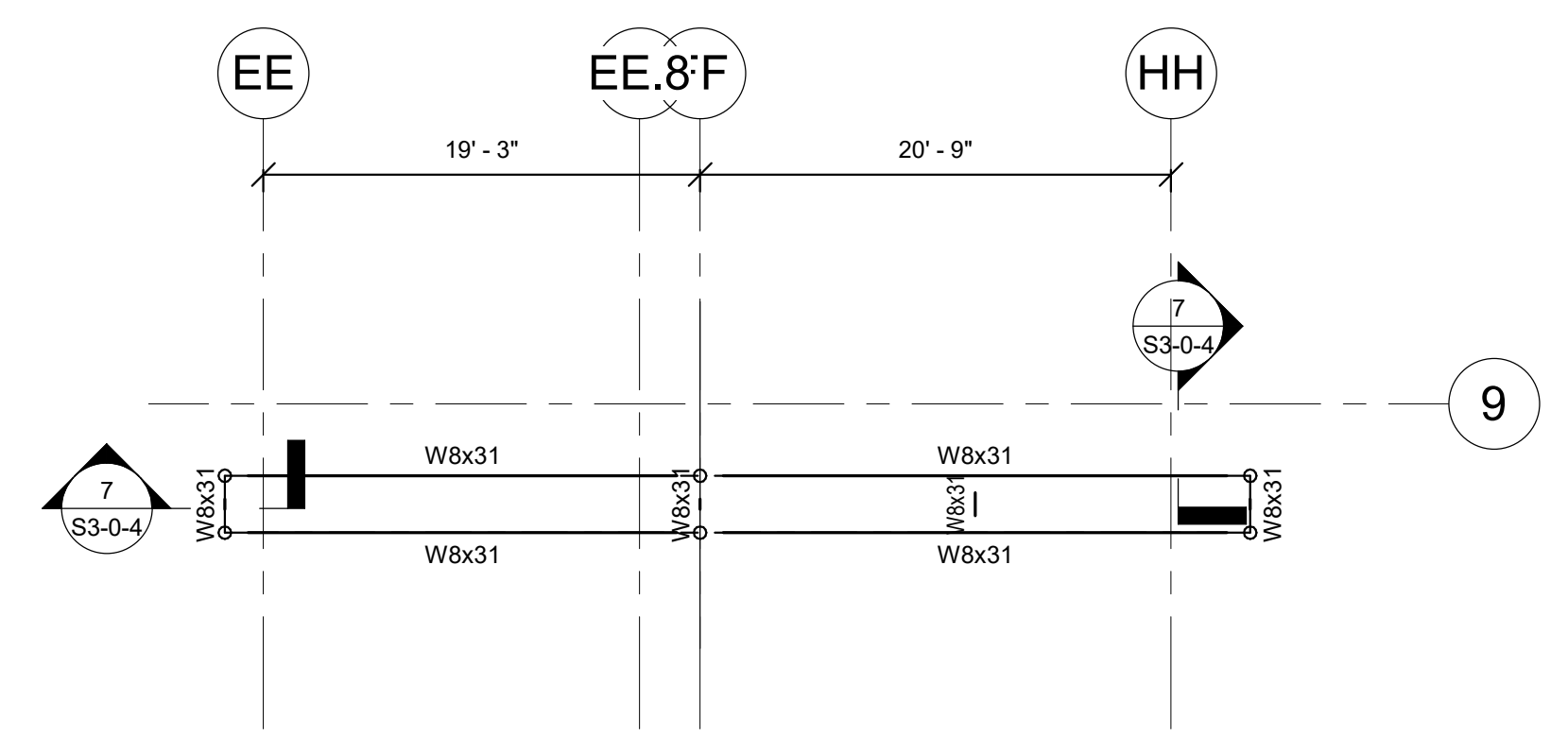
BRACE FRAME KEY	
0' TYP	WF BF-X
0' TYP	BF-X
0' TYP	WF BF-X



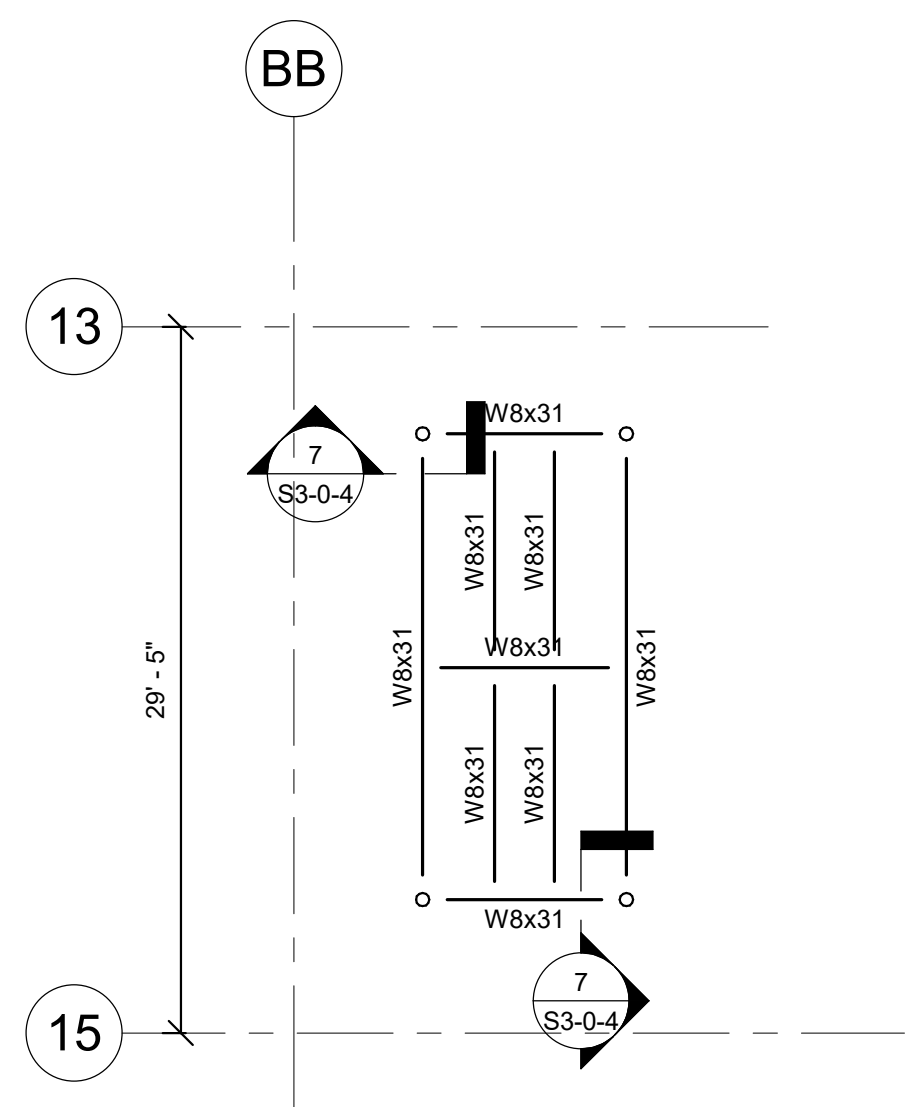
**ROOF DUNNAGE FRAMING
PART PLAN - AREA A**



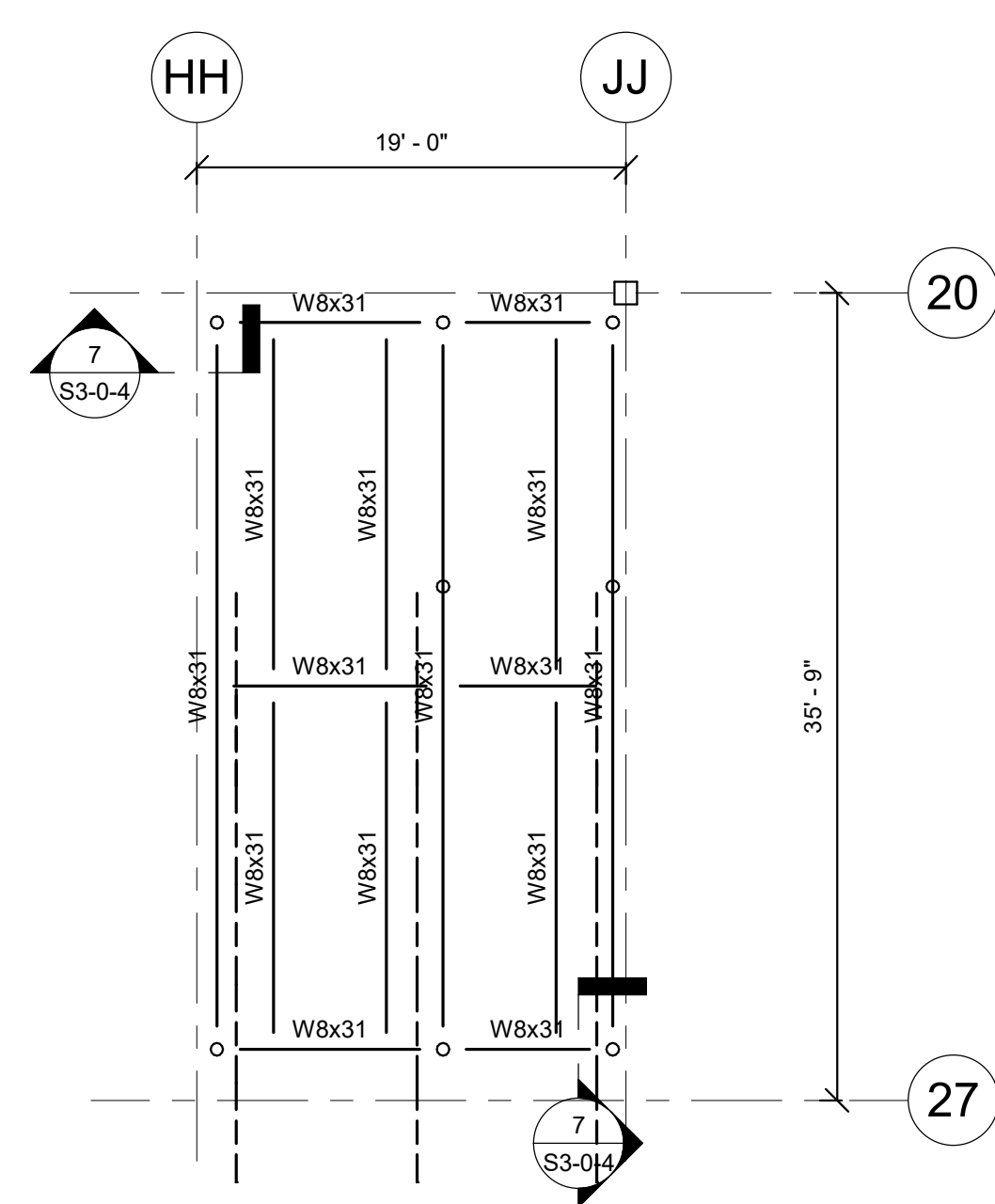
**ROOF DUNNAGE FRAMING
PART PLAN - AREA B**



**ROOF DUNNAGE FRAMING
PART PLAN - AREA C**



**ROOF DUNNAGE FRAMING
PART PLAN - AREA C**



**ROOF DUNNAGE FRAMING
PART PLAN - AREA D**

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03/31/2023	EARLY STRUCTURAL BID PACKAGE
REVISION LIST	
SS-S-1	4/14/2023 STRUCTURAL STEEL ADDENDUM 1

BID SET

August 28th, 2023

KEY PLAN

PROJECT NORTH

MAGNETIC NORTH

**ROOF DUNNAGE
FRAMING PARTS**

Scale: 1/8" = 1'-0"

Job No.: 20202

Drawn By: EDG

Date: August 28th, 2023

S1-1-6

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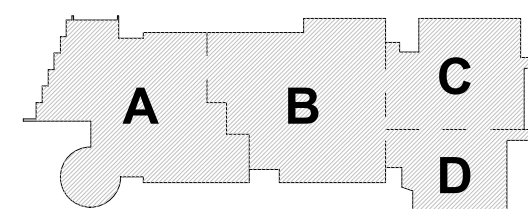
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REVISION LIST

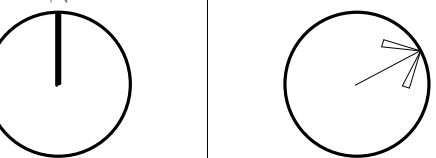
BID SET

August 28th, 2023



KEY PLAN

PROJECT NORTH MAGNETIC NORTH



SECTIONS

Scale: As indicated

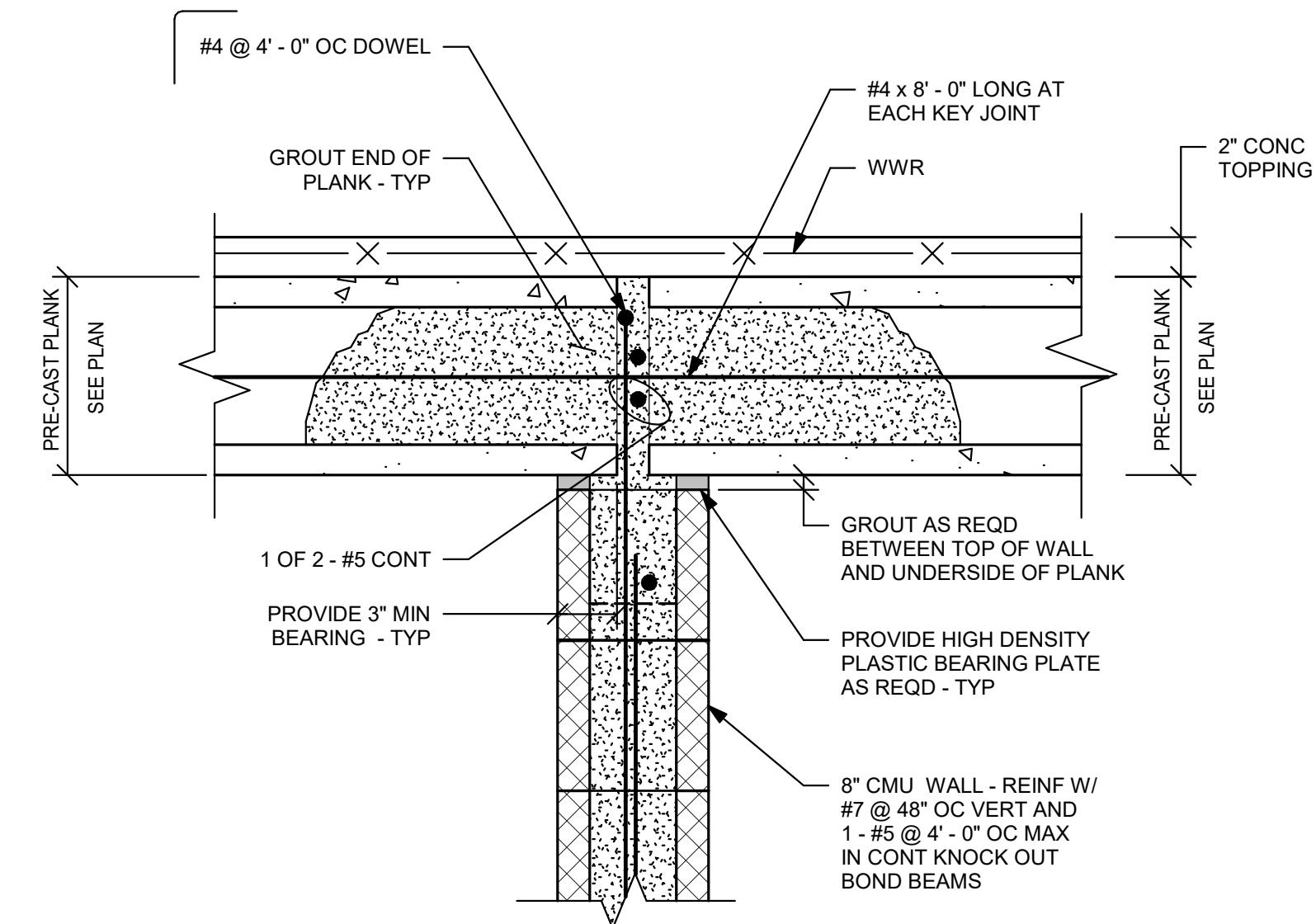
Job No.: 20202

Drawn By: EDG

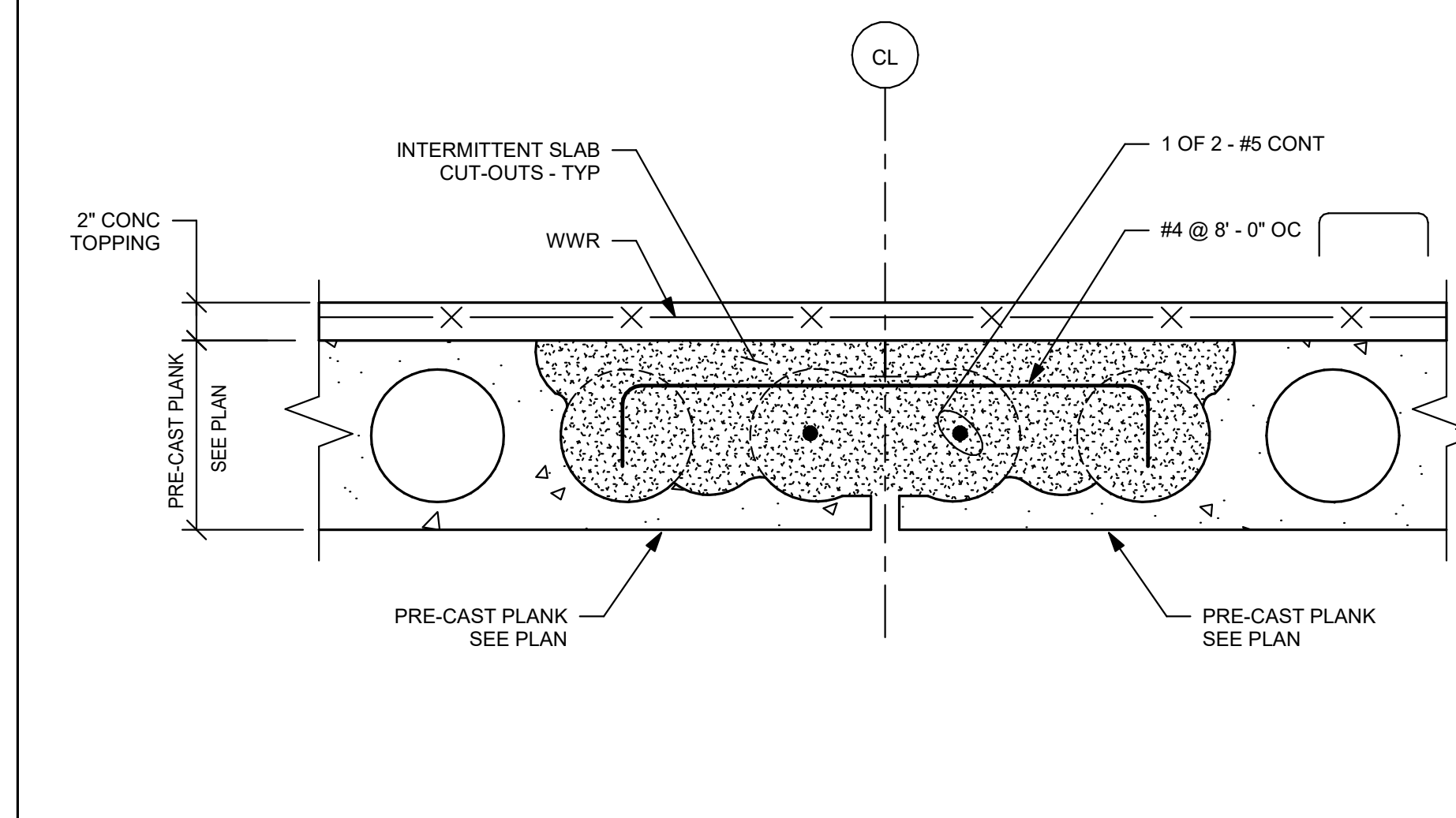
Date: August 28th, 2023

S2-0-2

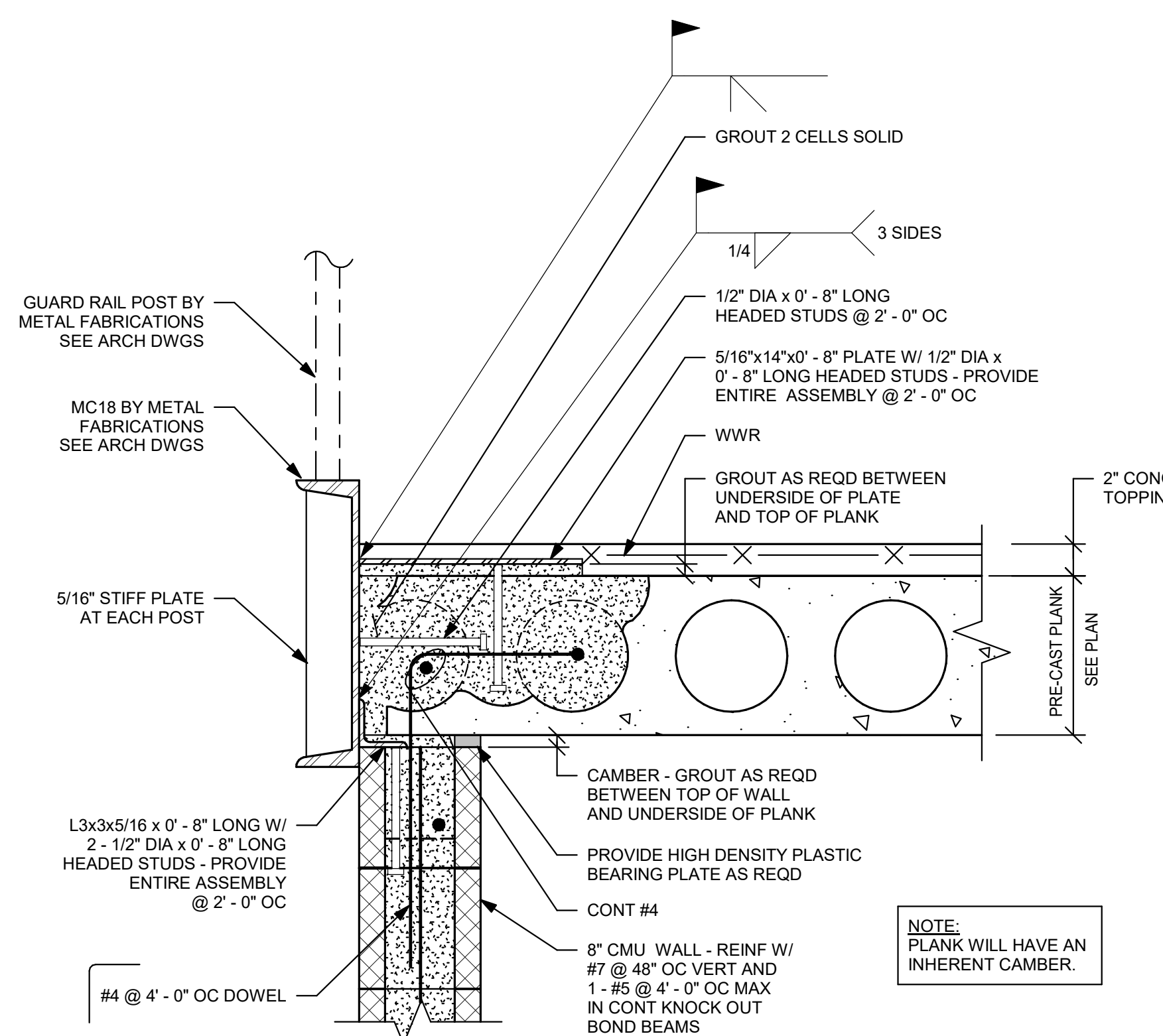
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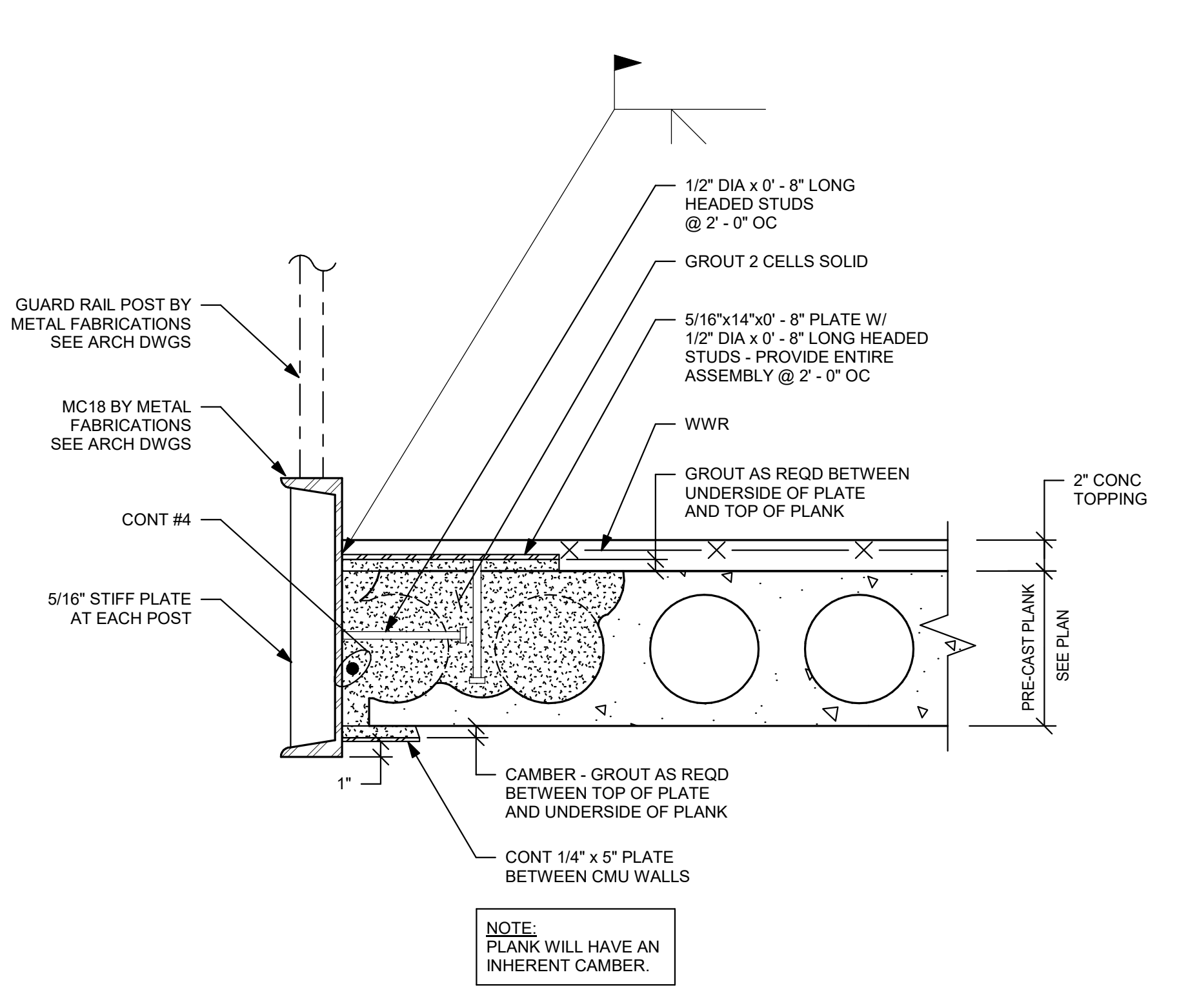
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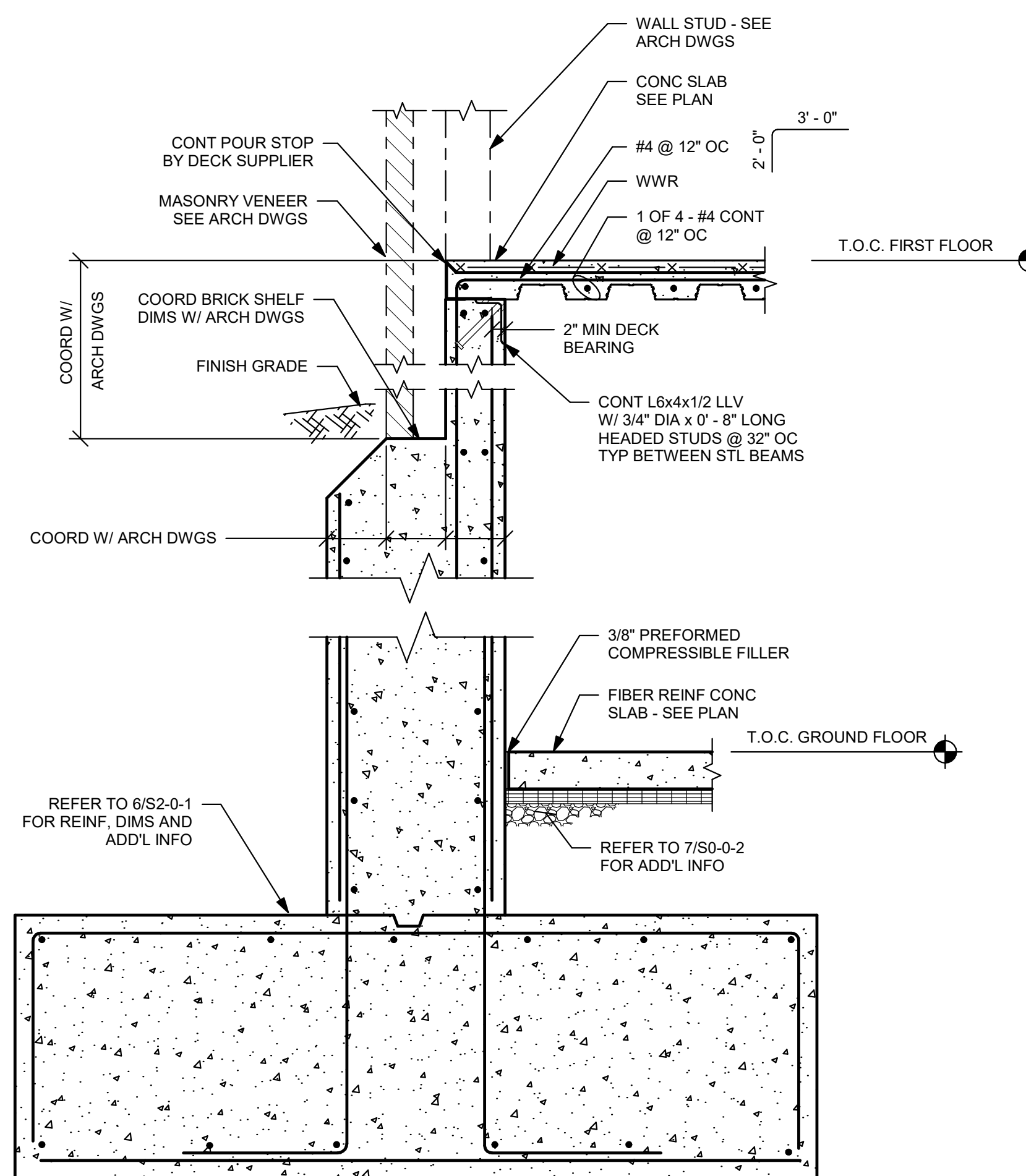
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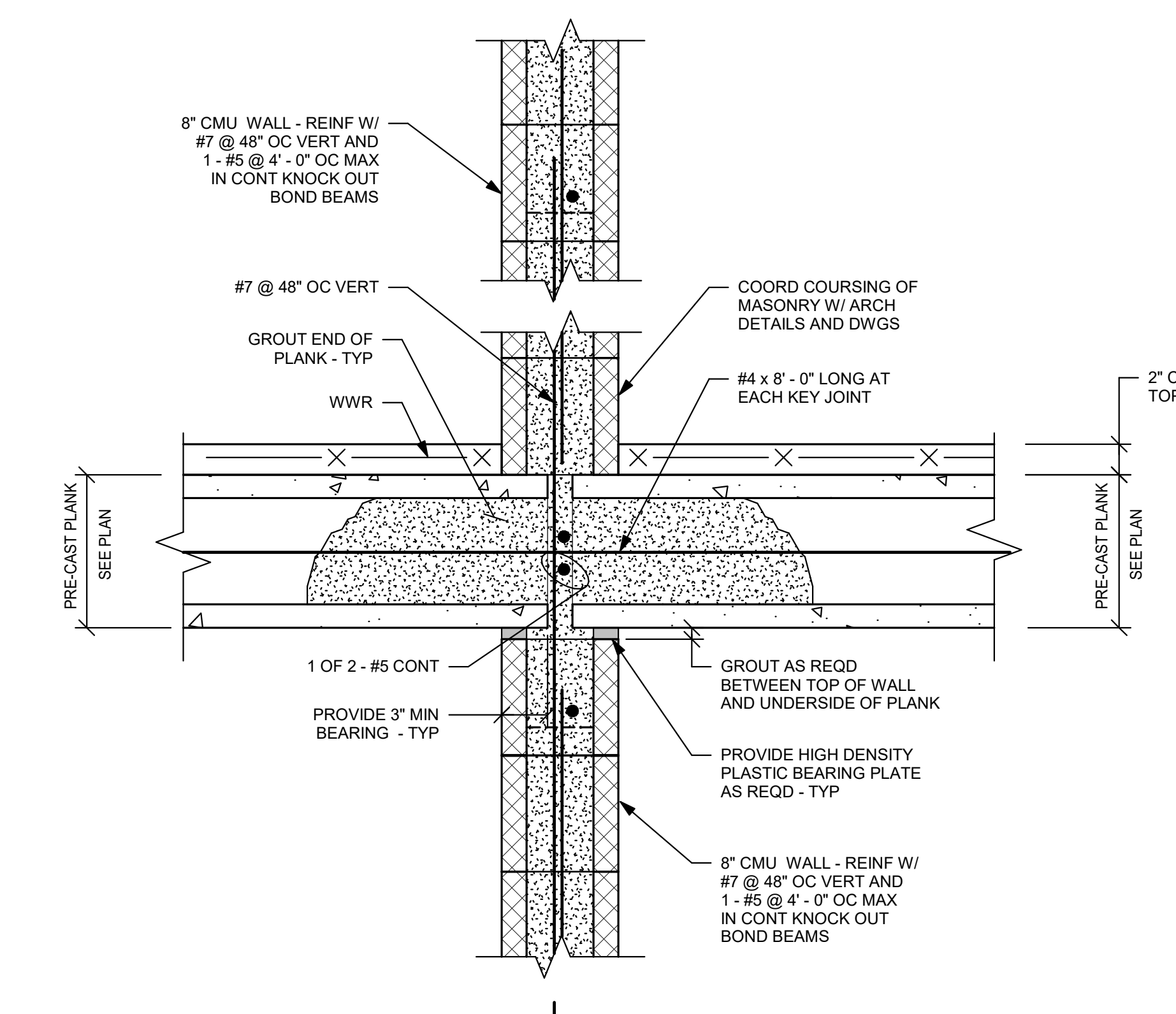
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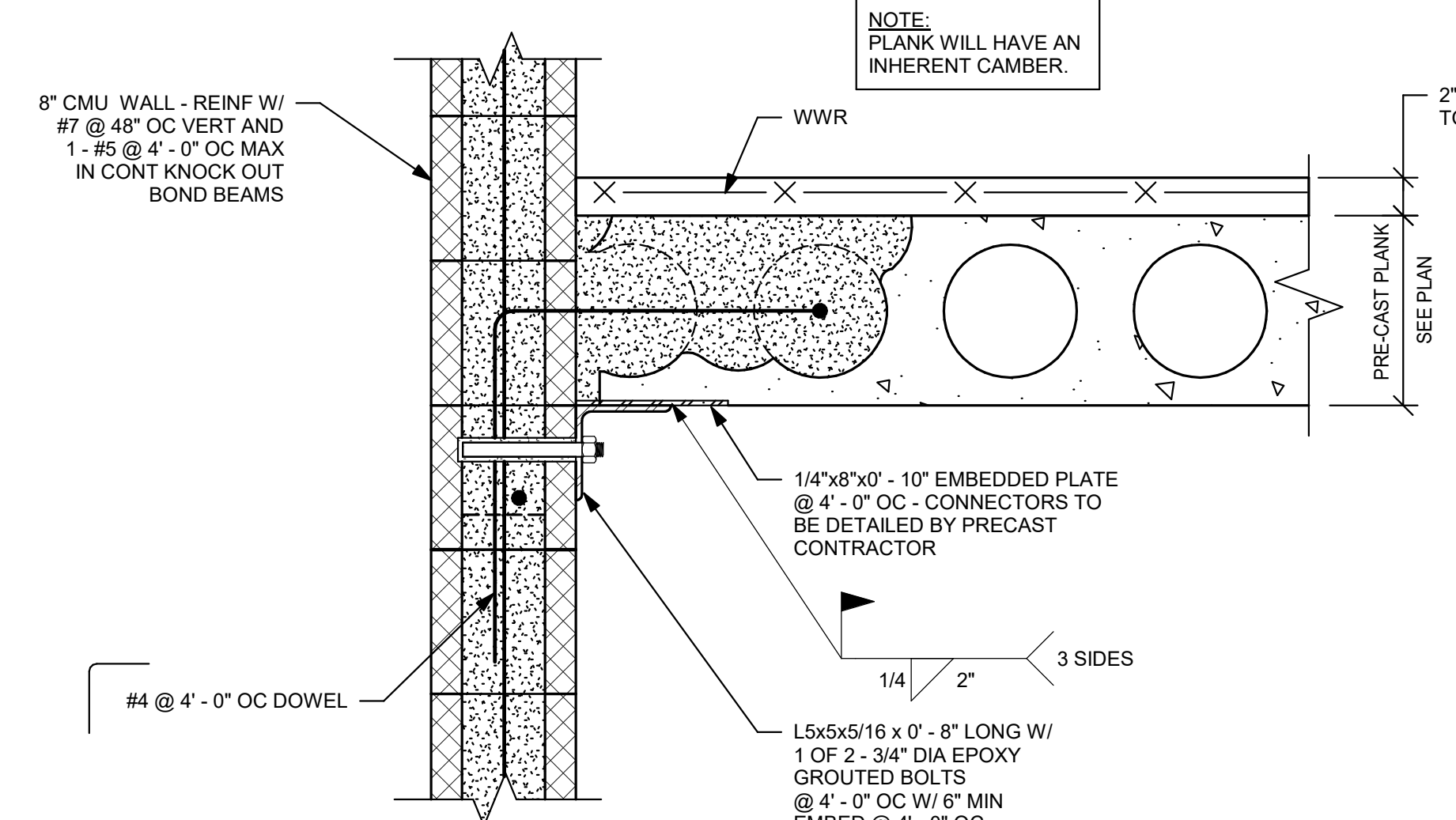
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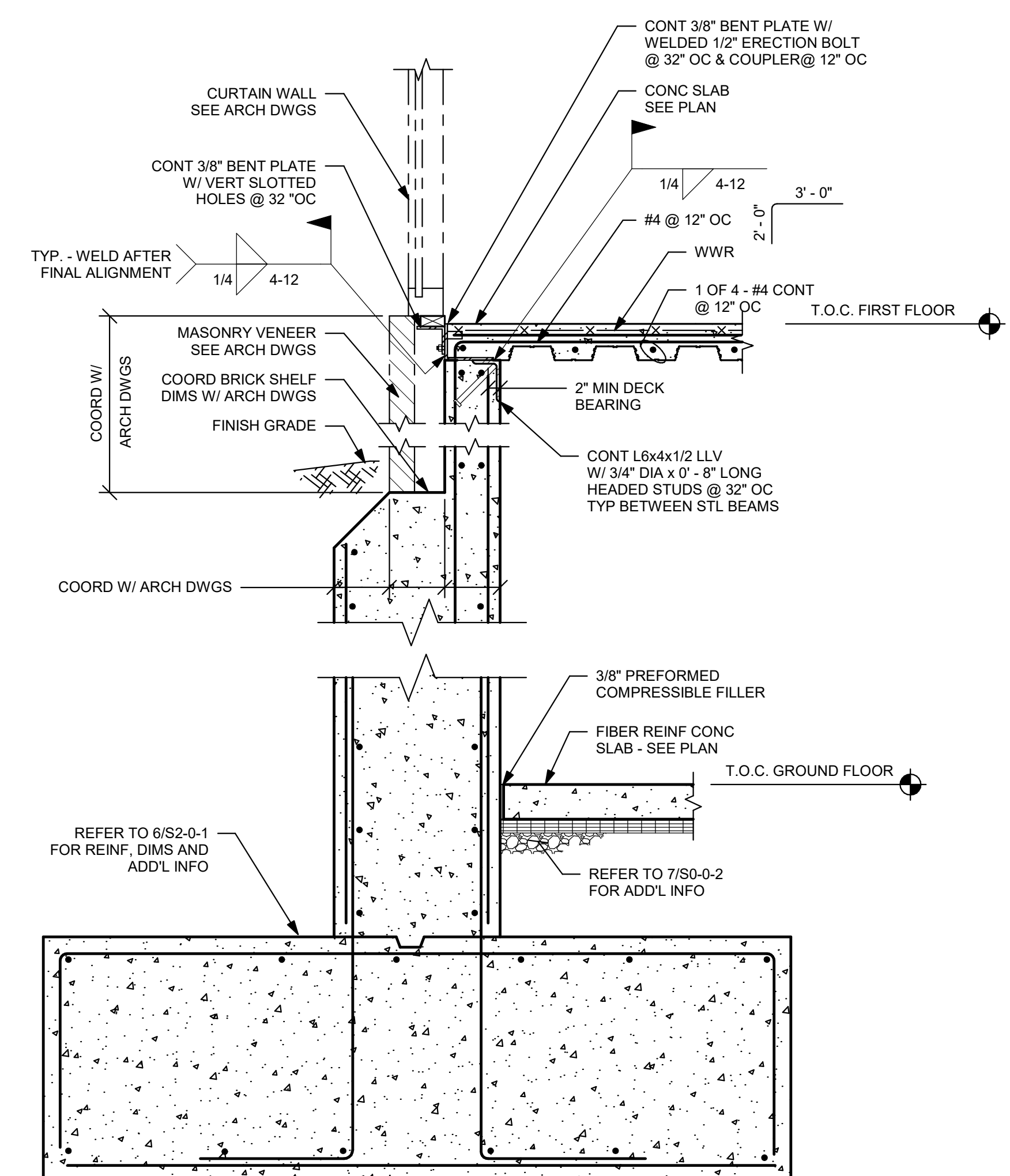
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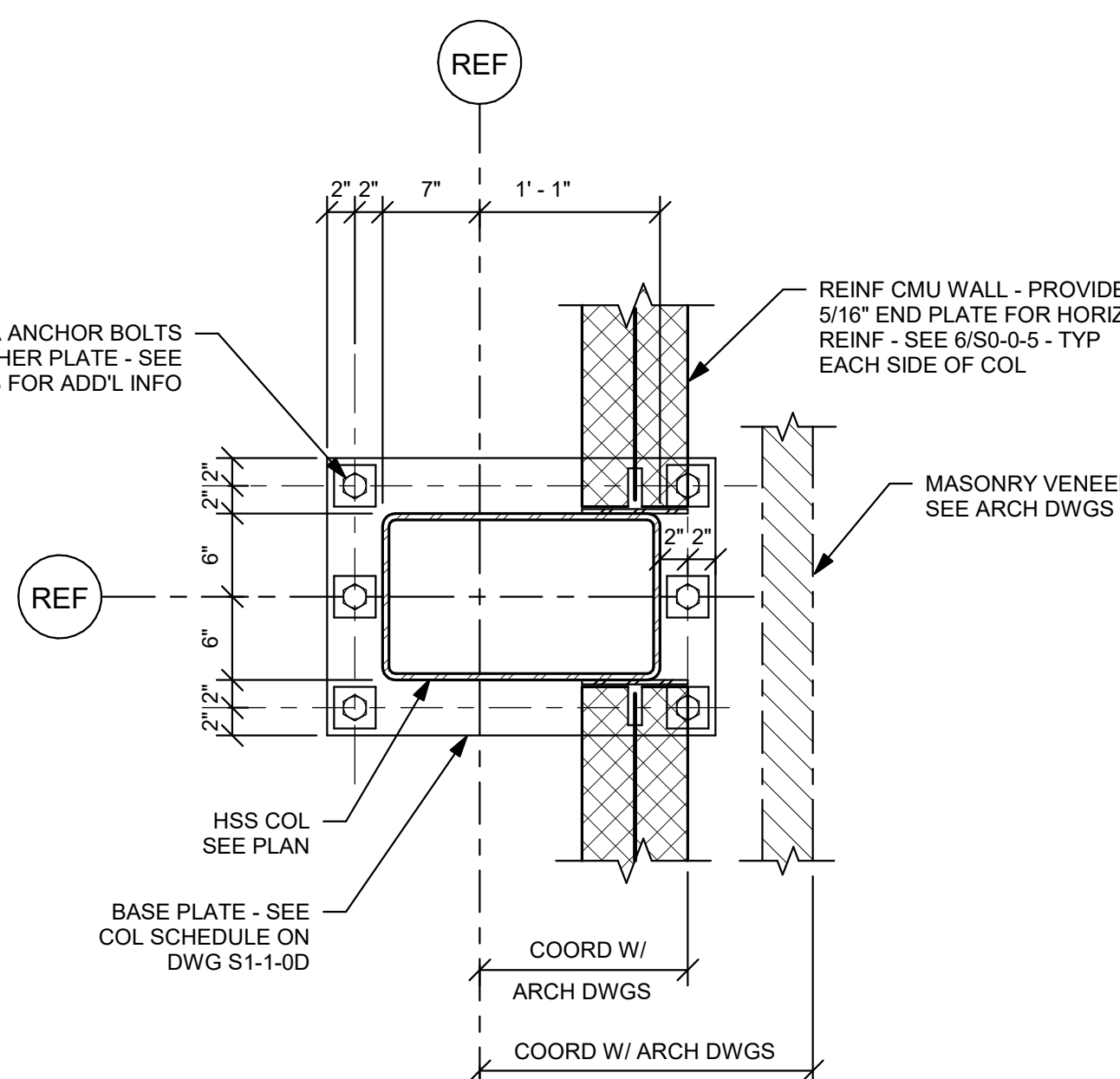
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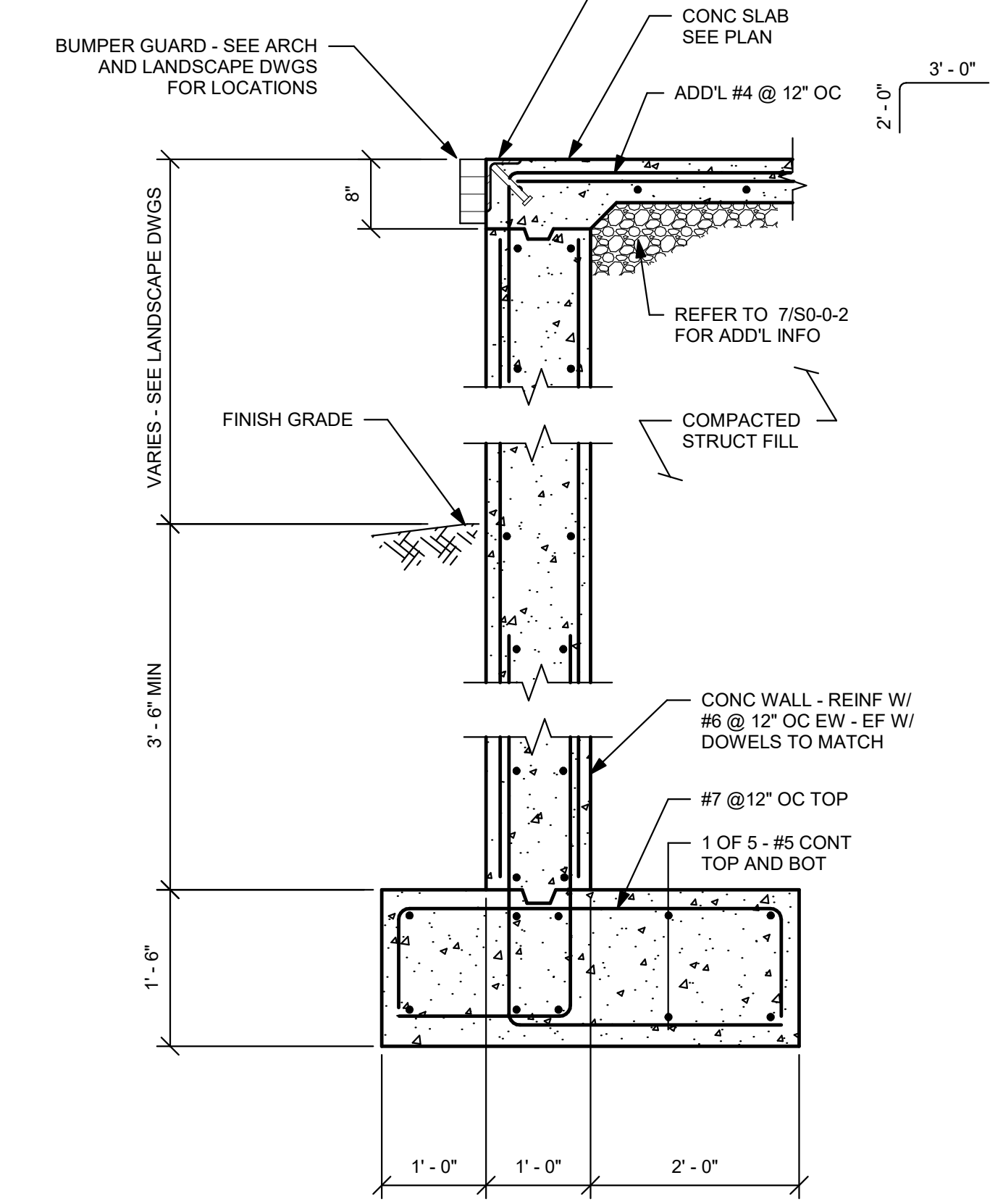
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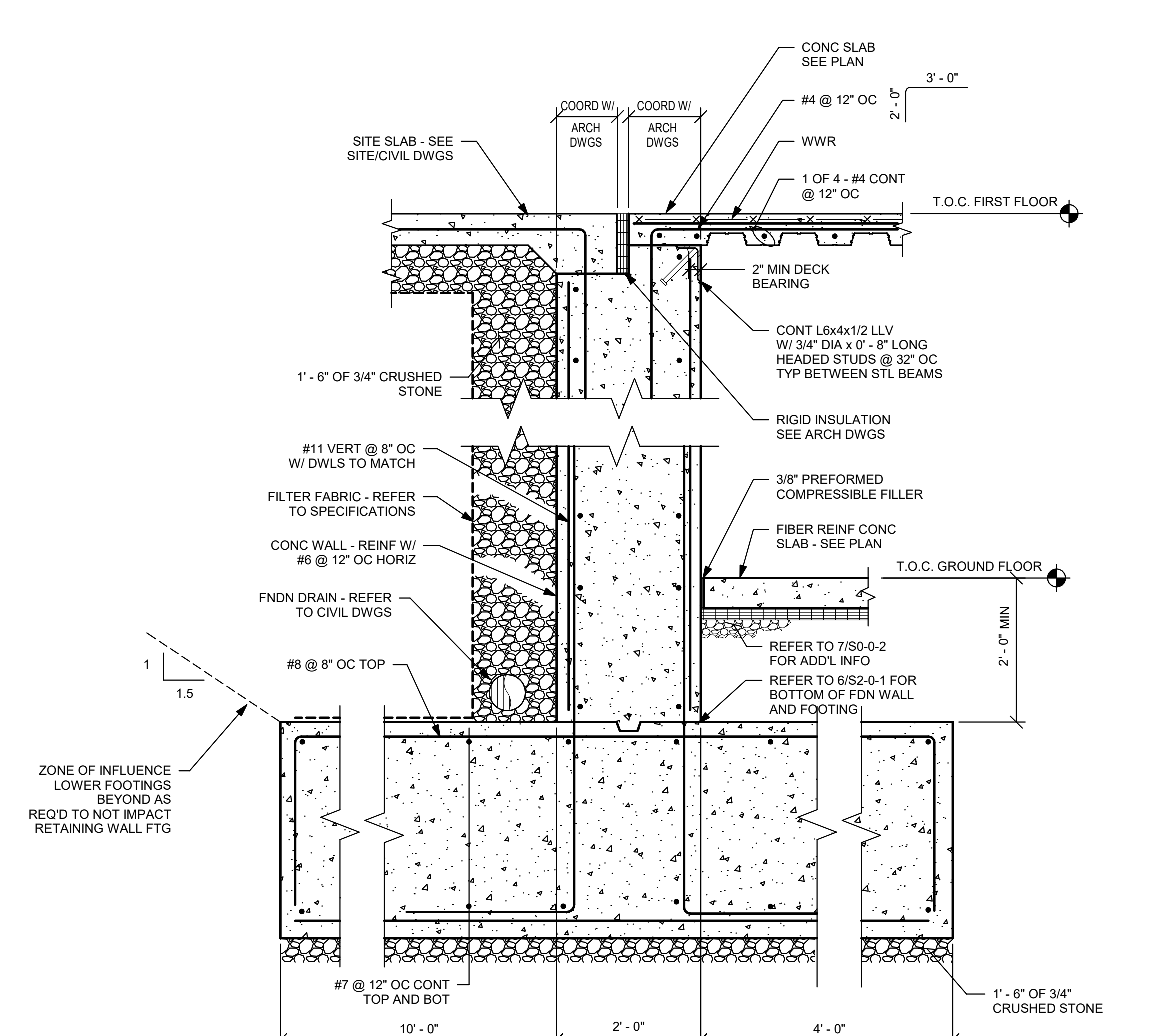
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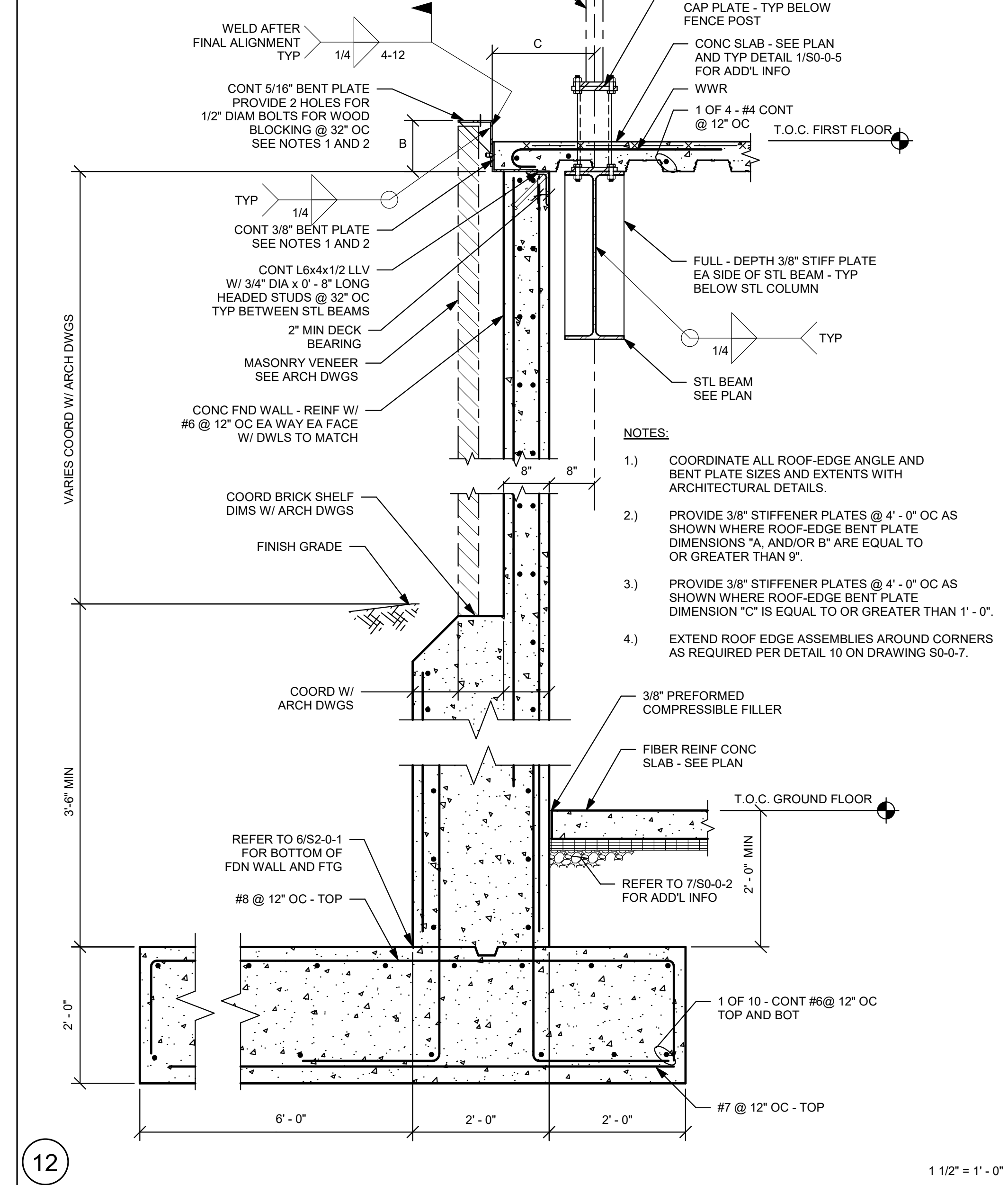
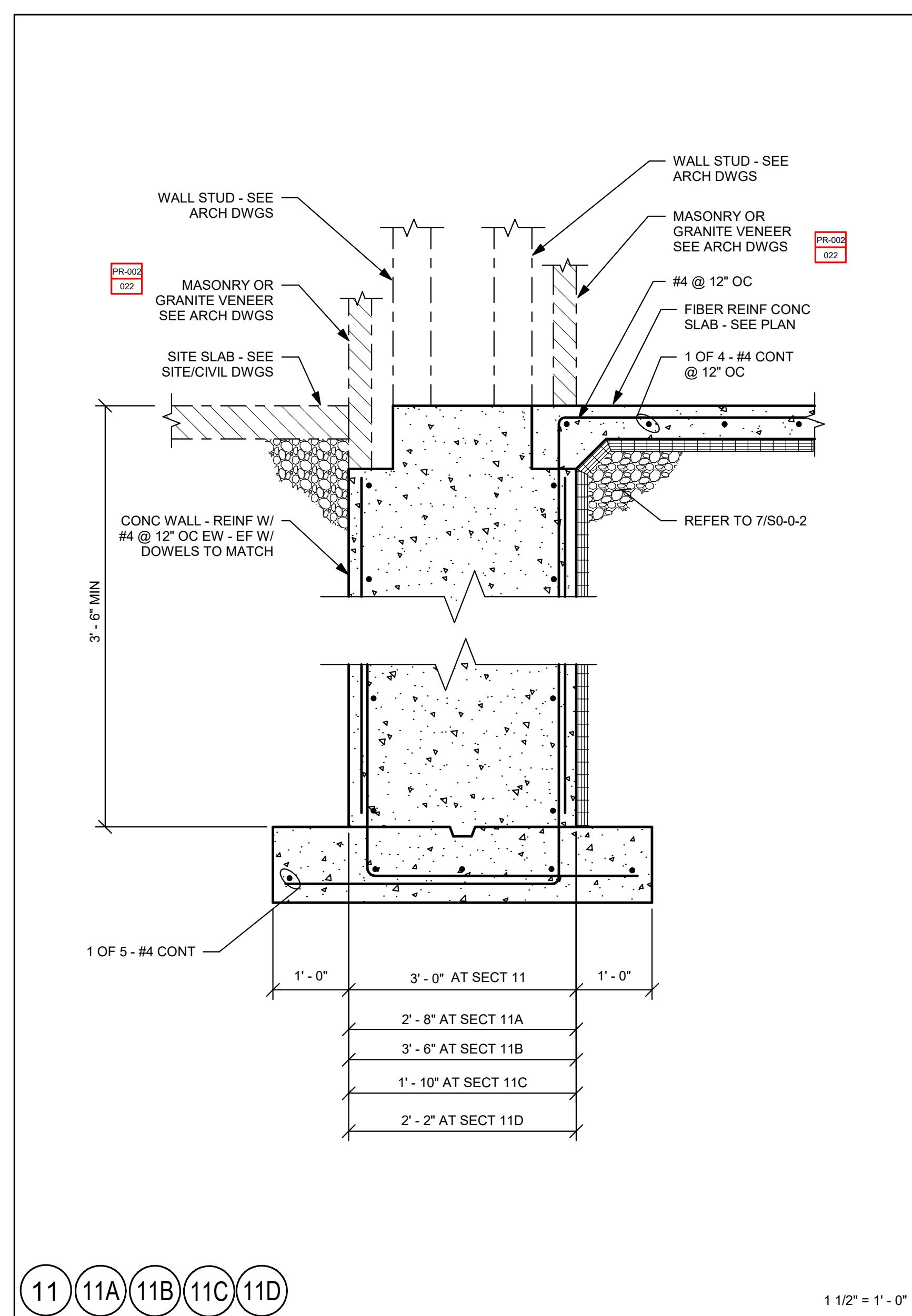
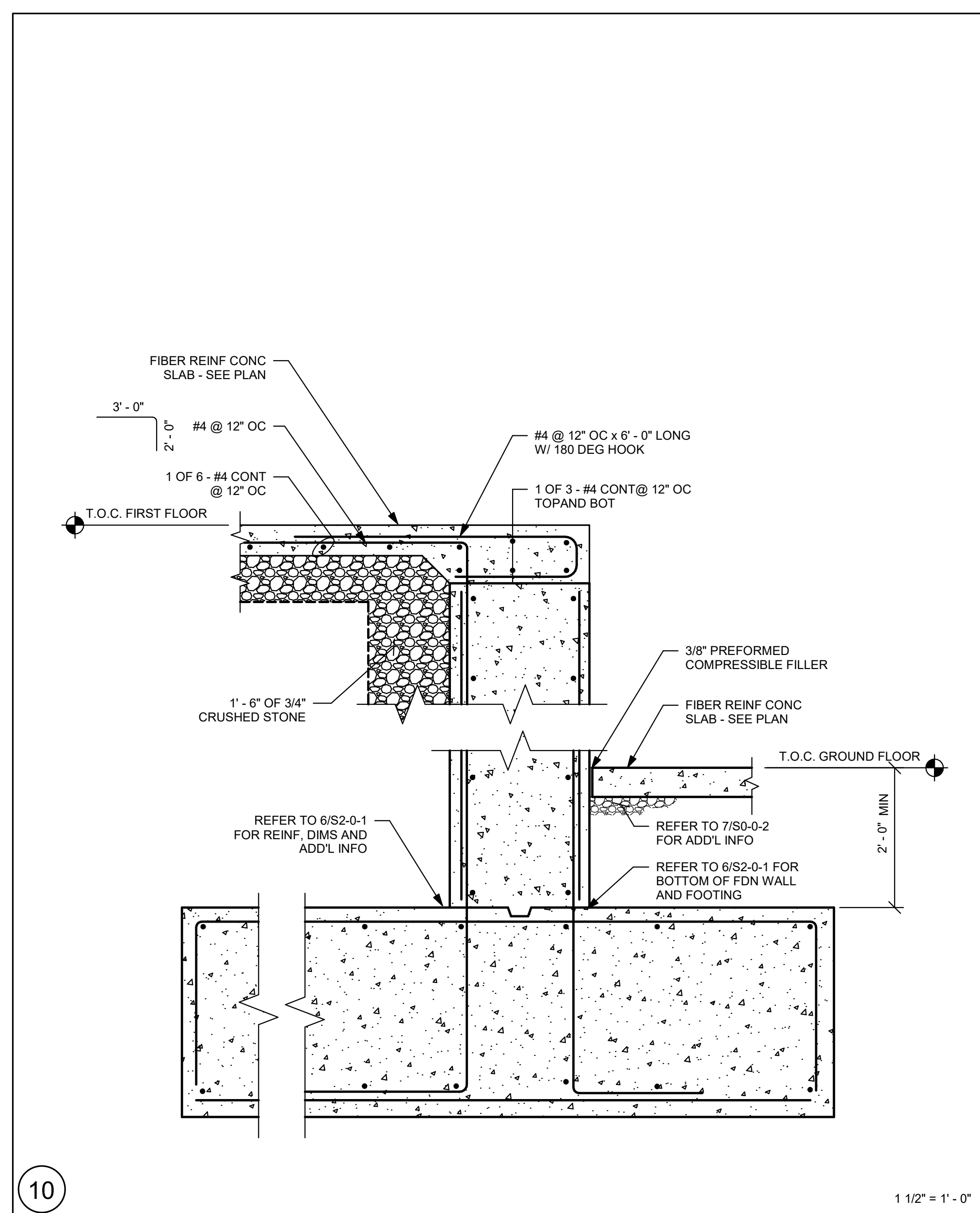
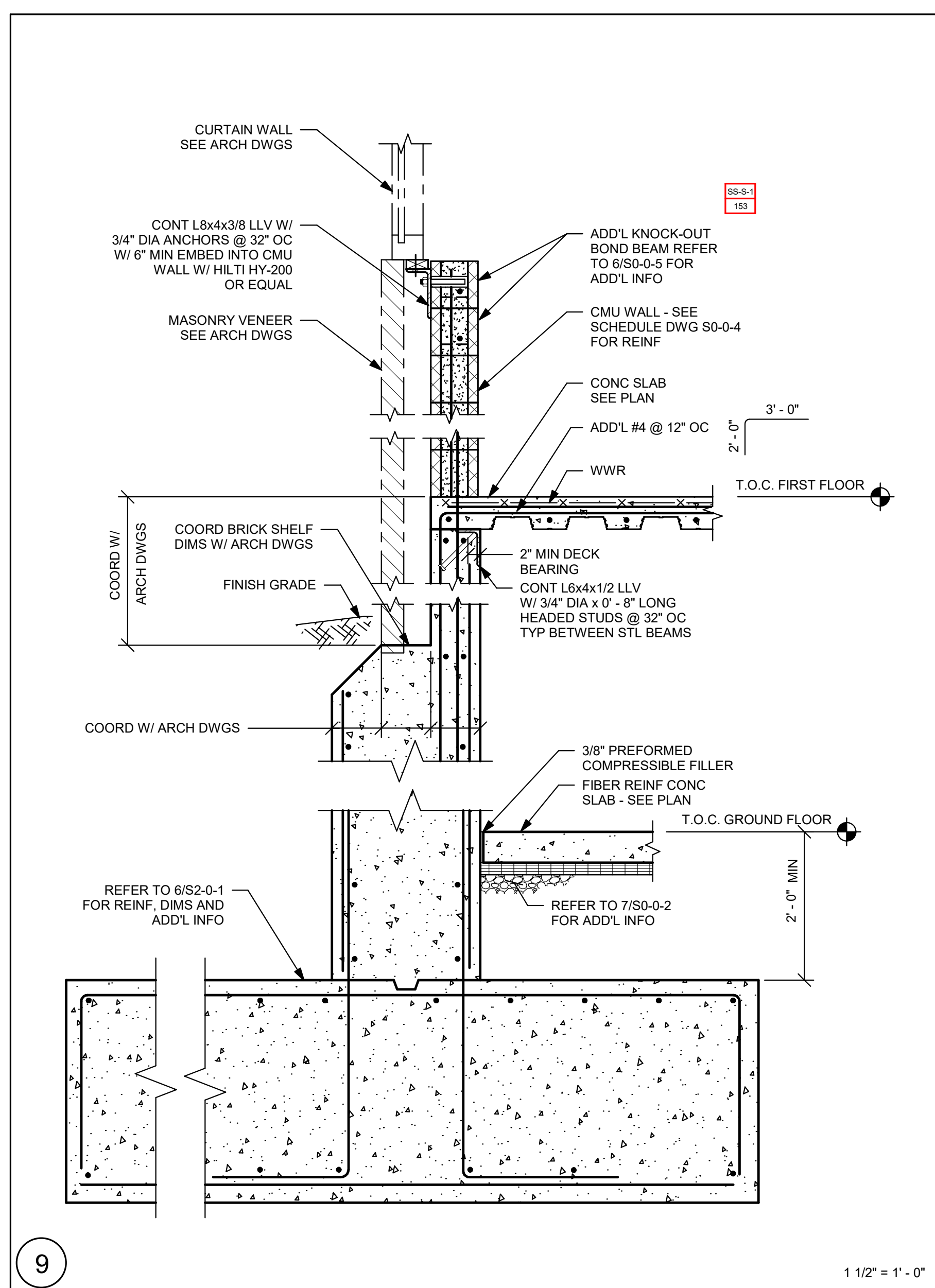
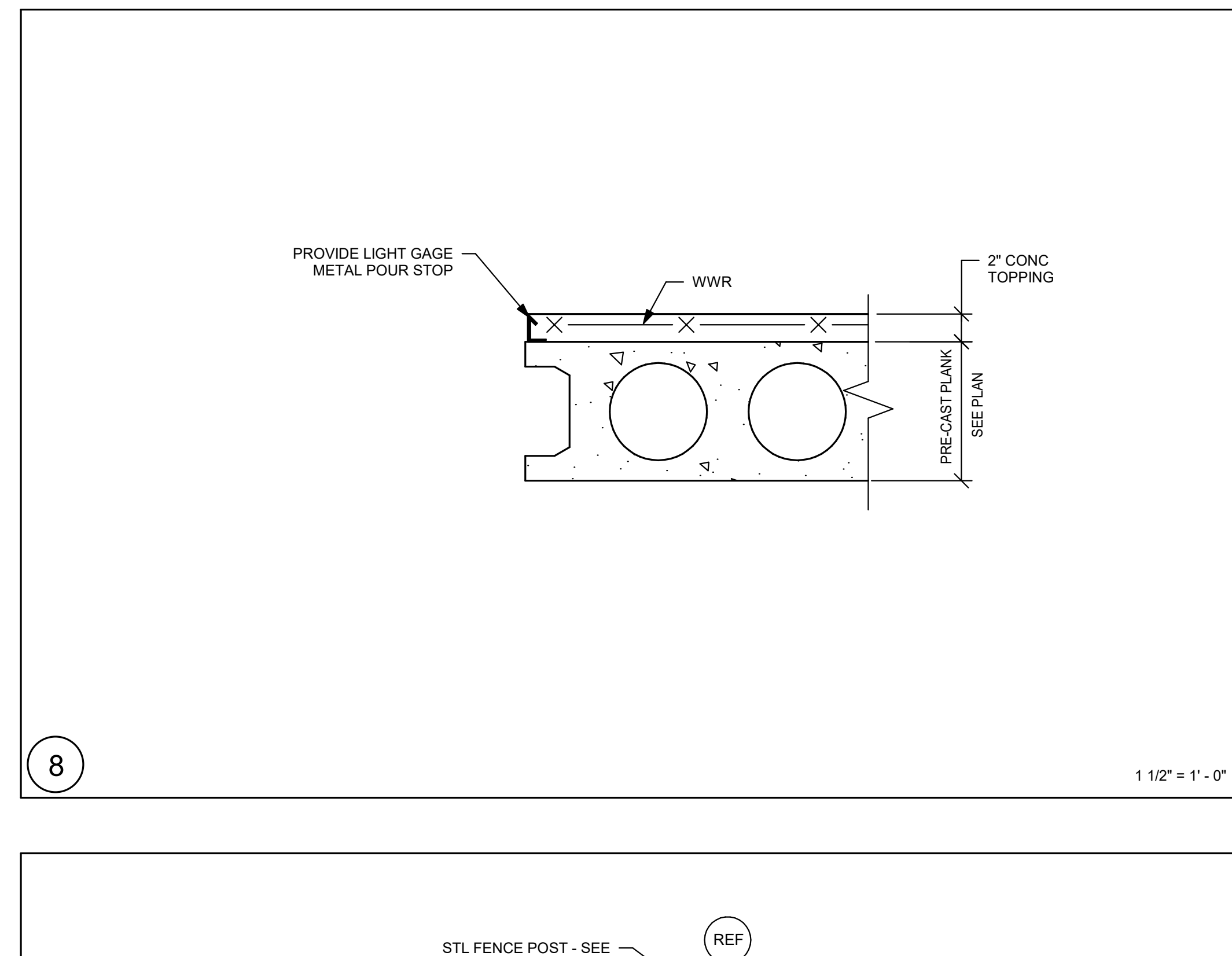
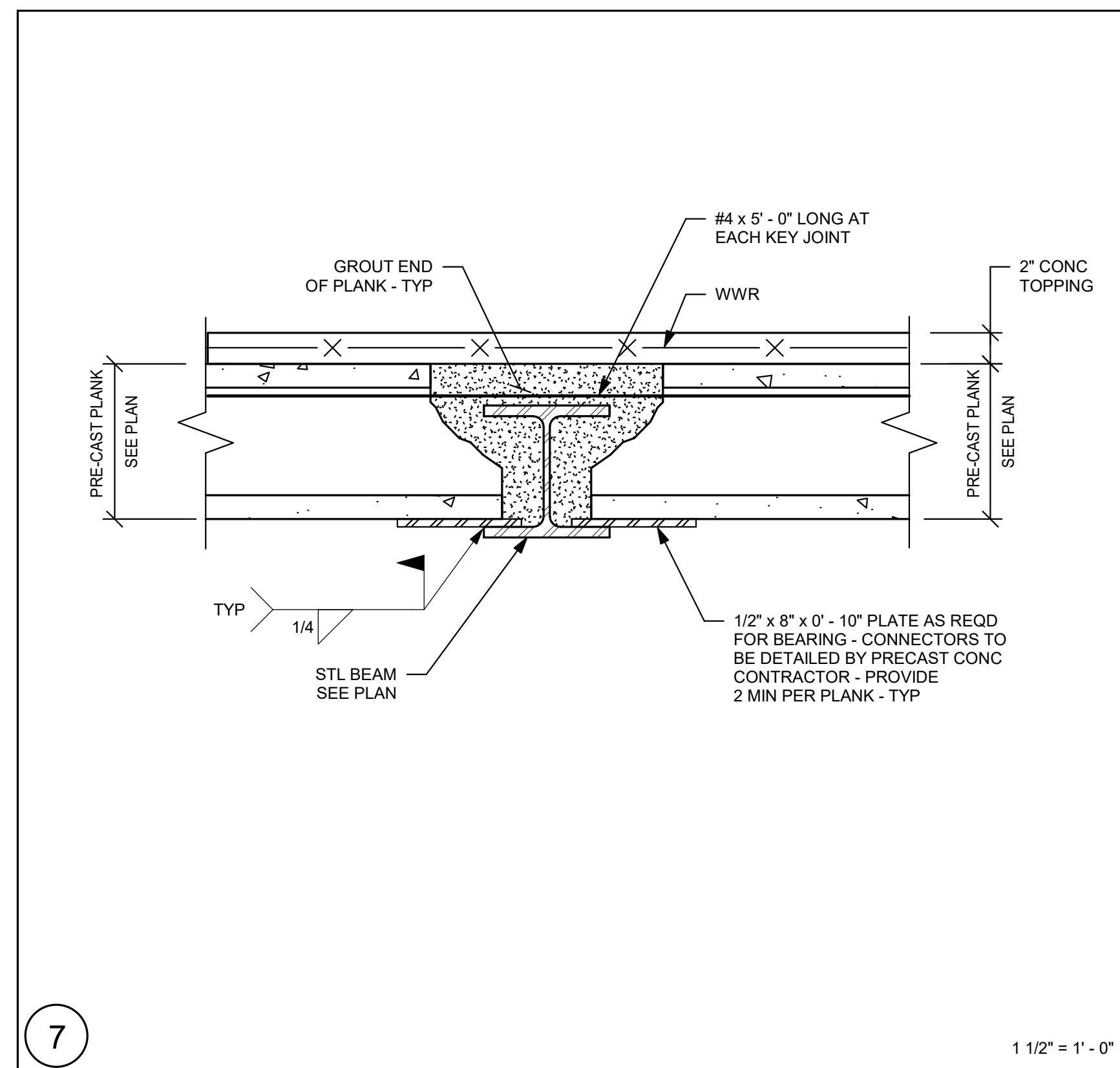
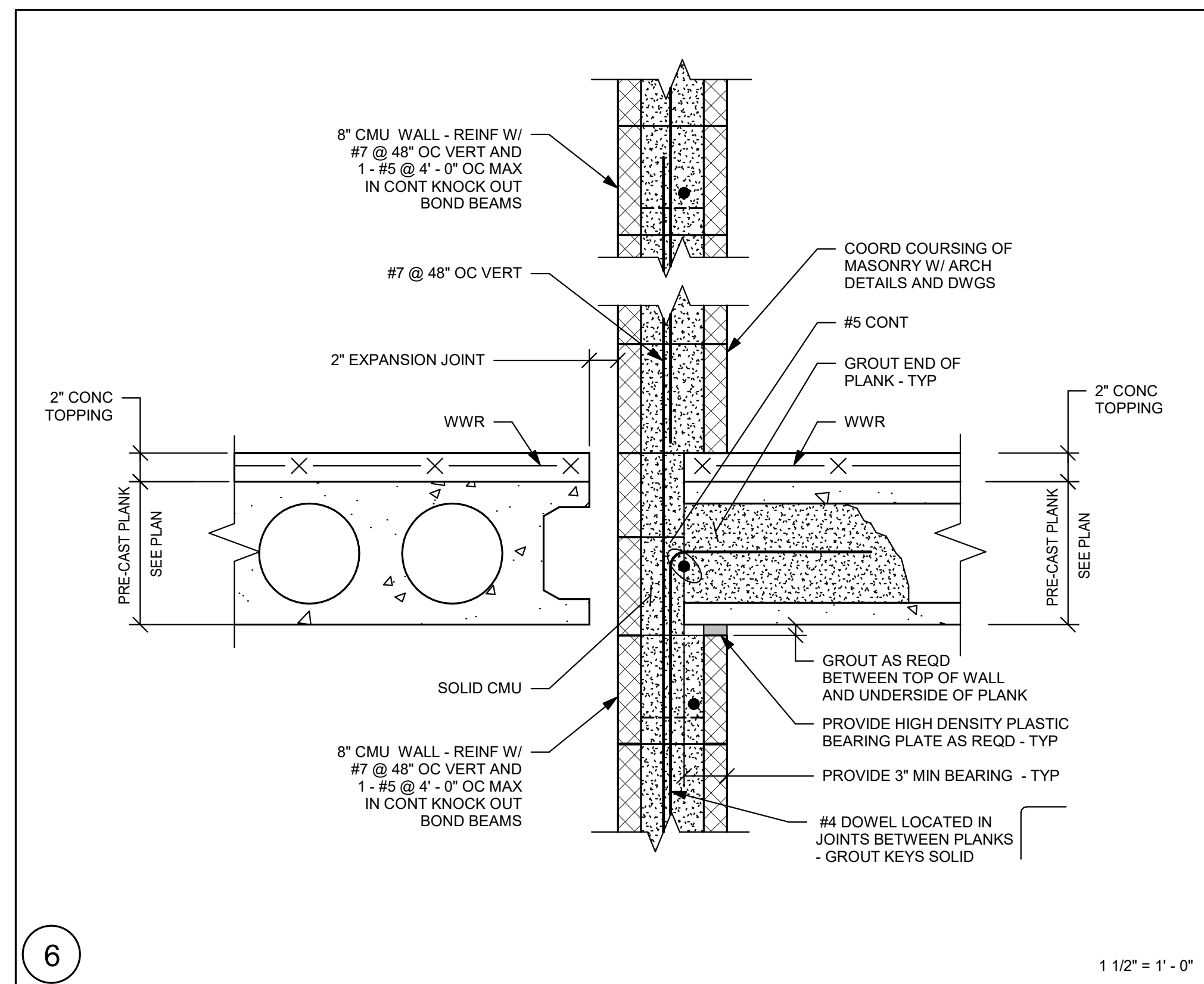
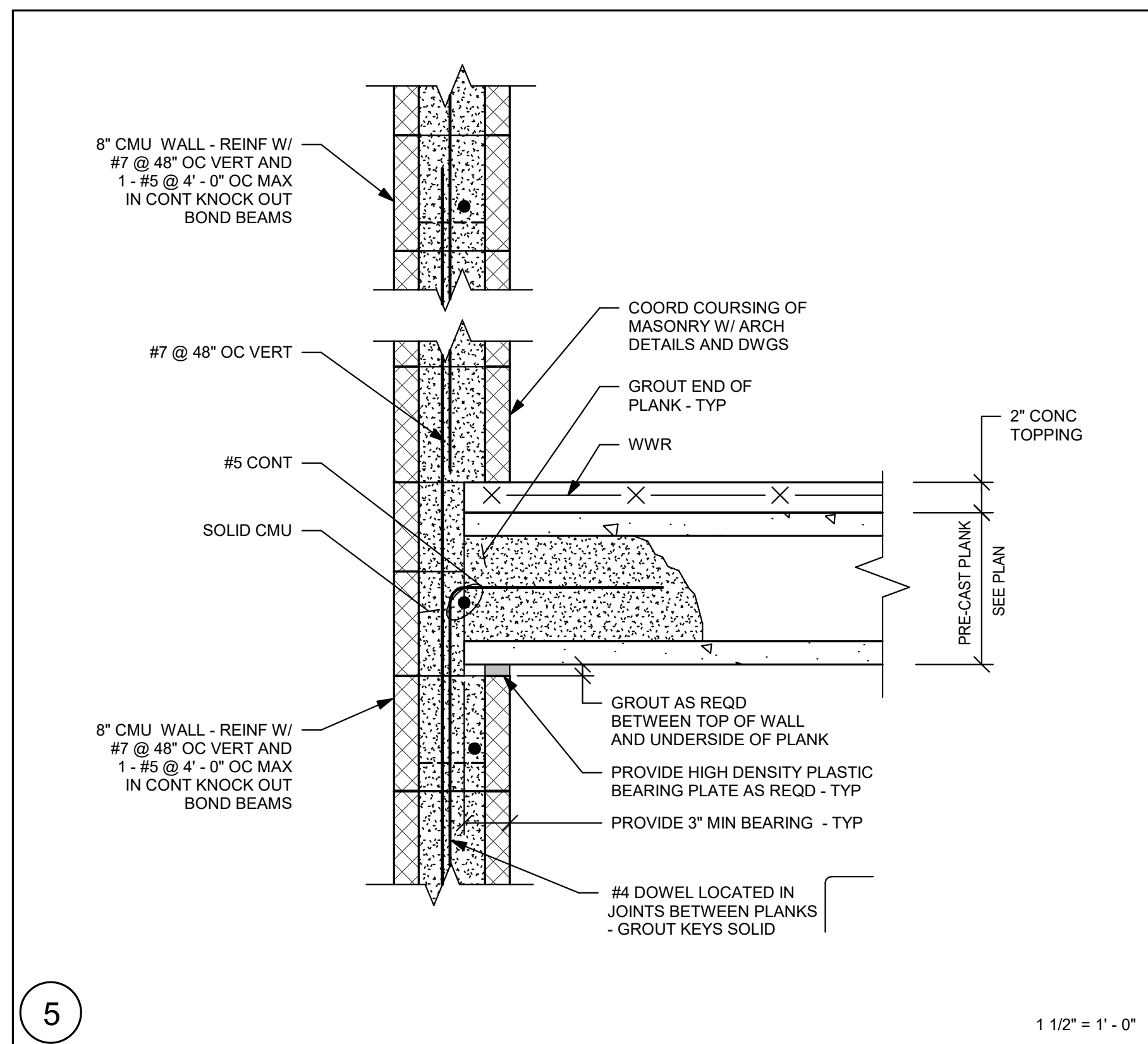
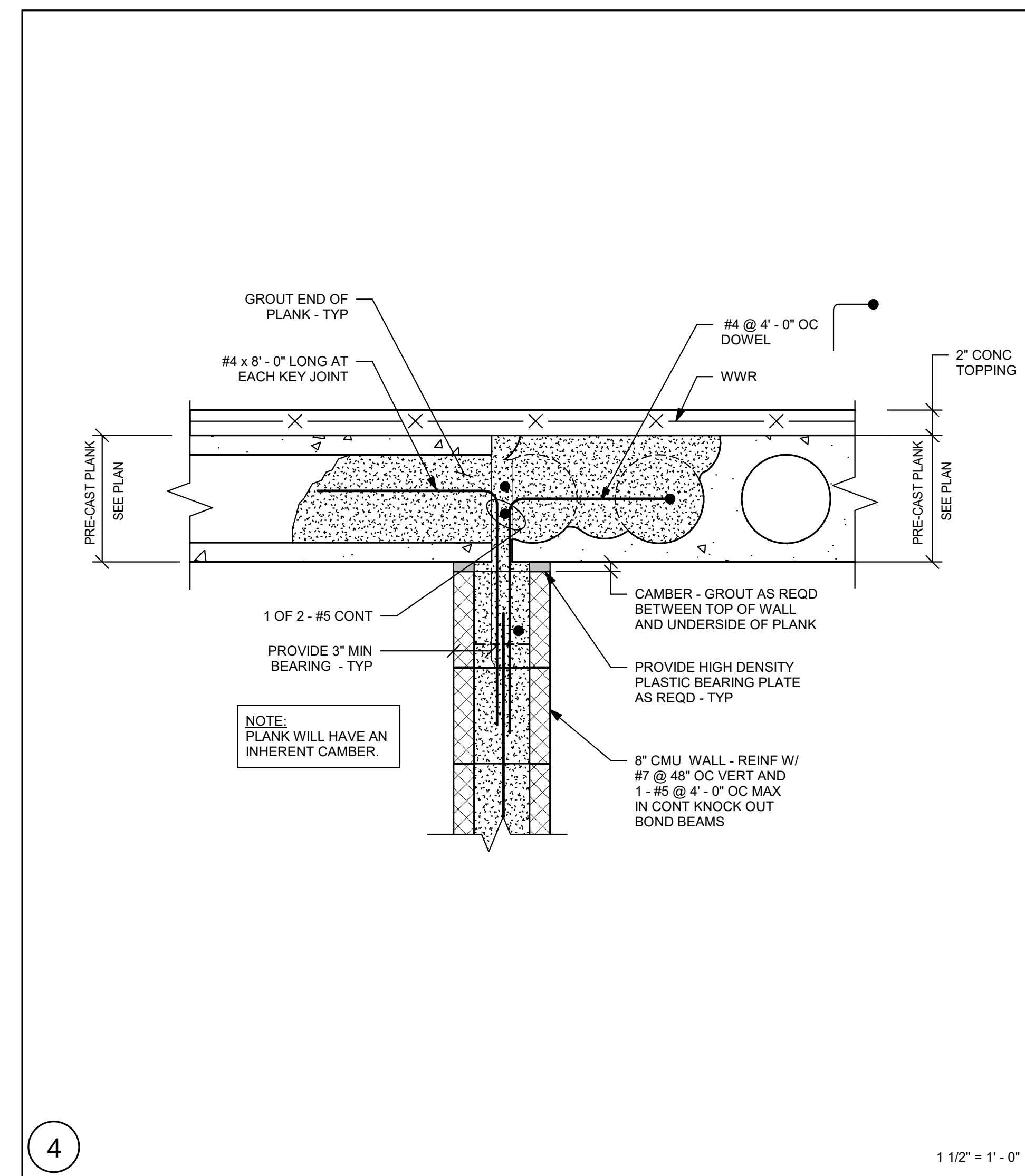
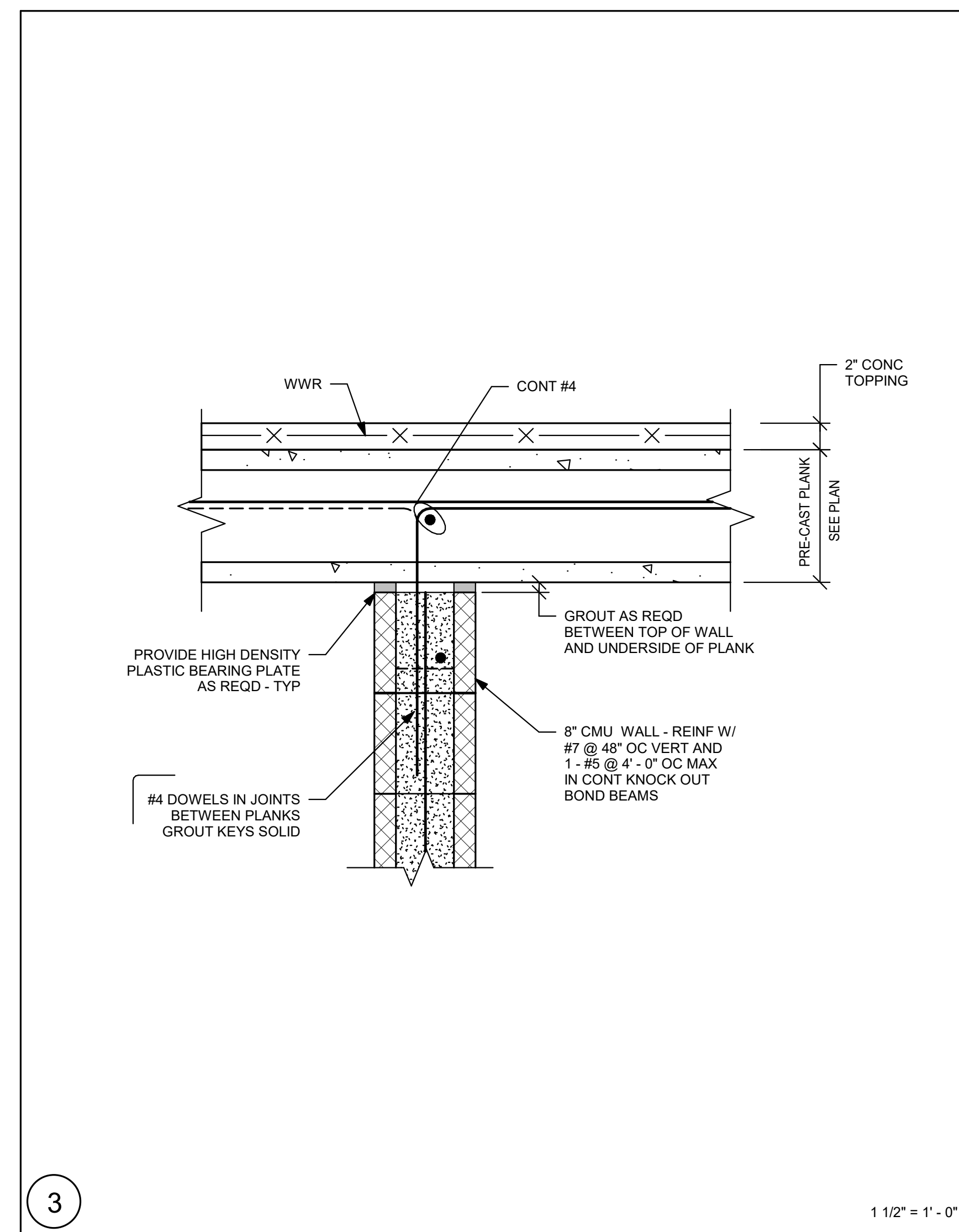
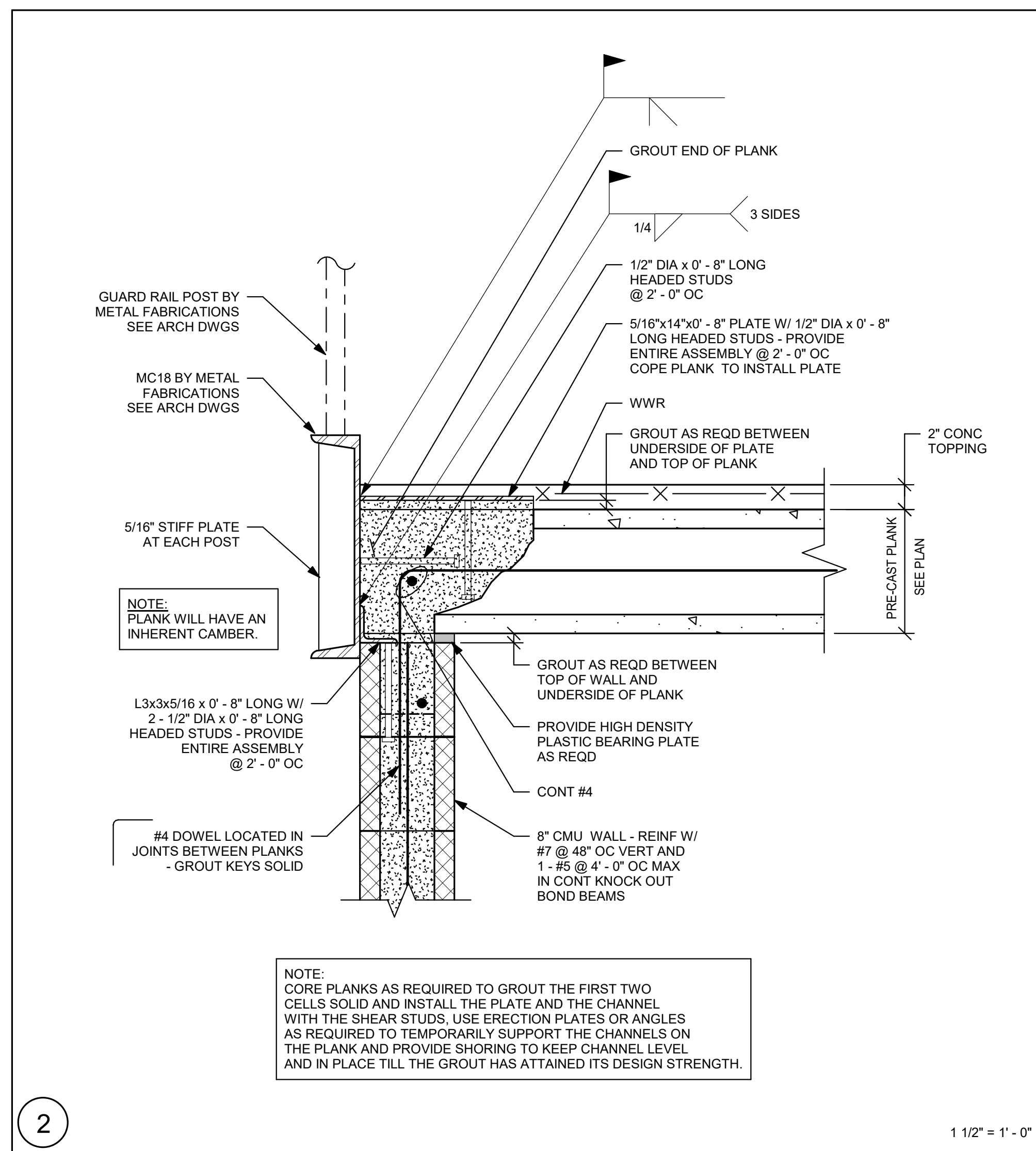
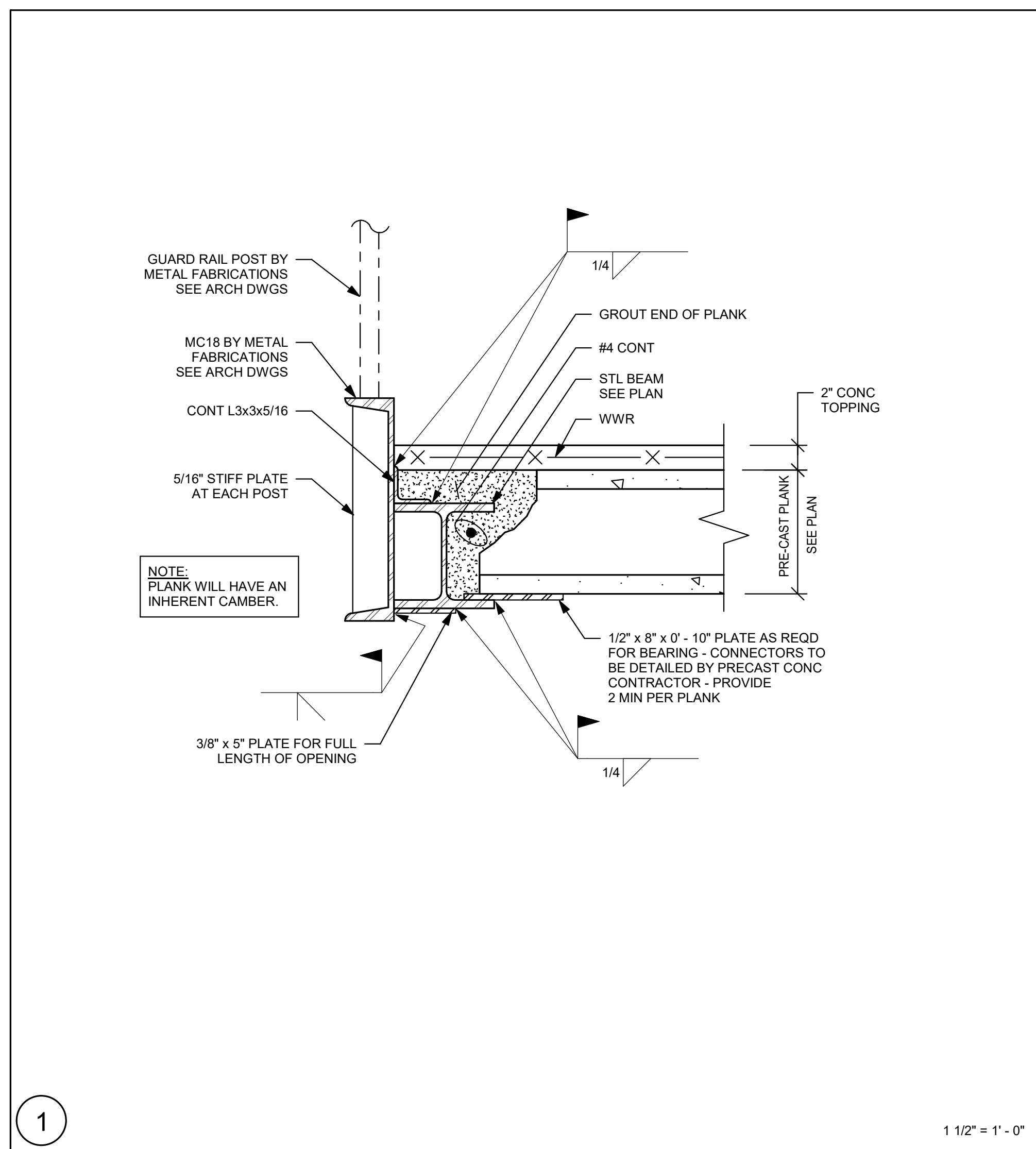
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9



8



NORTHEAST METRO TECH

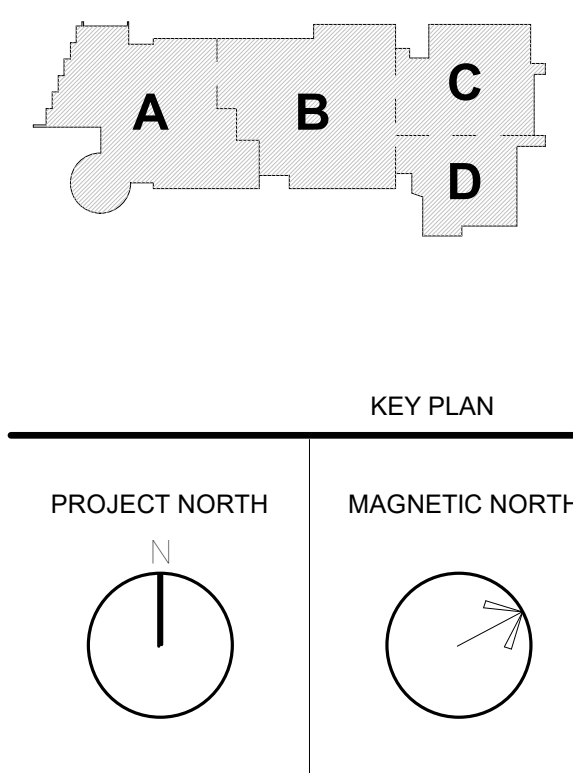
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REVISION LIST	
SS-S-1	4/14/2023 STRUCTURAL STEEL ADDENDUM 1
PR-002	6/29/2023 MISCELLANEOUS STRUCTURAL REVISIONS

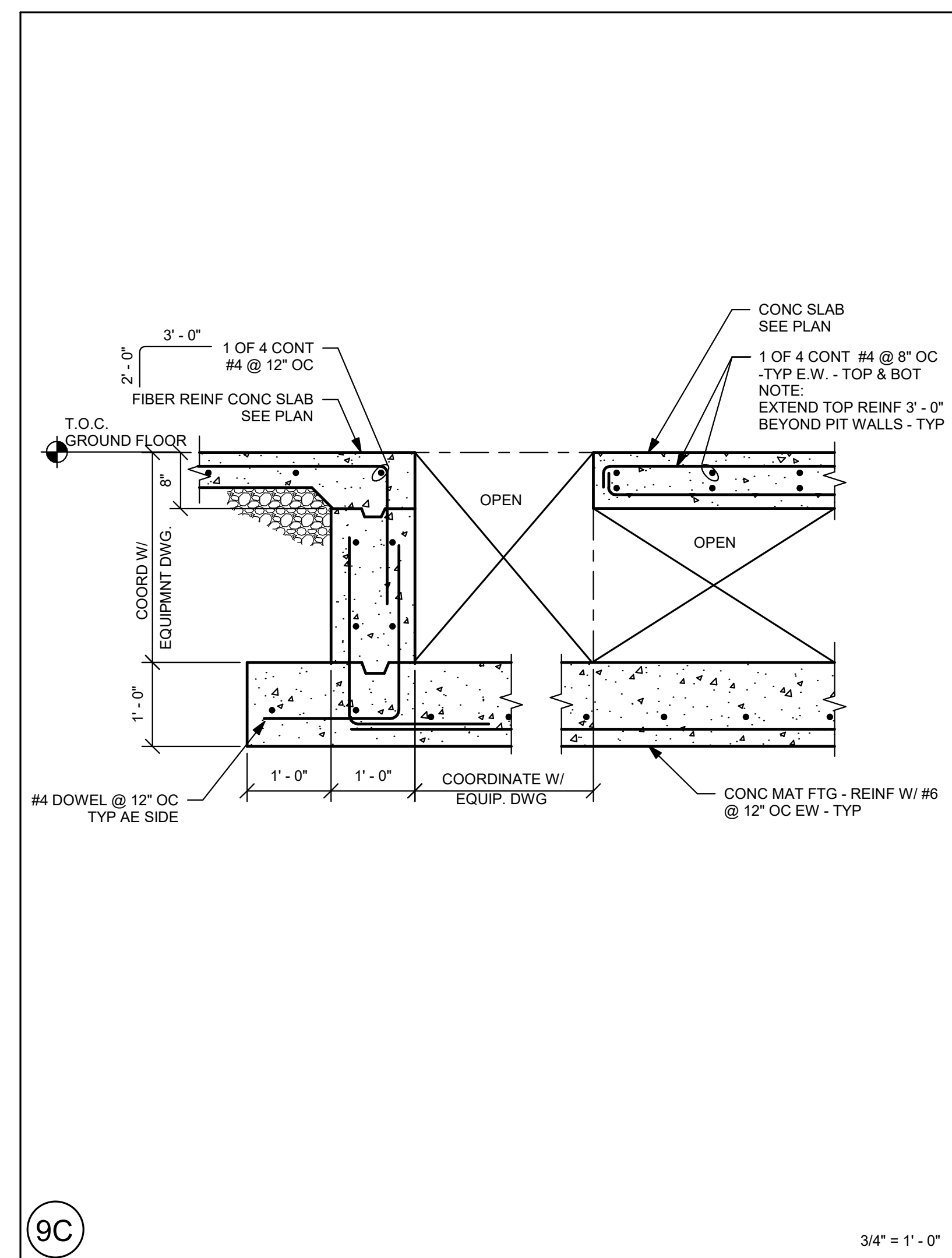
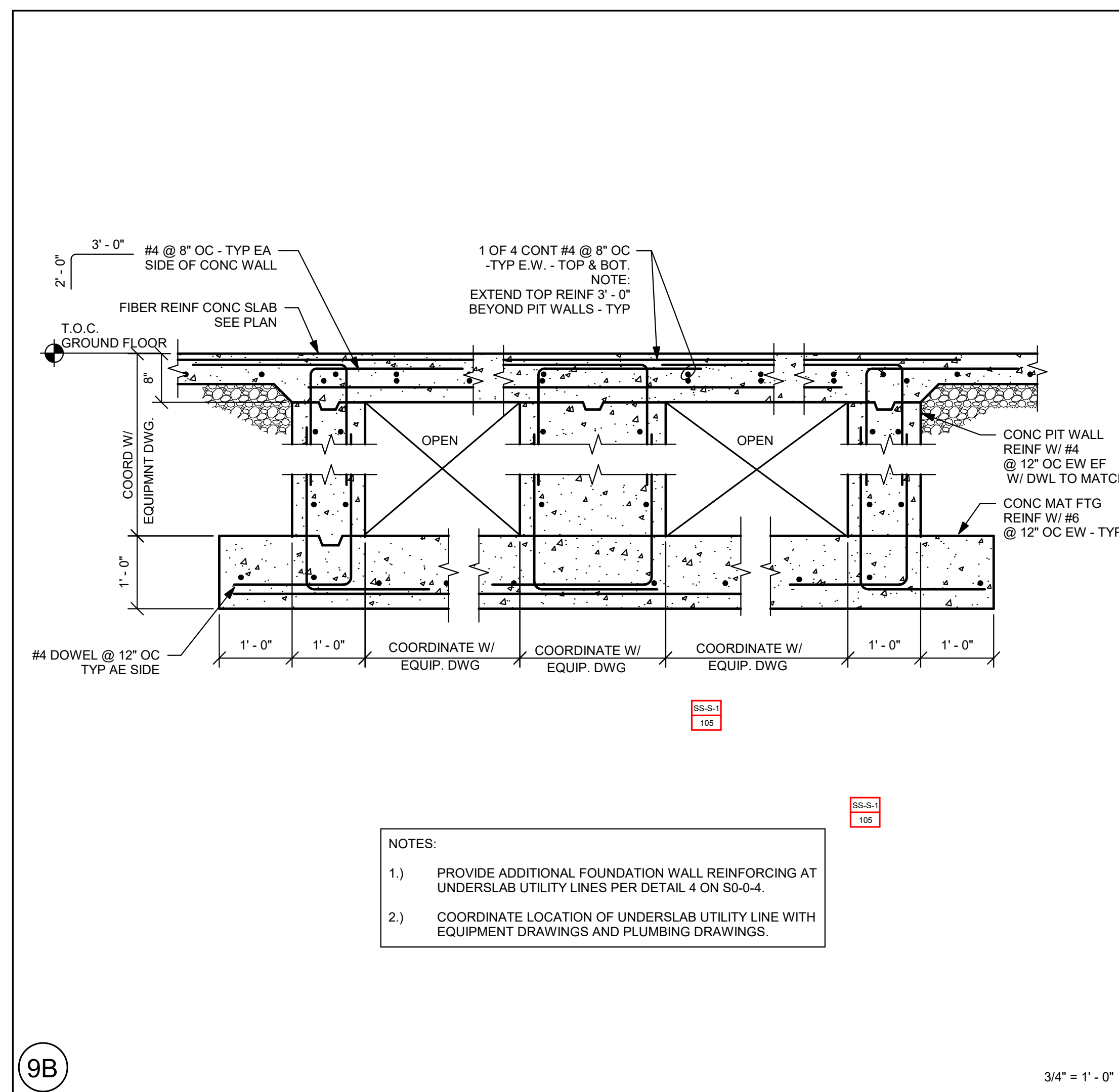
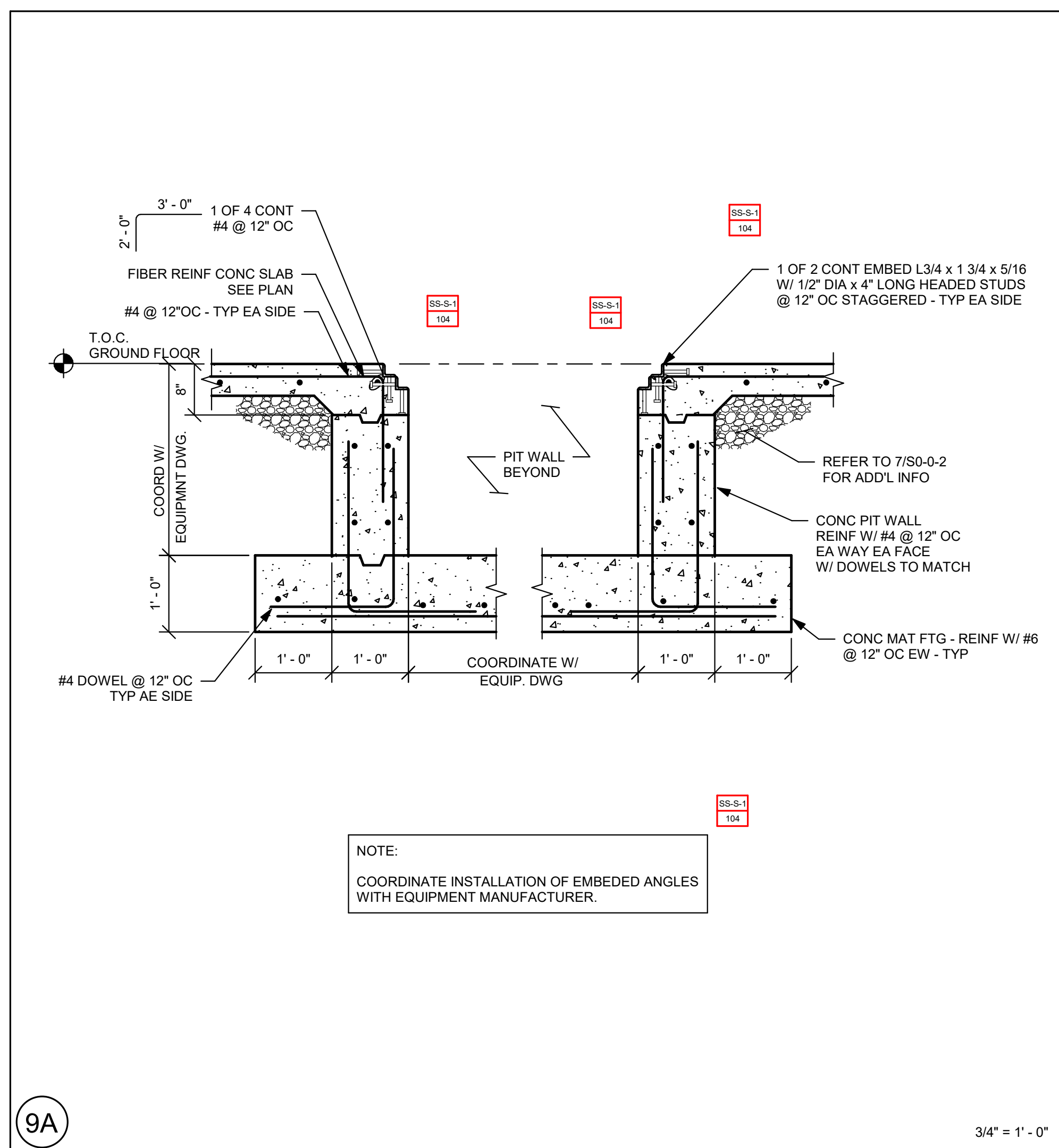
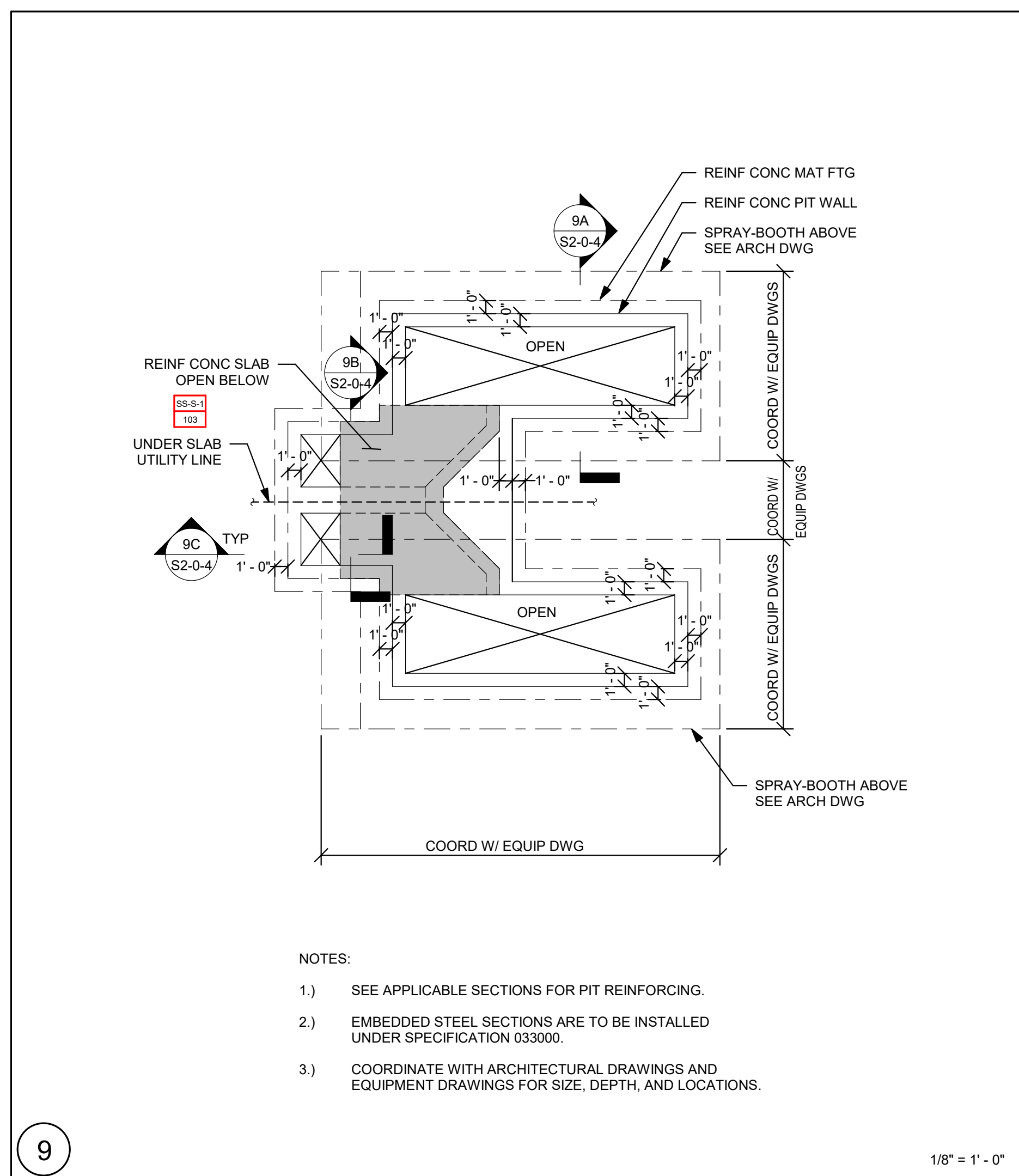
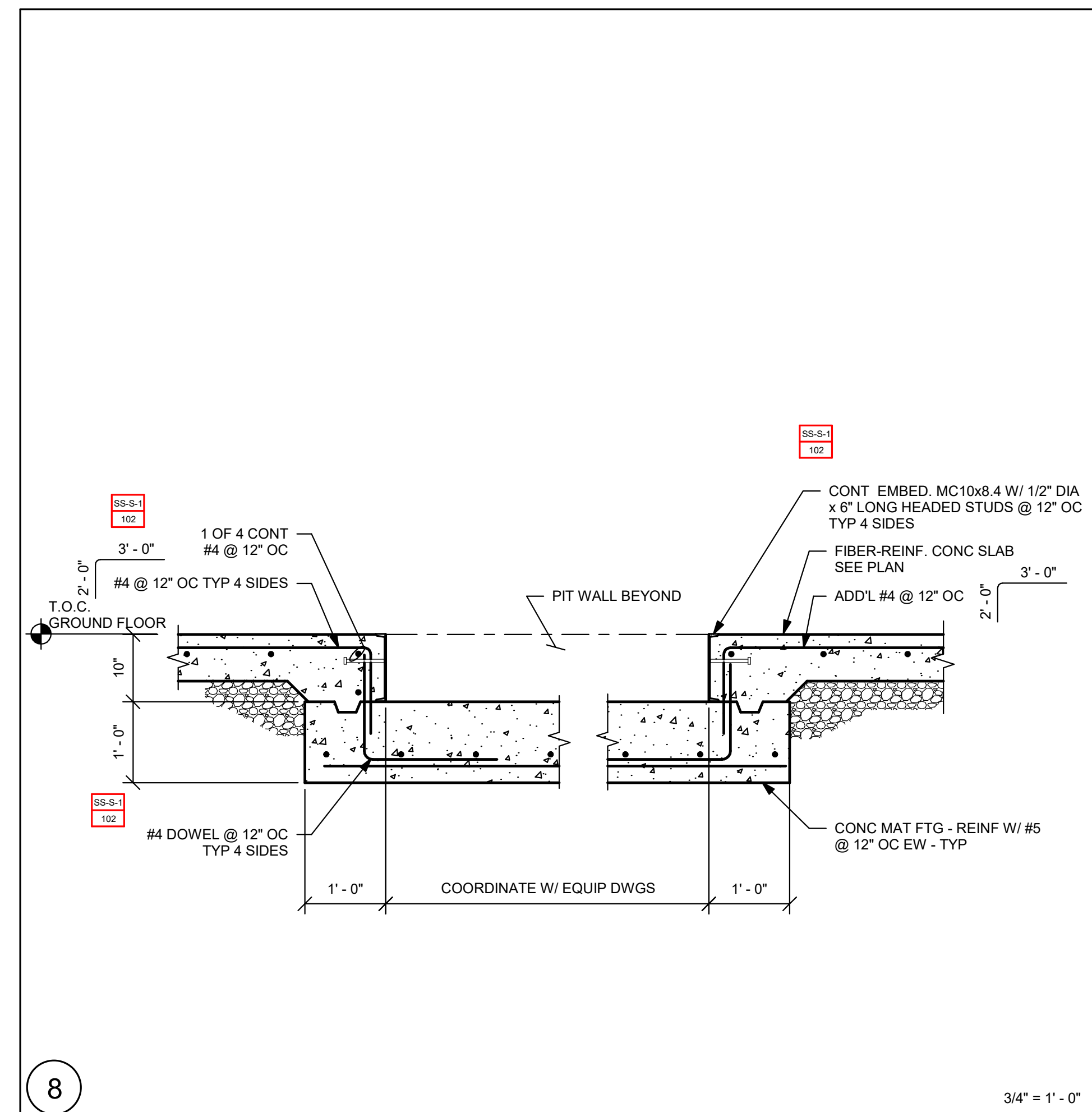
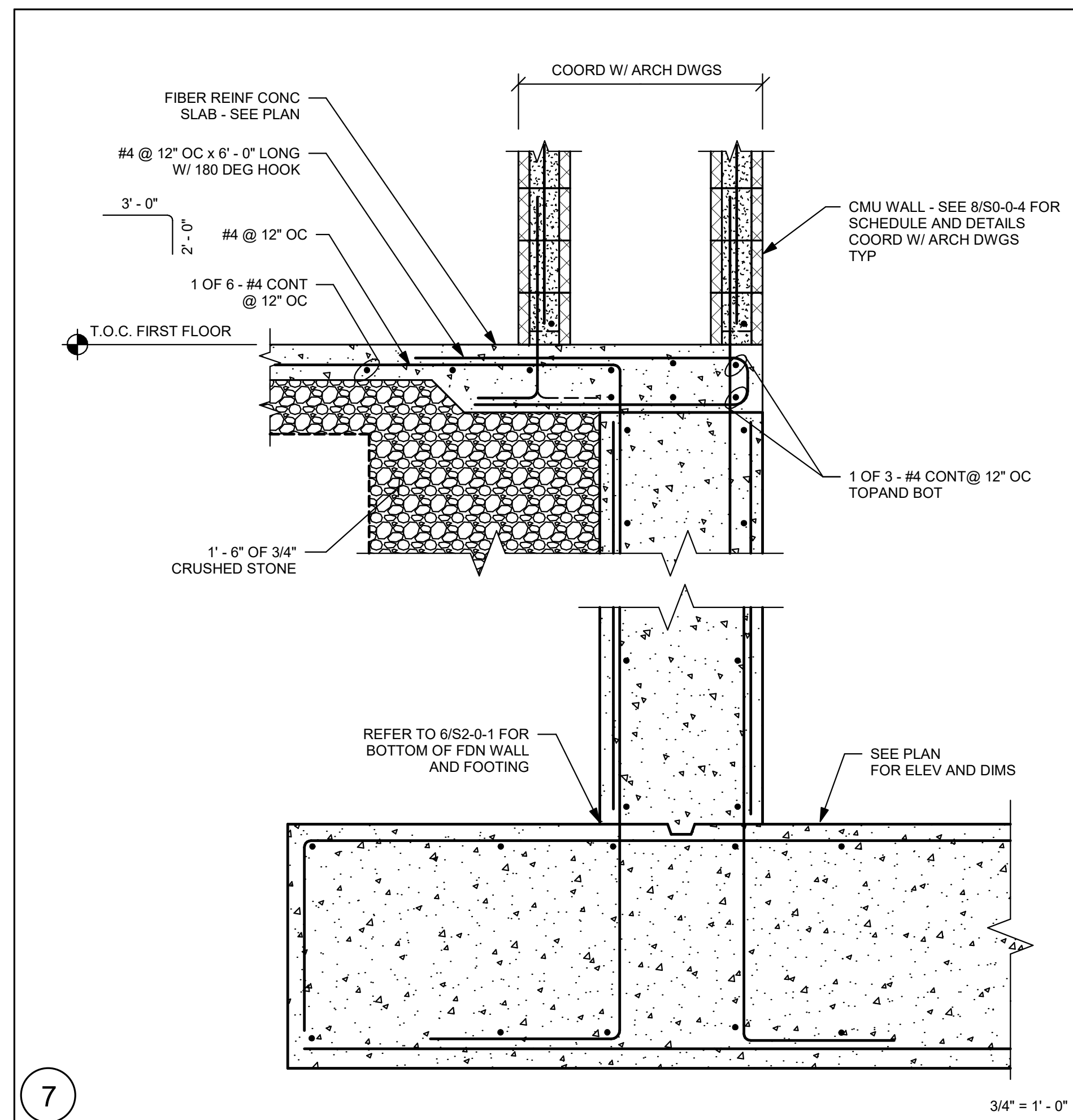
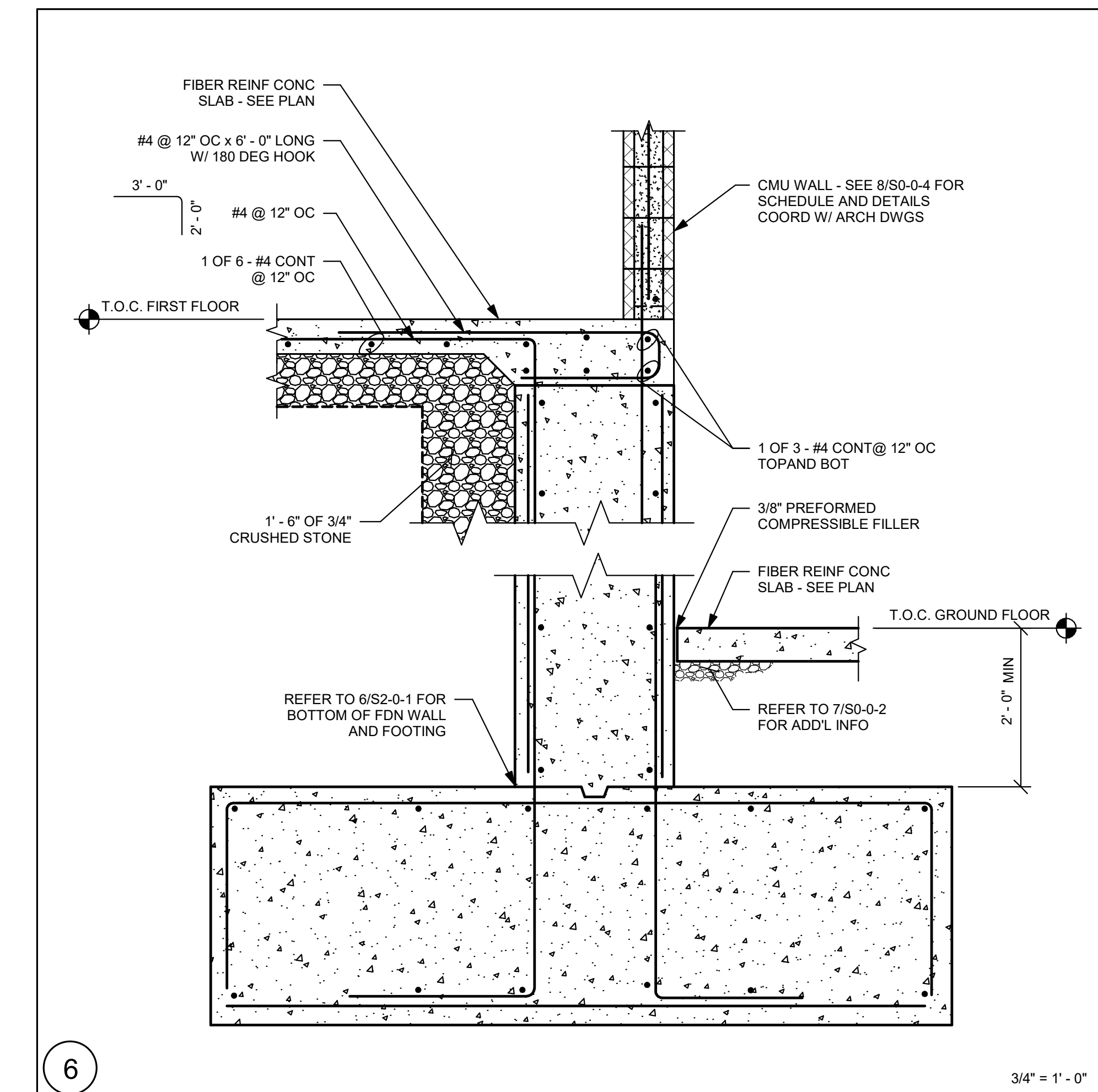
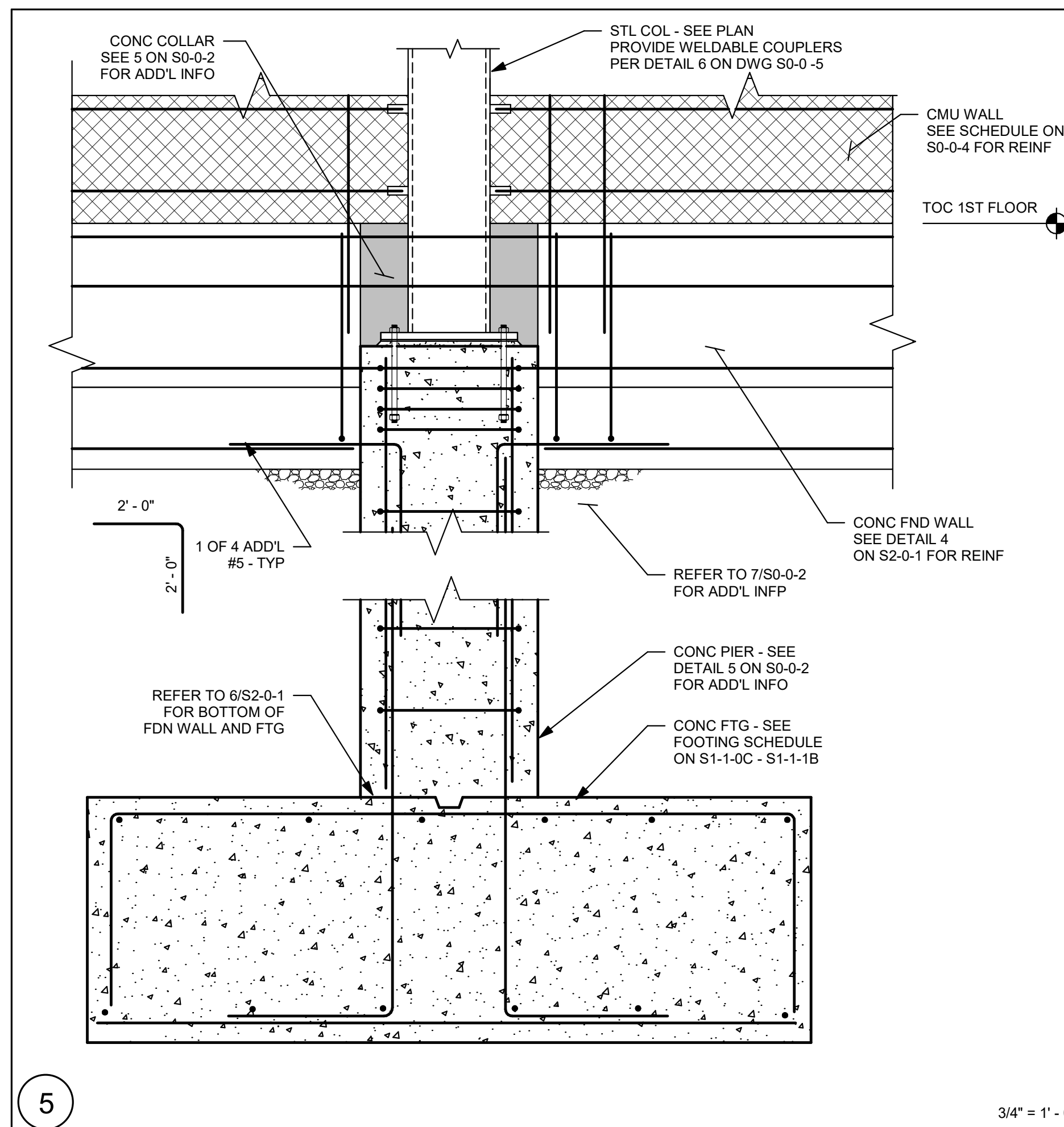
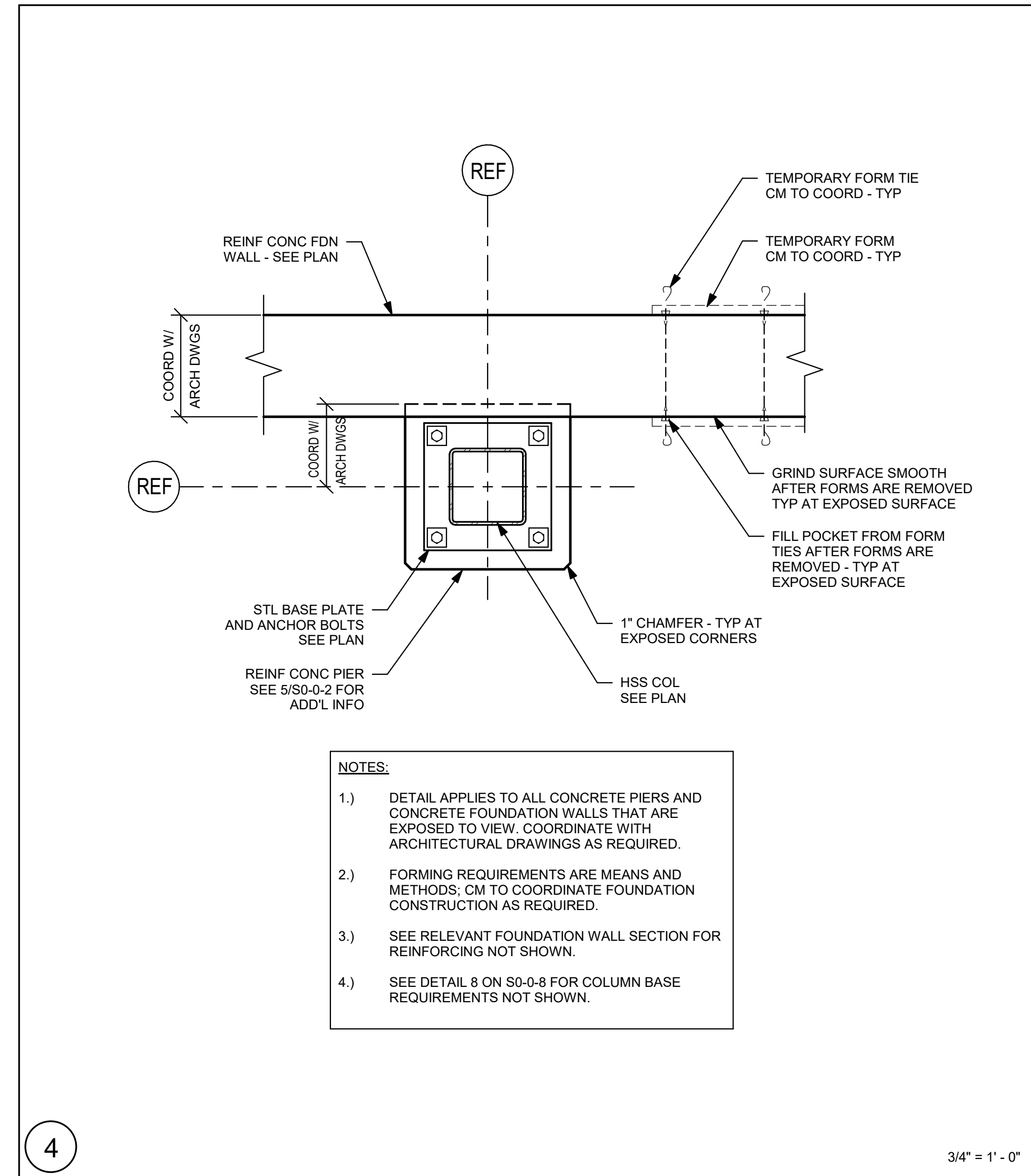
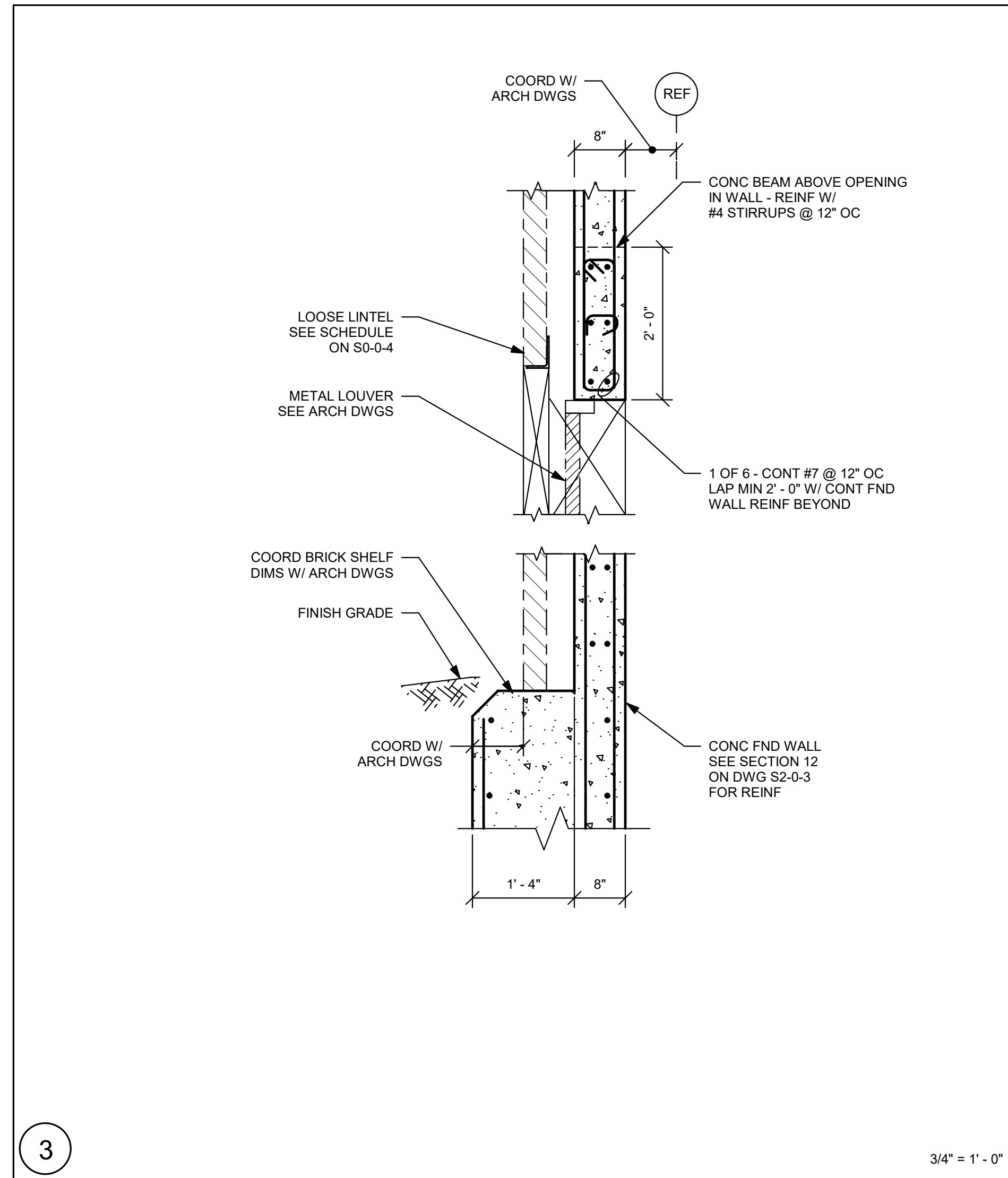
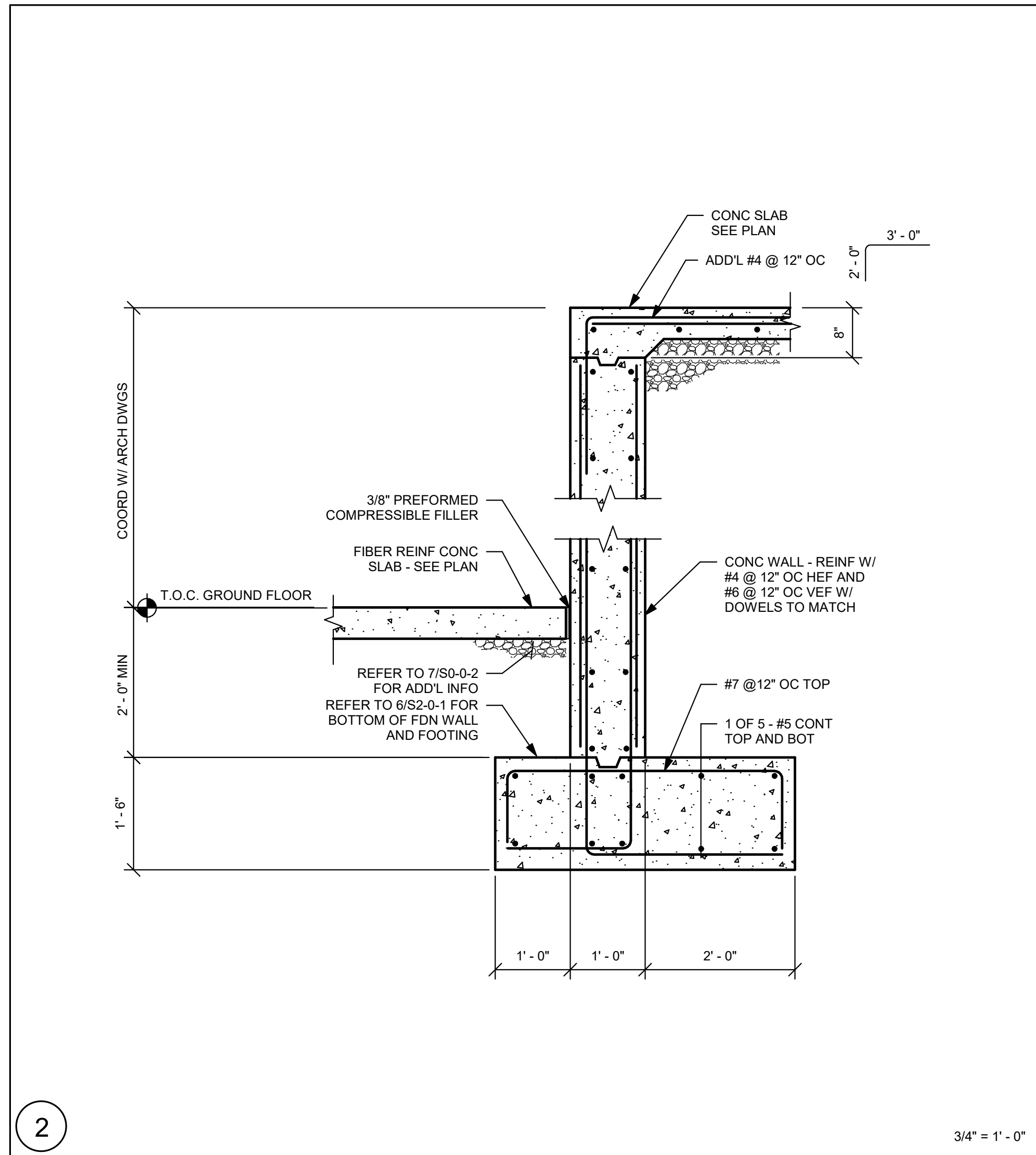
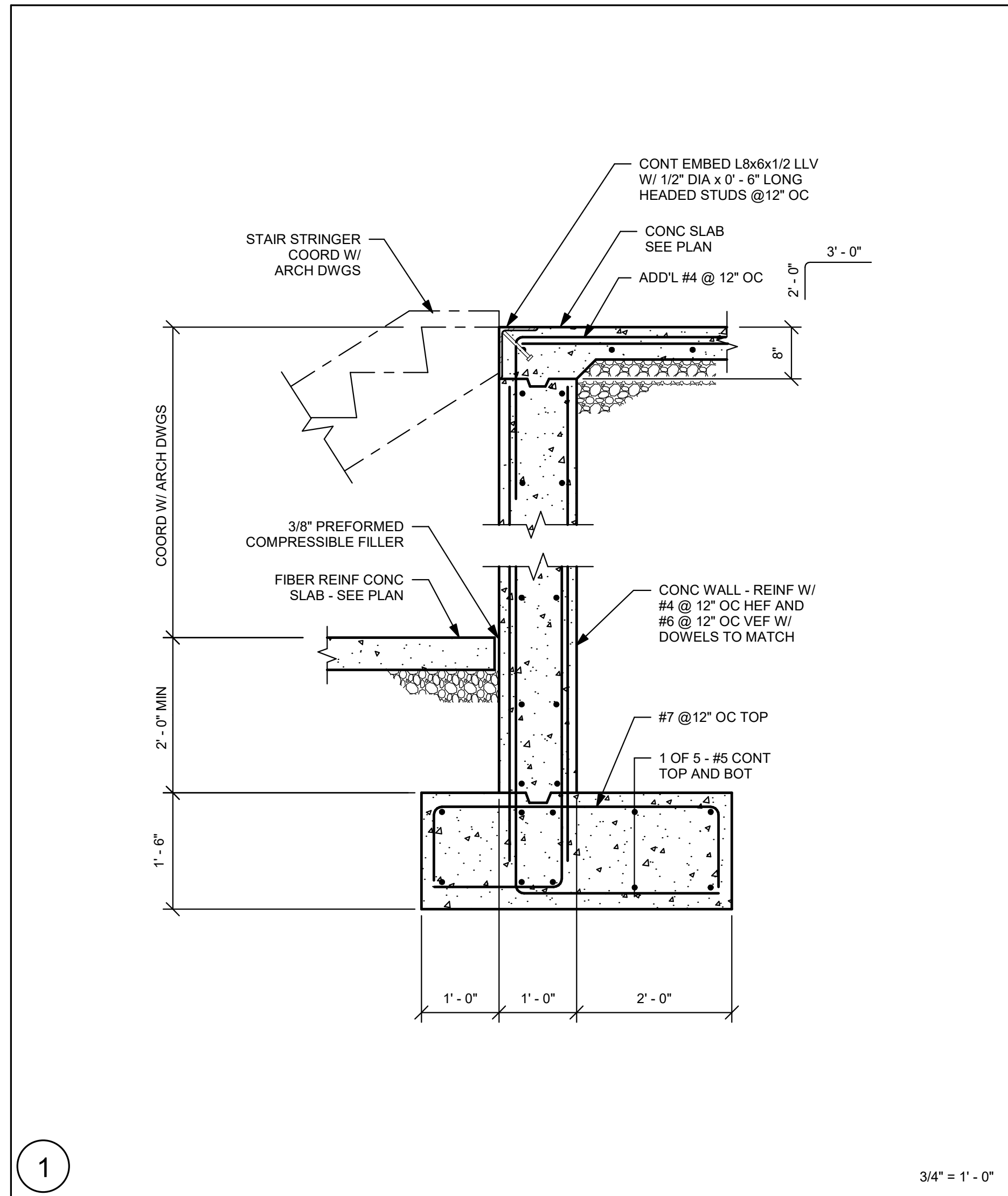
03/31/2023	EARLY STRUCTURAL BID PACKAGE
REVISION LIST	
SS-S-1	4/14/2023 STRUCTURAL STEEL ADDENDUM 1
PR-002	6/29/2023 MISCELLANEOUS STRUCTURAL REVISIONS

BID SET
August 28th, 2023



SECTIONS

Scale: As Indicated
Job No.: 20202
Drawn By: EDG
Date: August 28th, 2023
S2-0-3



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REVISION LIST	
03/12/2023	EARLY STRUCTURAL BID PACKAGE
SS-S-1	4/14/2023 STRUCTURAL STEEL ADDENDUM 1

BID SET

August 28th, 2023

KEY PLAN

PROJECT NORTH

MAGNETIC NORTH

SECTIONS

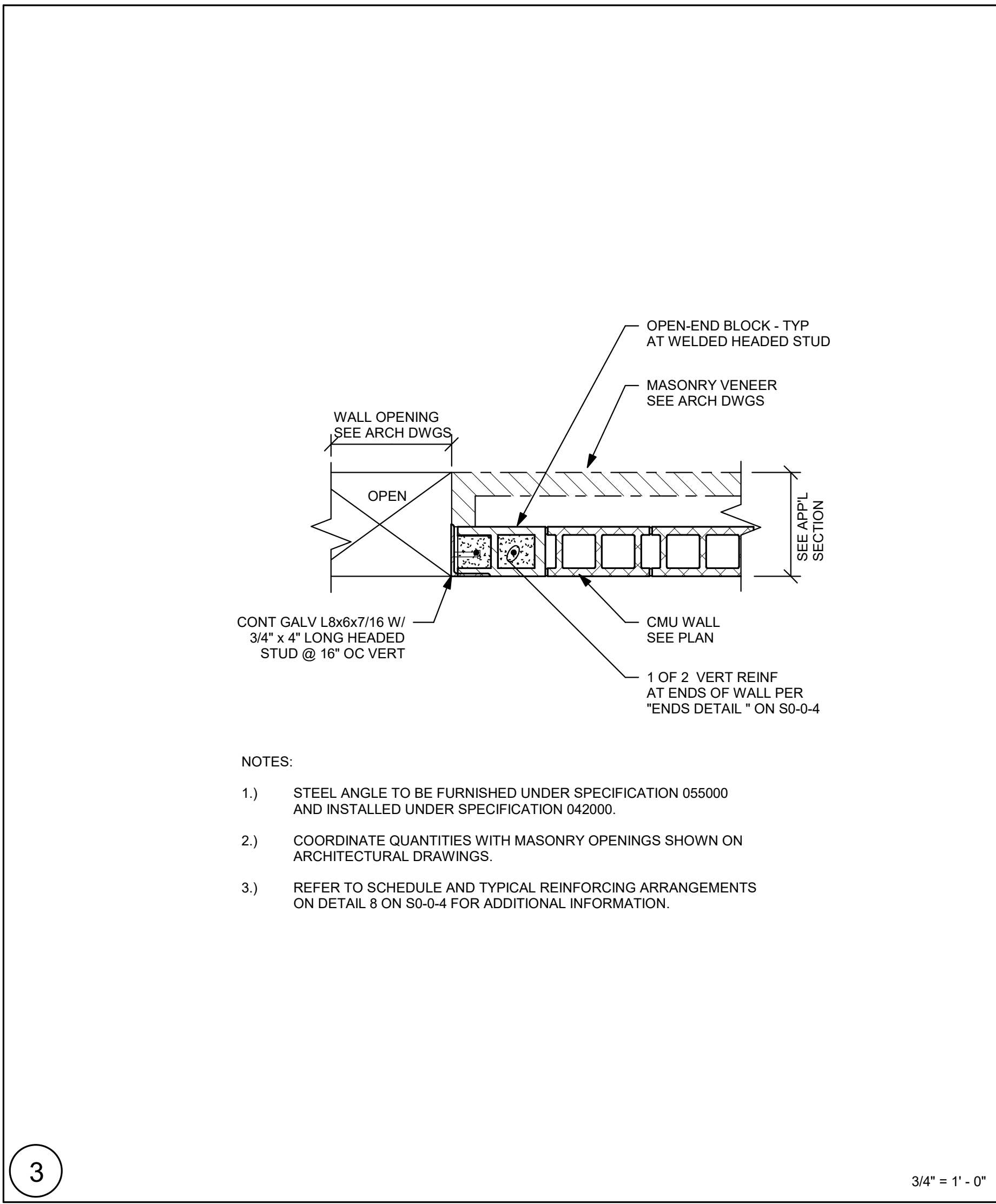
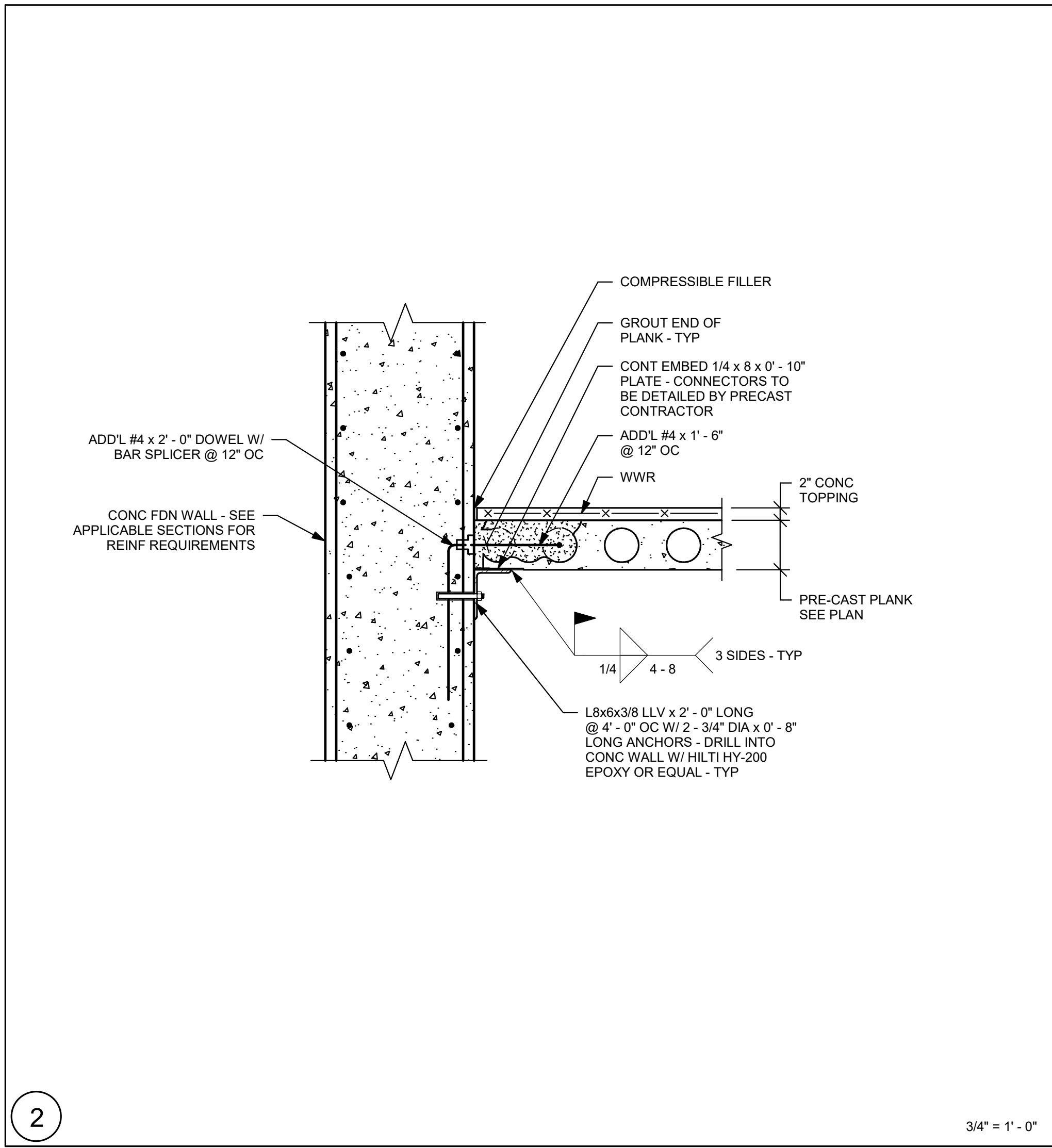
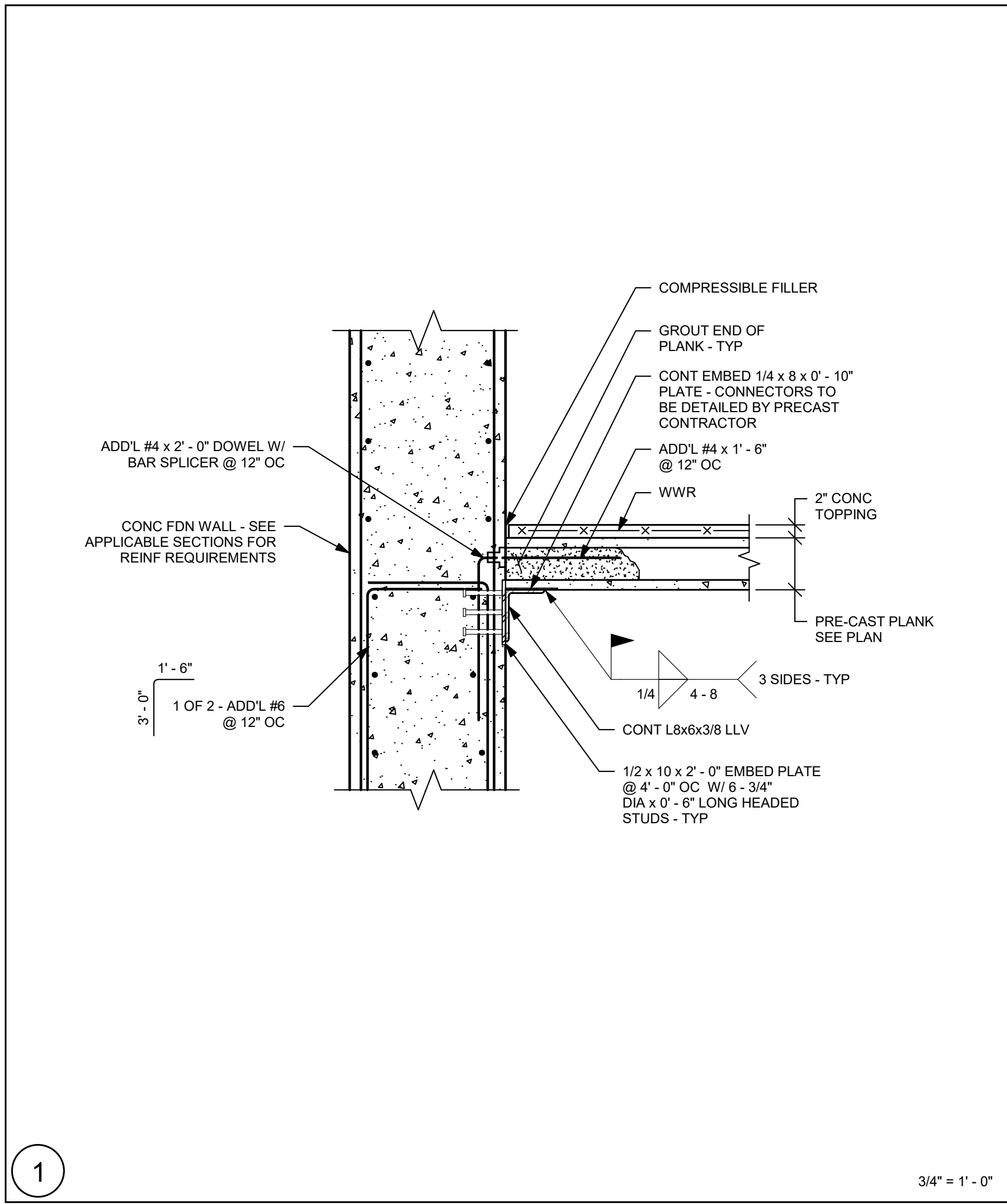
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Job No.: 20202

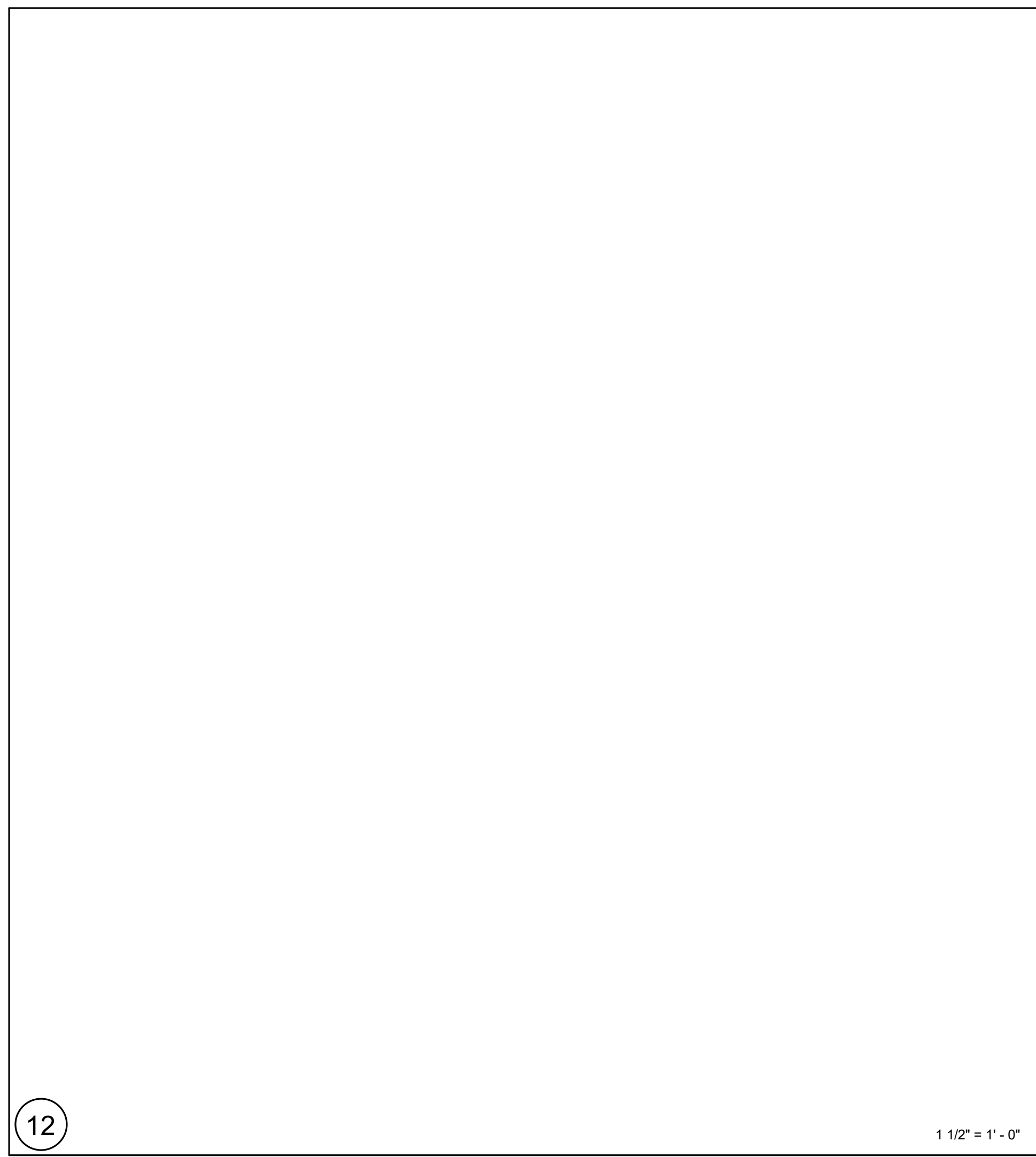
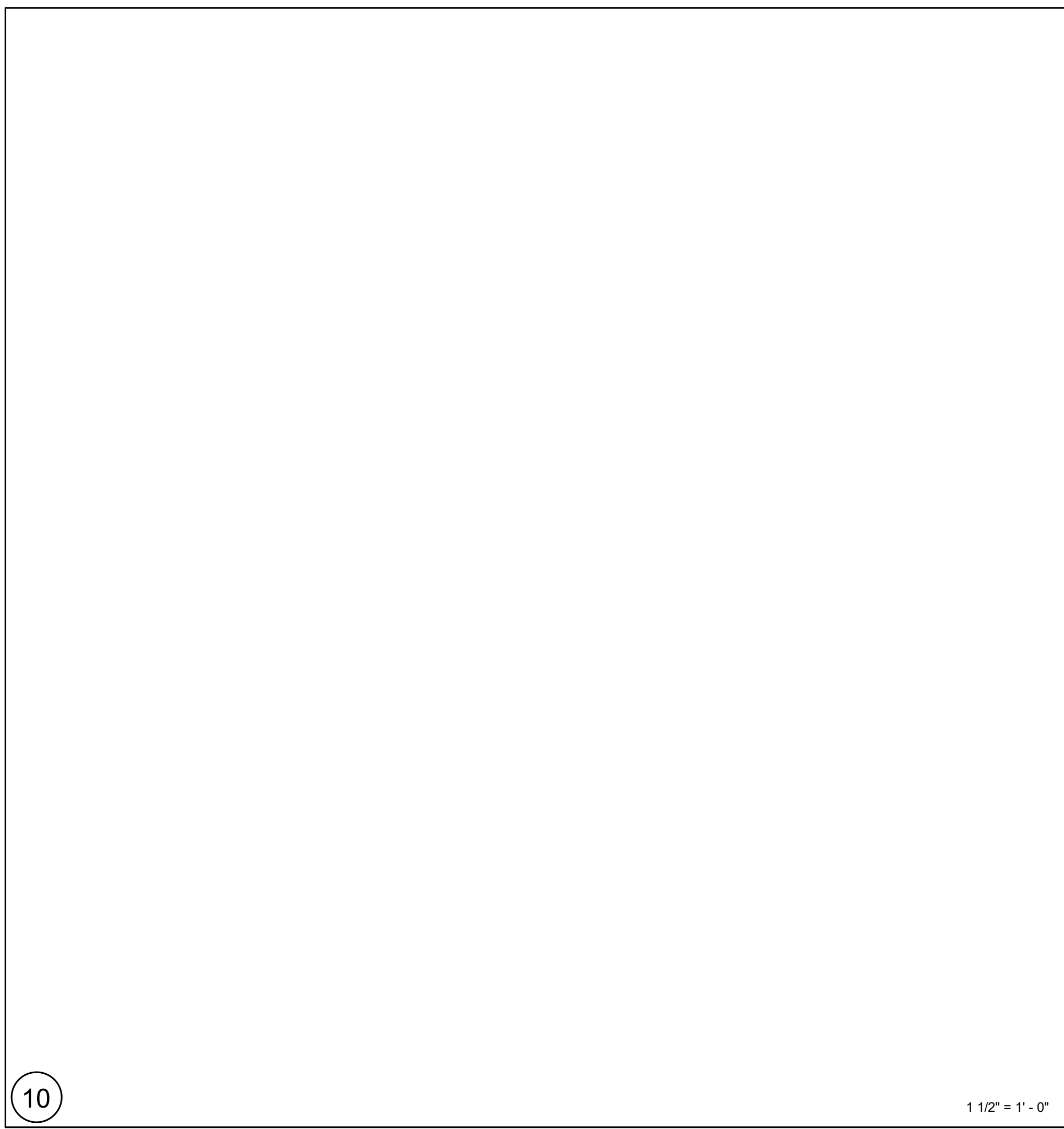
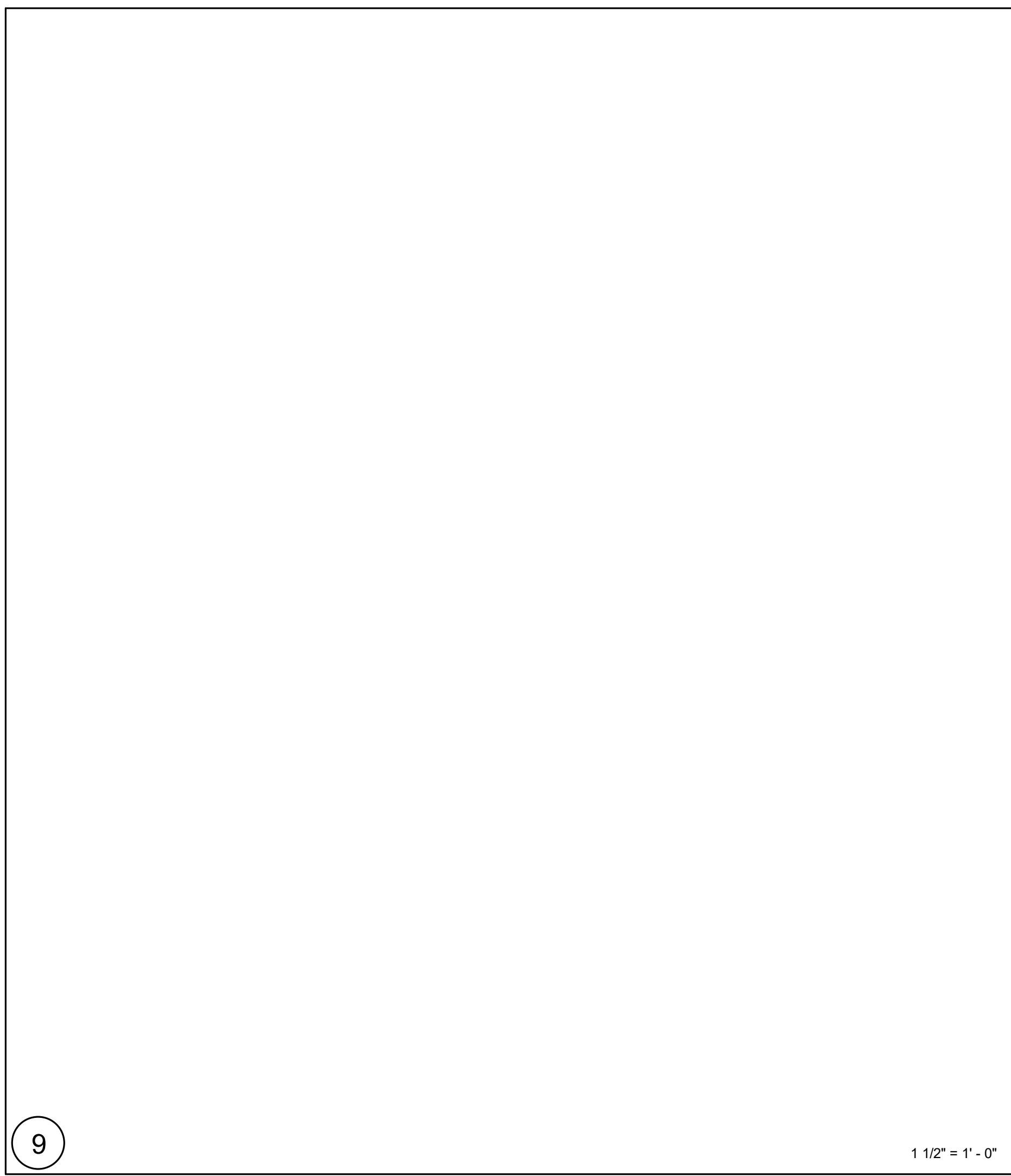
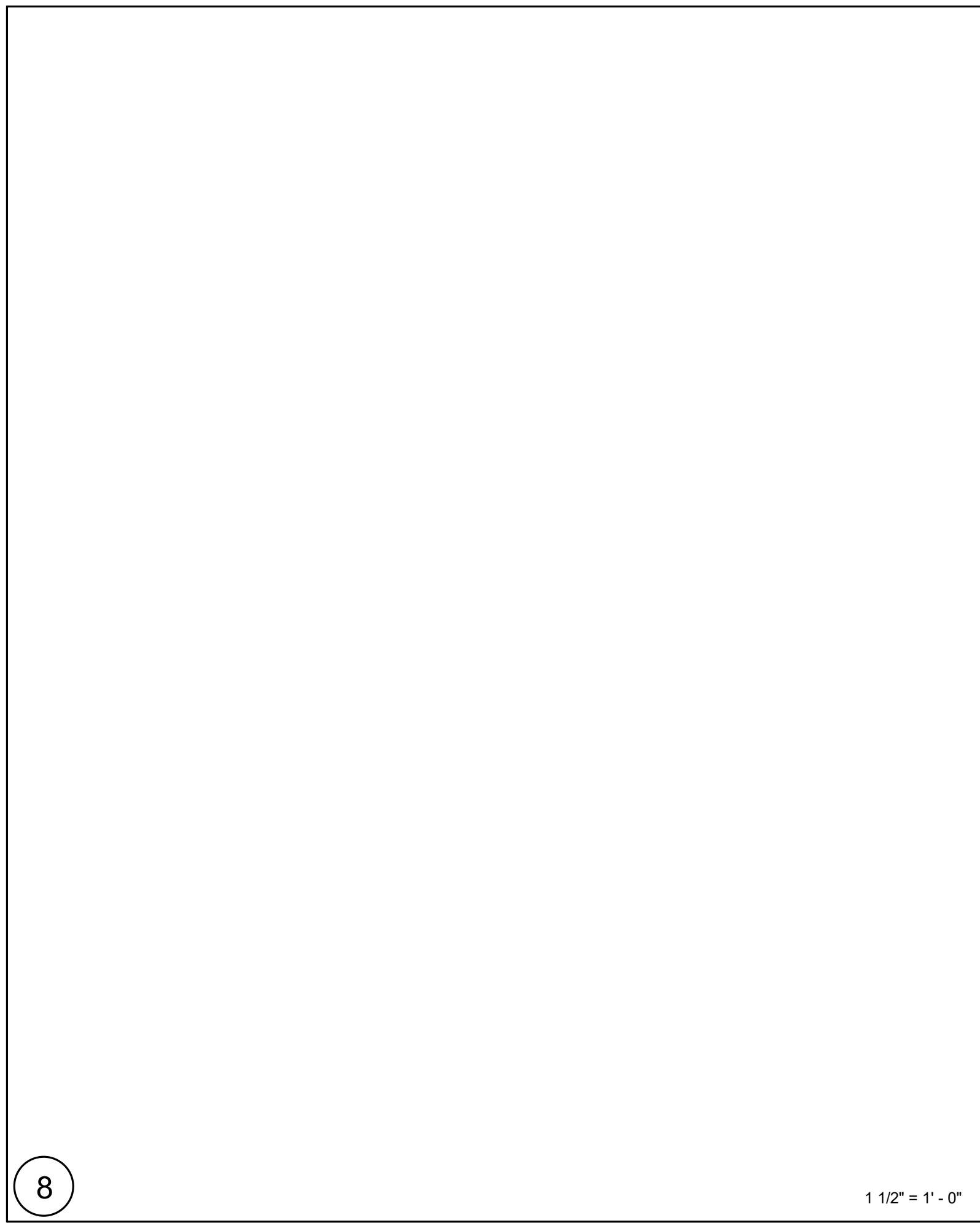
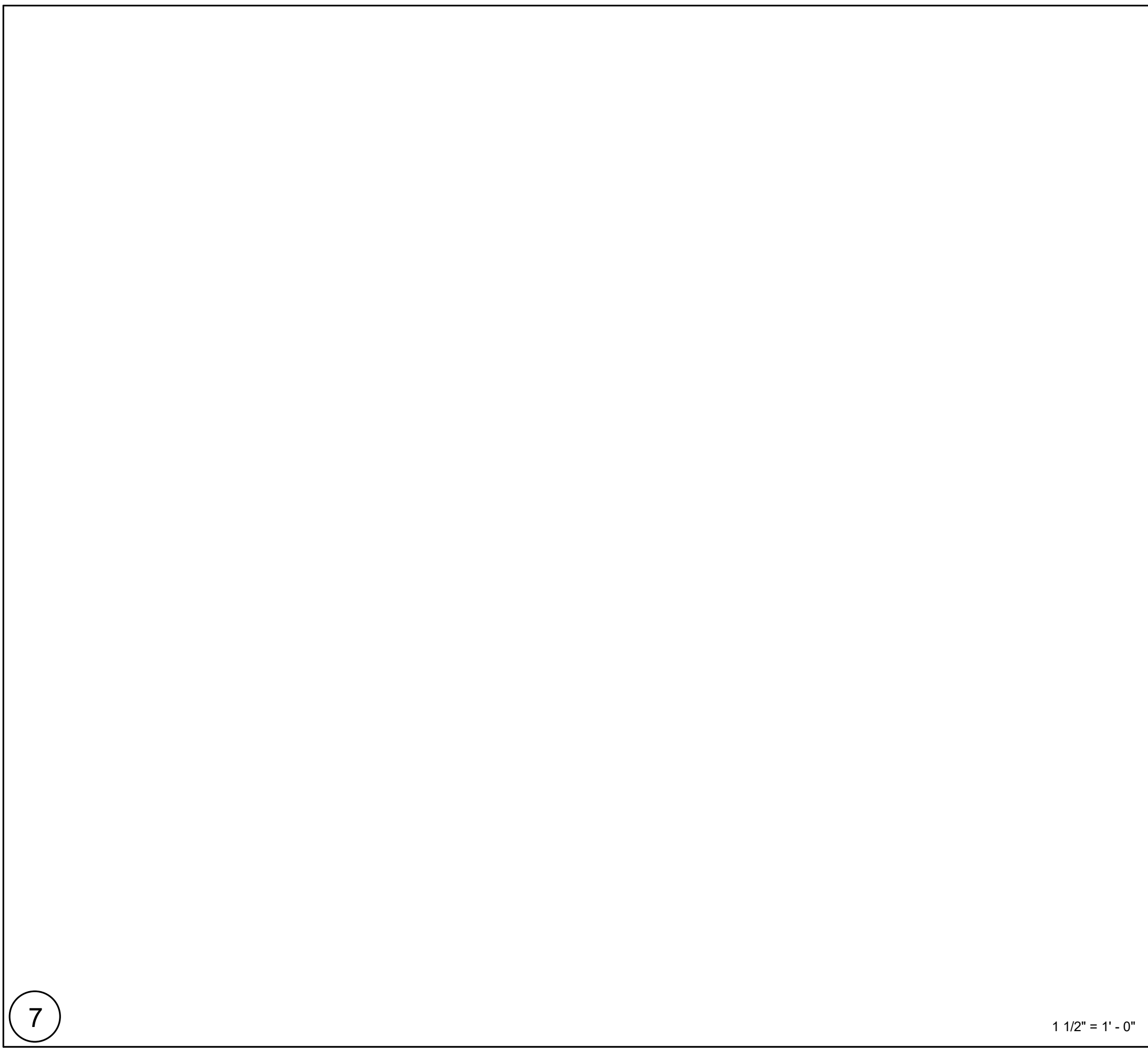
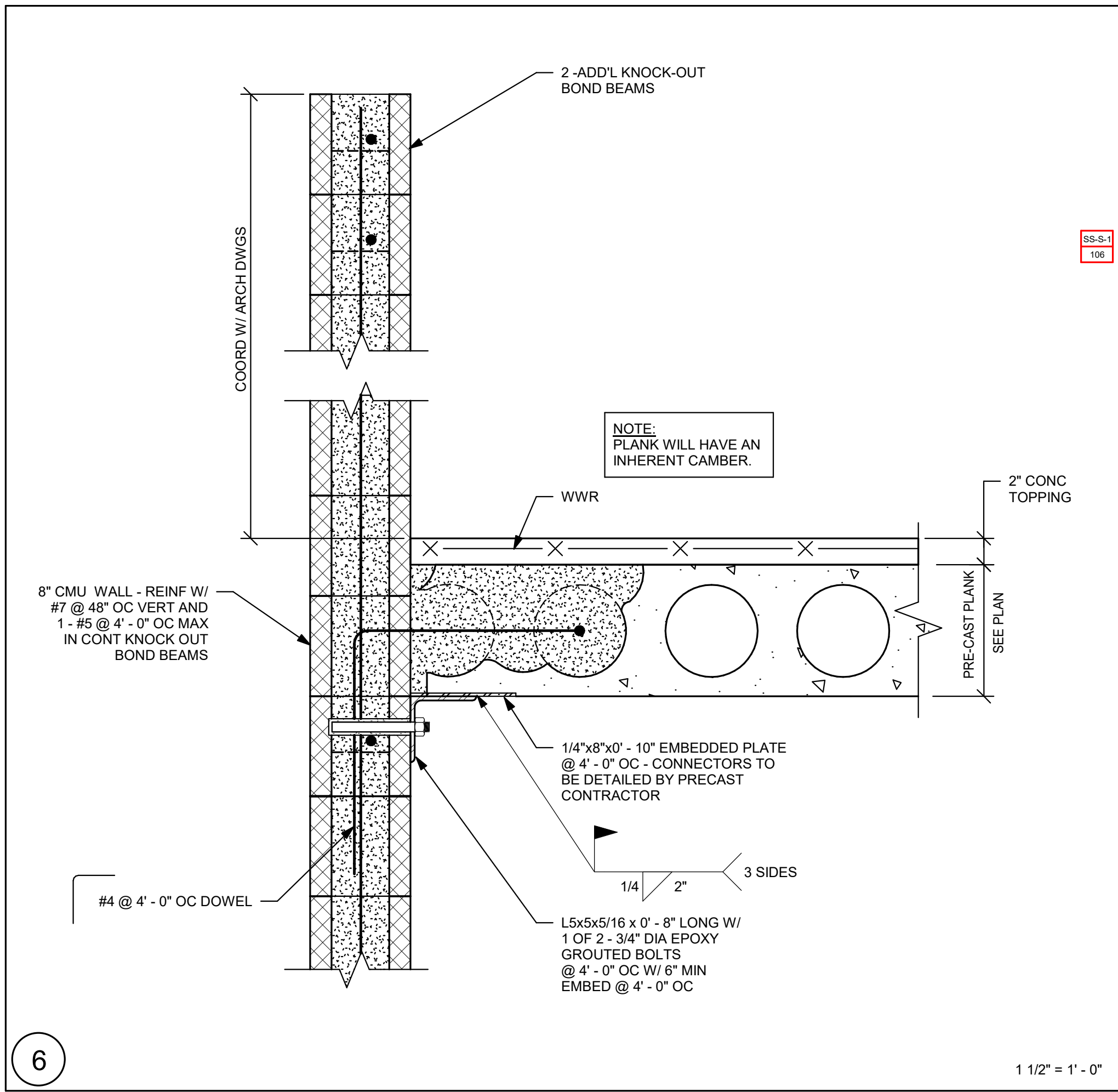
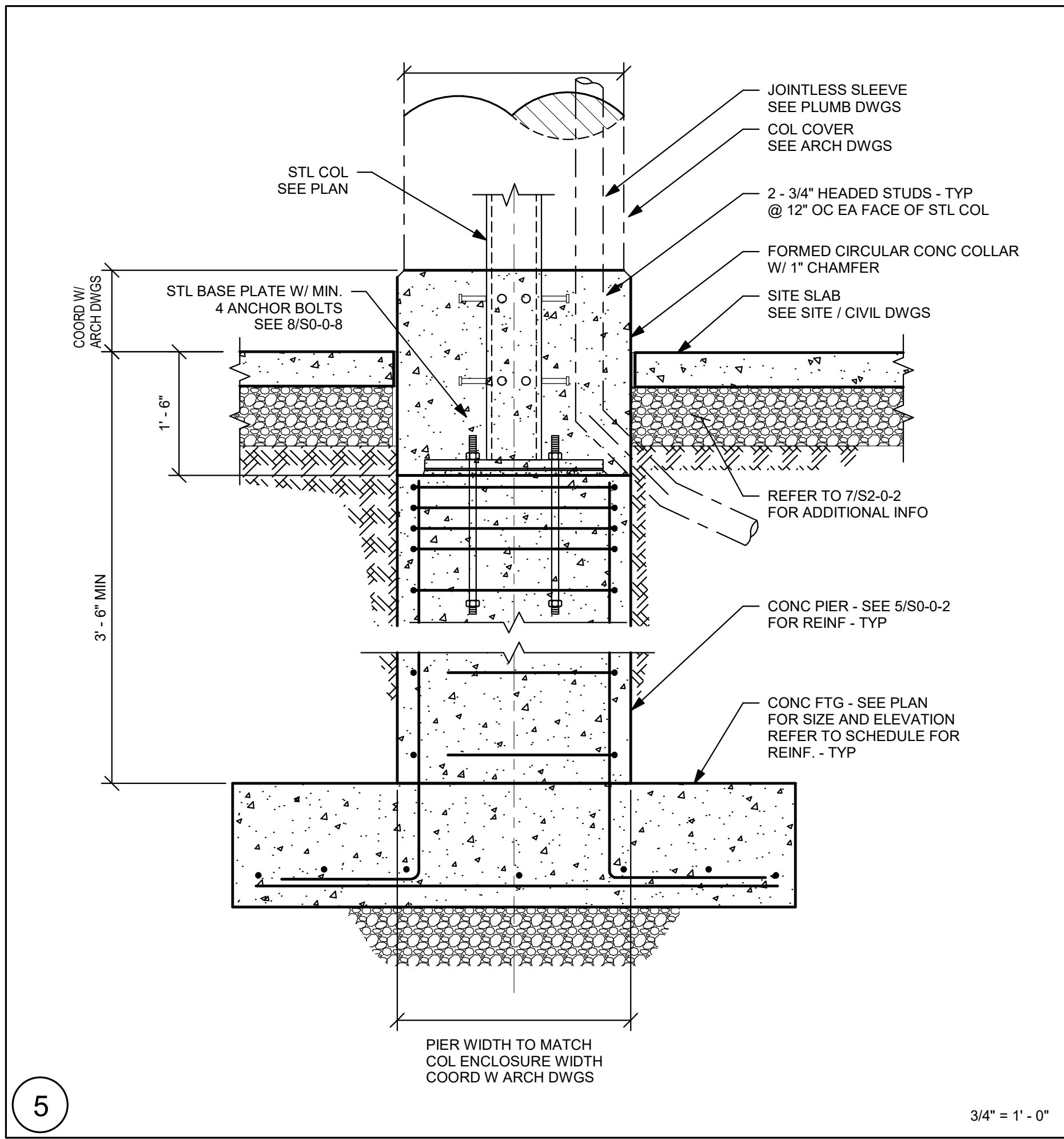
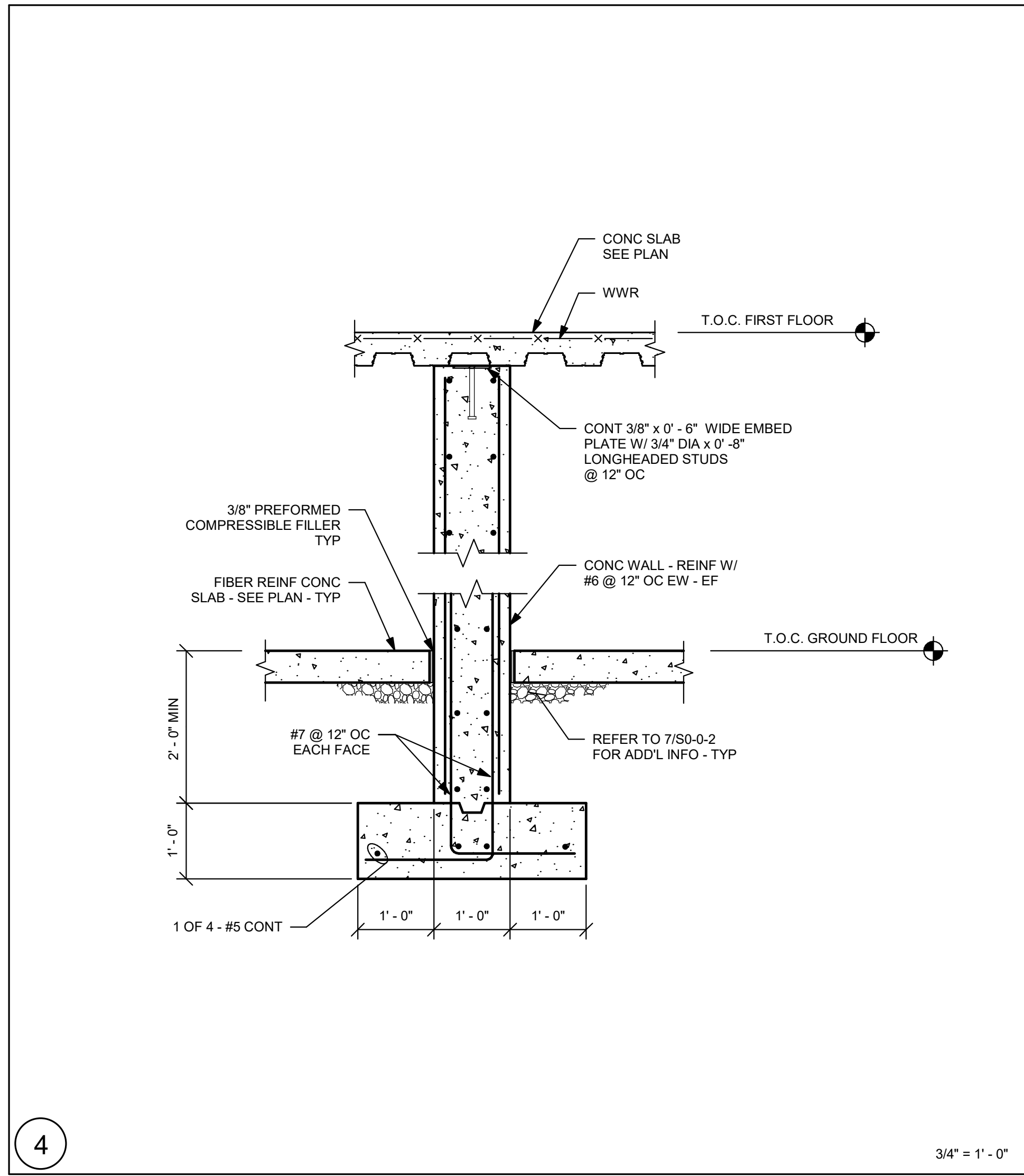
Drawn By: EDG

Date: August 28th, 2023

S2-0-4



- NOTES:
- 1.) STEEL ANGLE TO BE FURNISHED UNDER SPECIFICATION 055000 AND INSTALLED UNDER SPECIFICATION 042000.
 - 2.) COORDINATE QUANTITIES WITH MASONRY OPENINGS SHOWN ON ARCHITECTURAL DRAWINGS.
 - 3.) REFER TO SCHEDULE AND TYPICAL REINFORCING ARRANGEMENTS ON DETAIL S ON S0-0-4 FOR ADDITIONAL INFORMATION.



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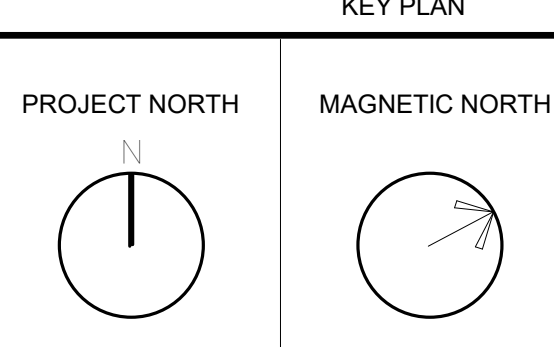
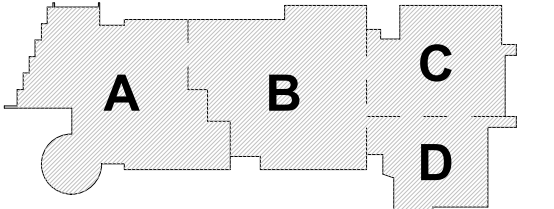
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BID SET

August 28th, 2023



SECTIONS

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Job No.: 20202
Drawn By: EDG
Date: August 28th, 2023
S2-0-5

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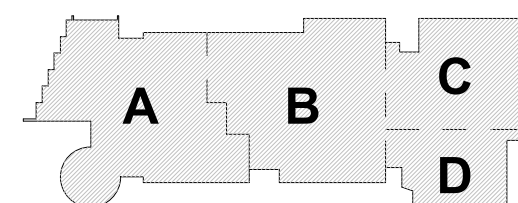
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REVISION LIST	
SS-S-1	4/14/2023 STRUCTURAL STEEL ADDENDUM 1

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BID SET

August 28th, 2023



KEY PLAN

PROJECT NORTH
MAGNETIC NORTH

SECTIONS

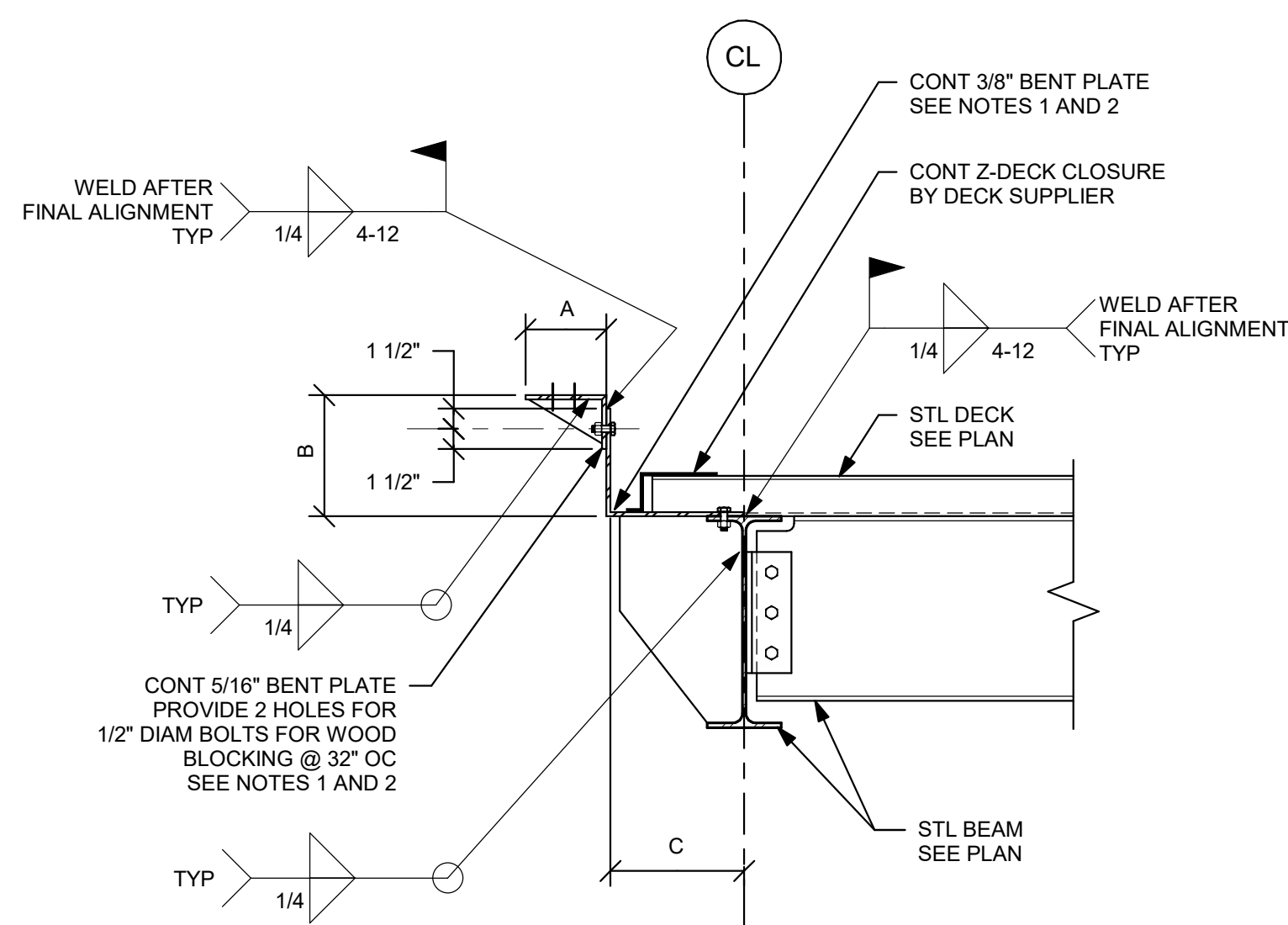
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Job No.: 20202

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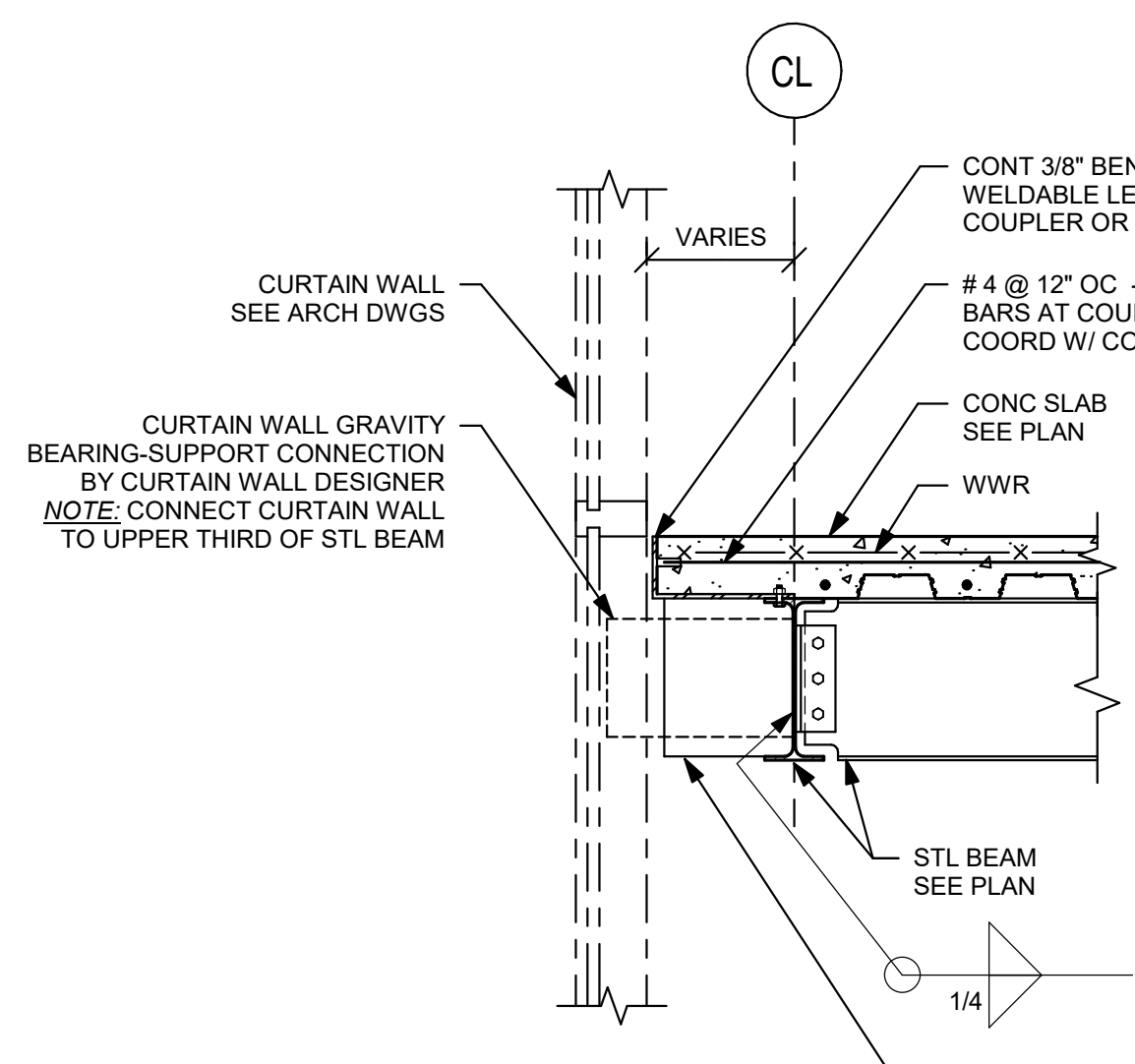
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S3-0-1

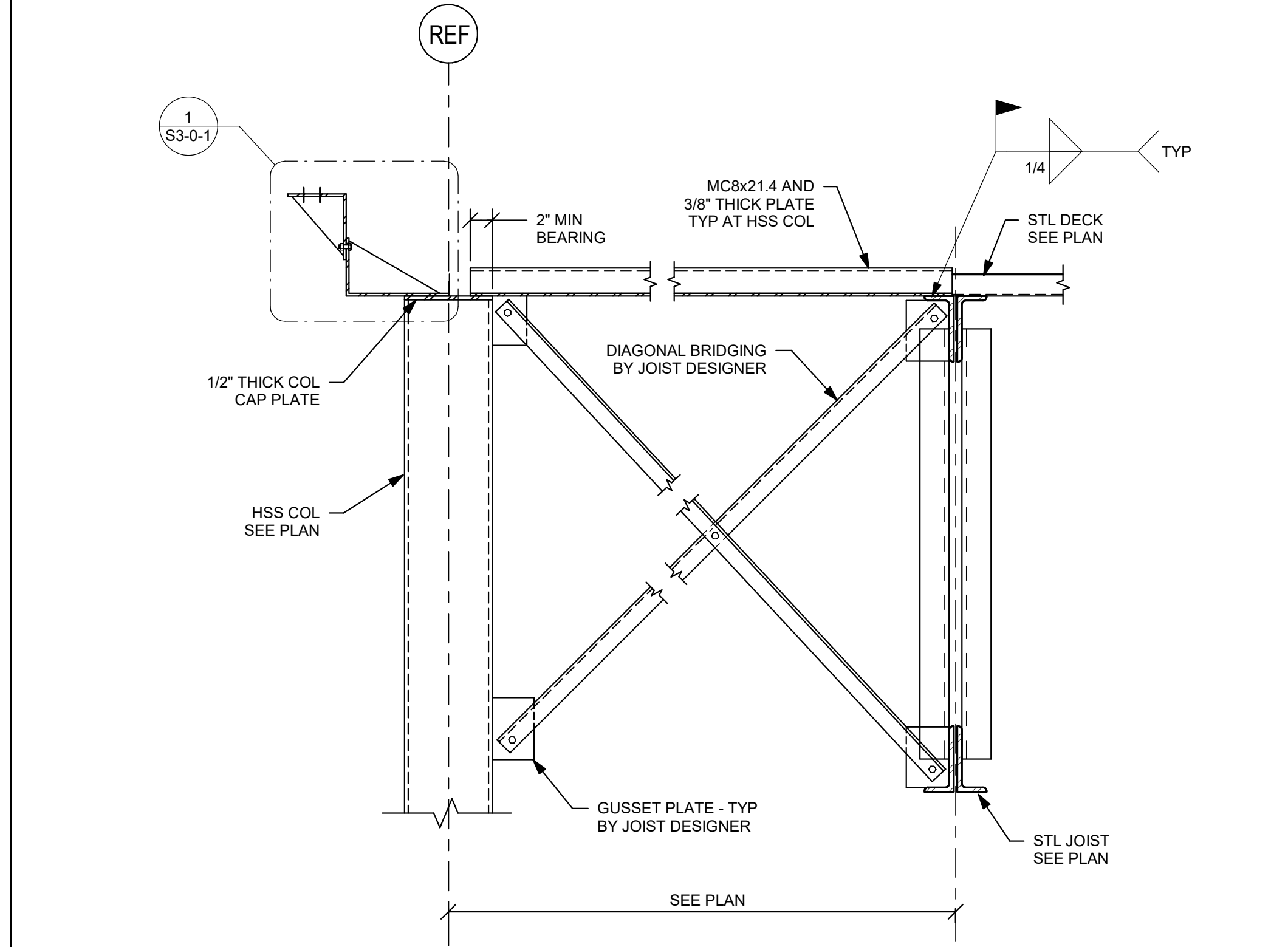
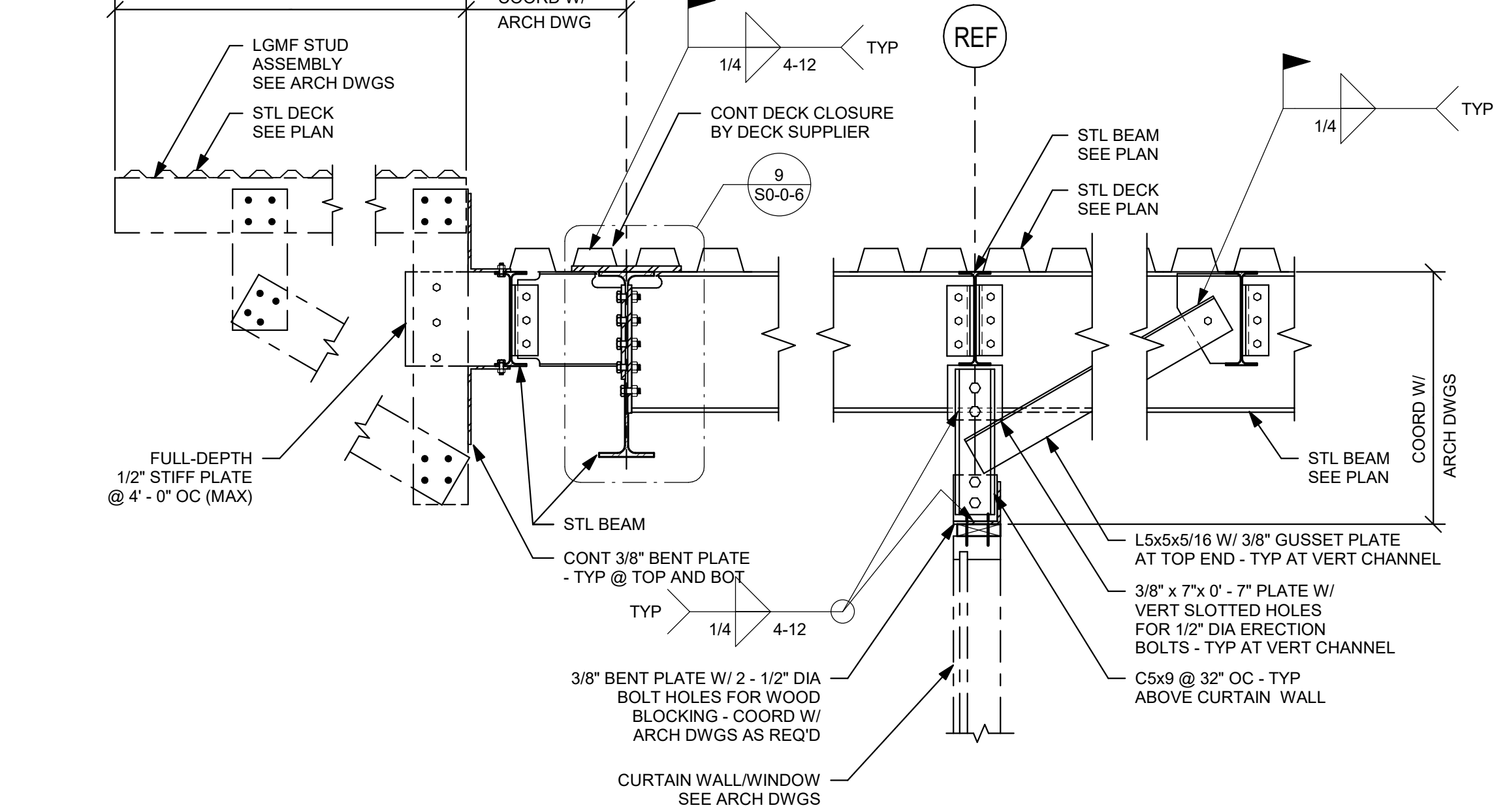
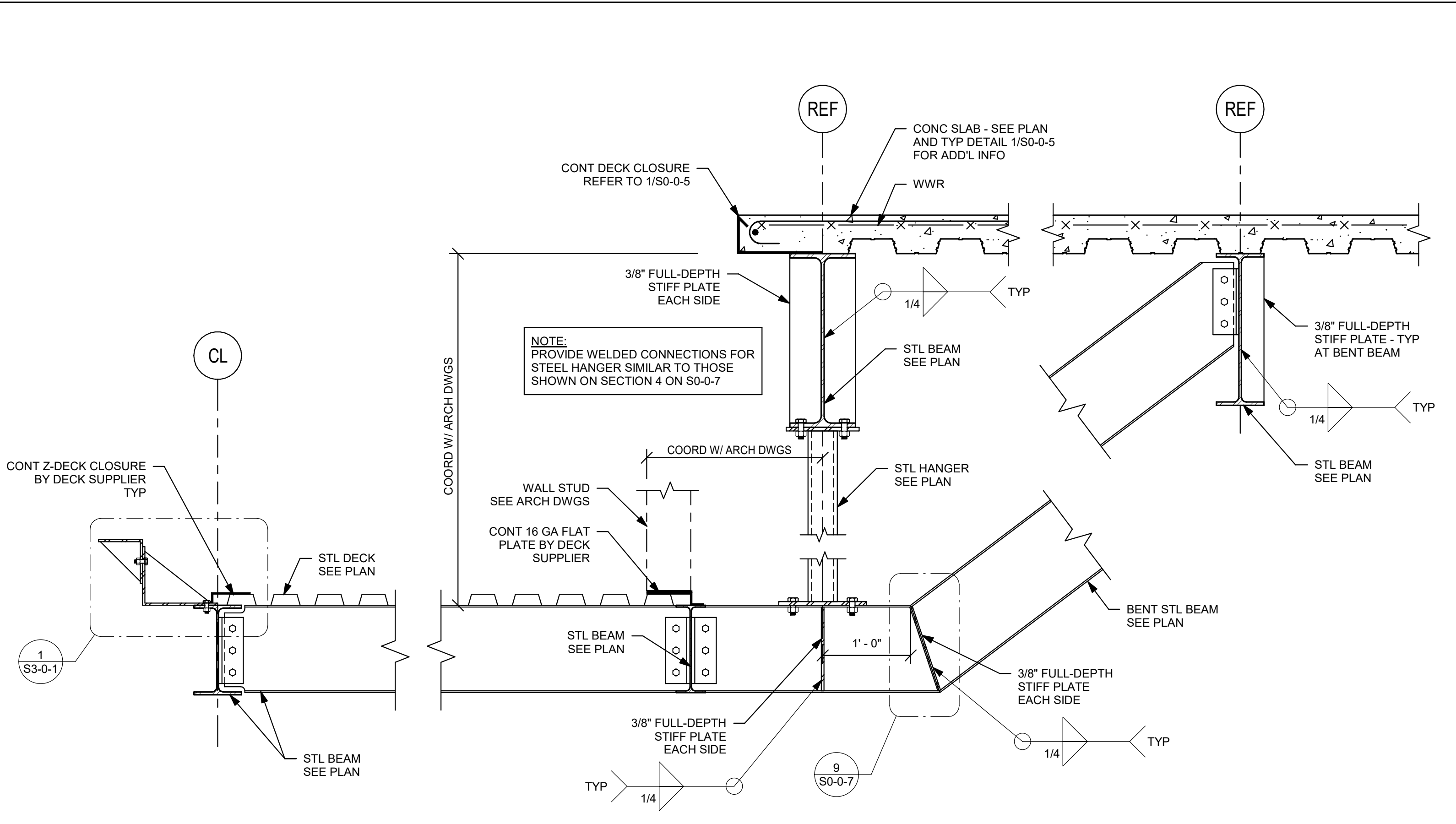
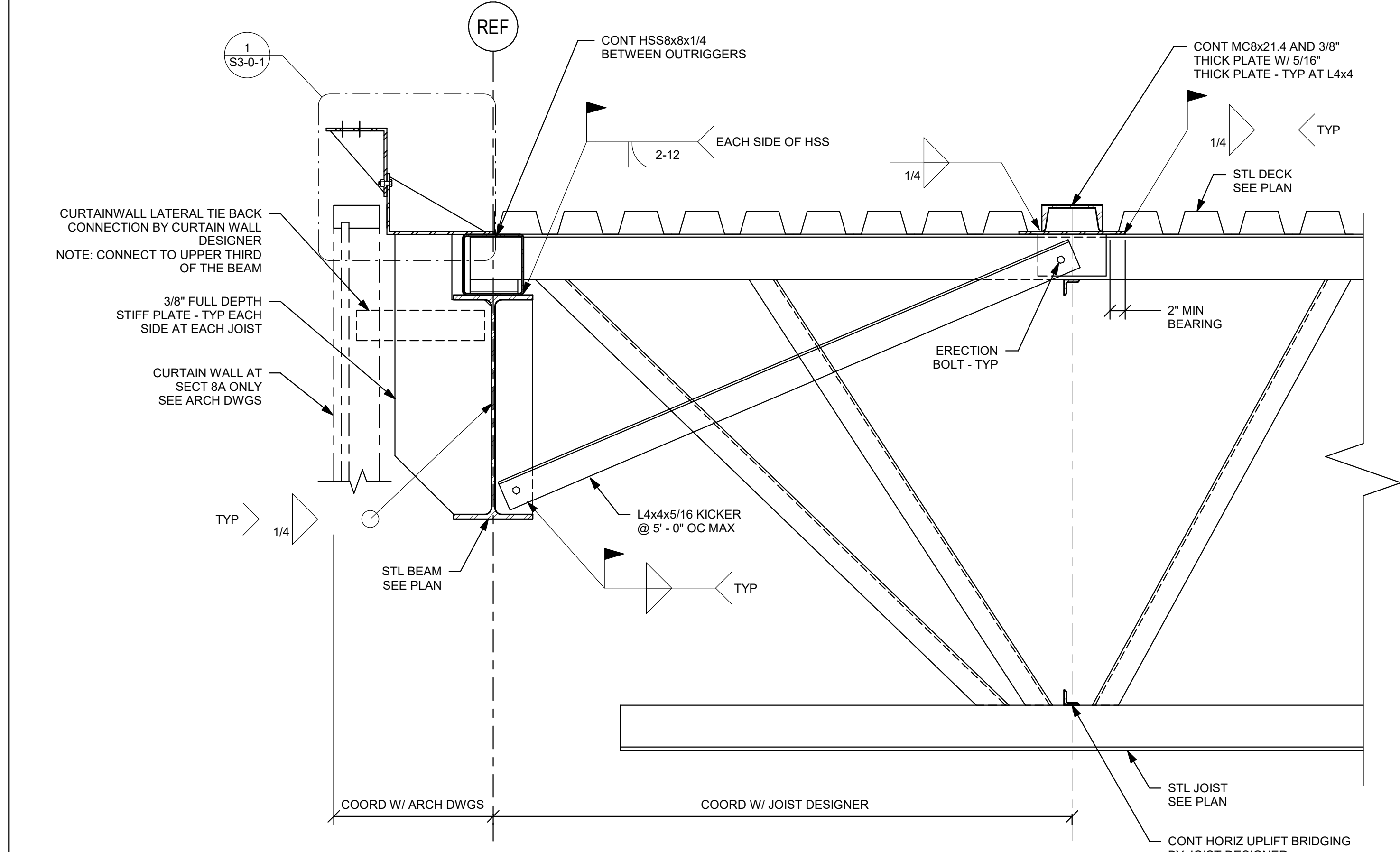
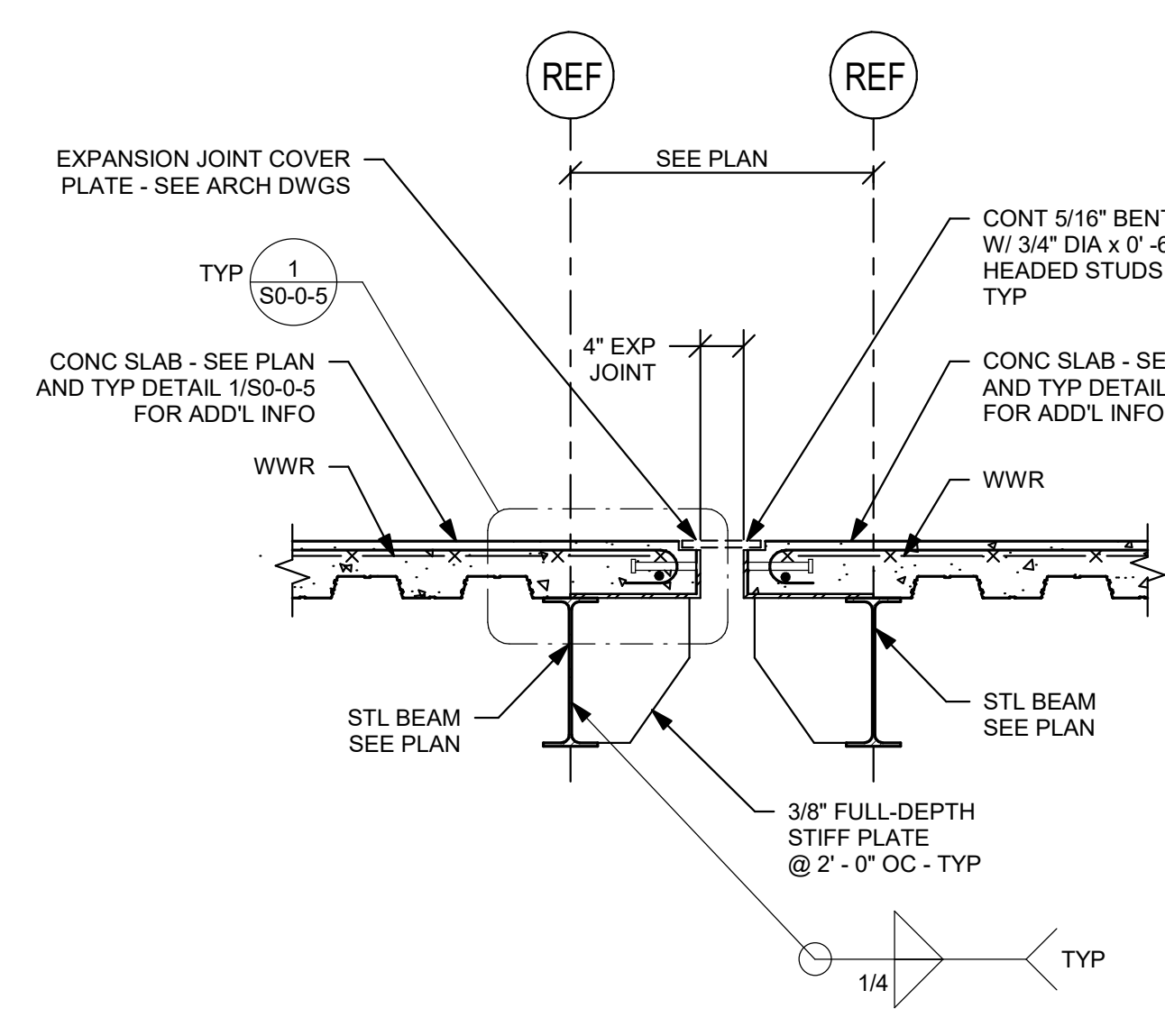
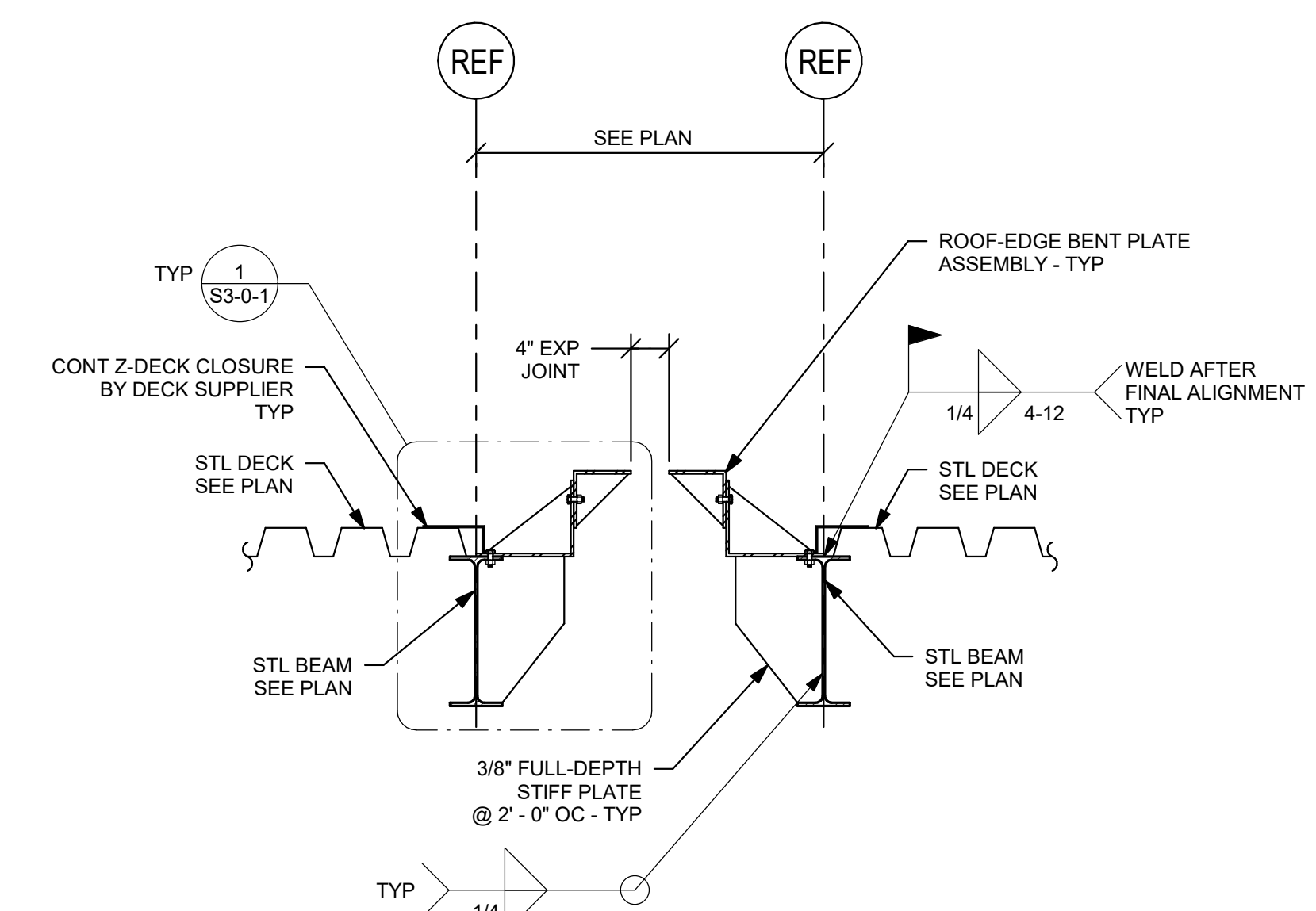
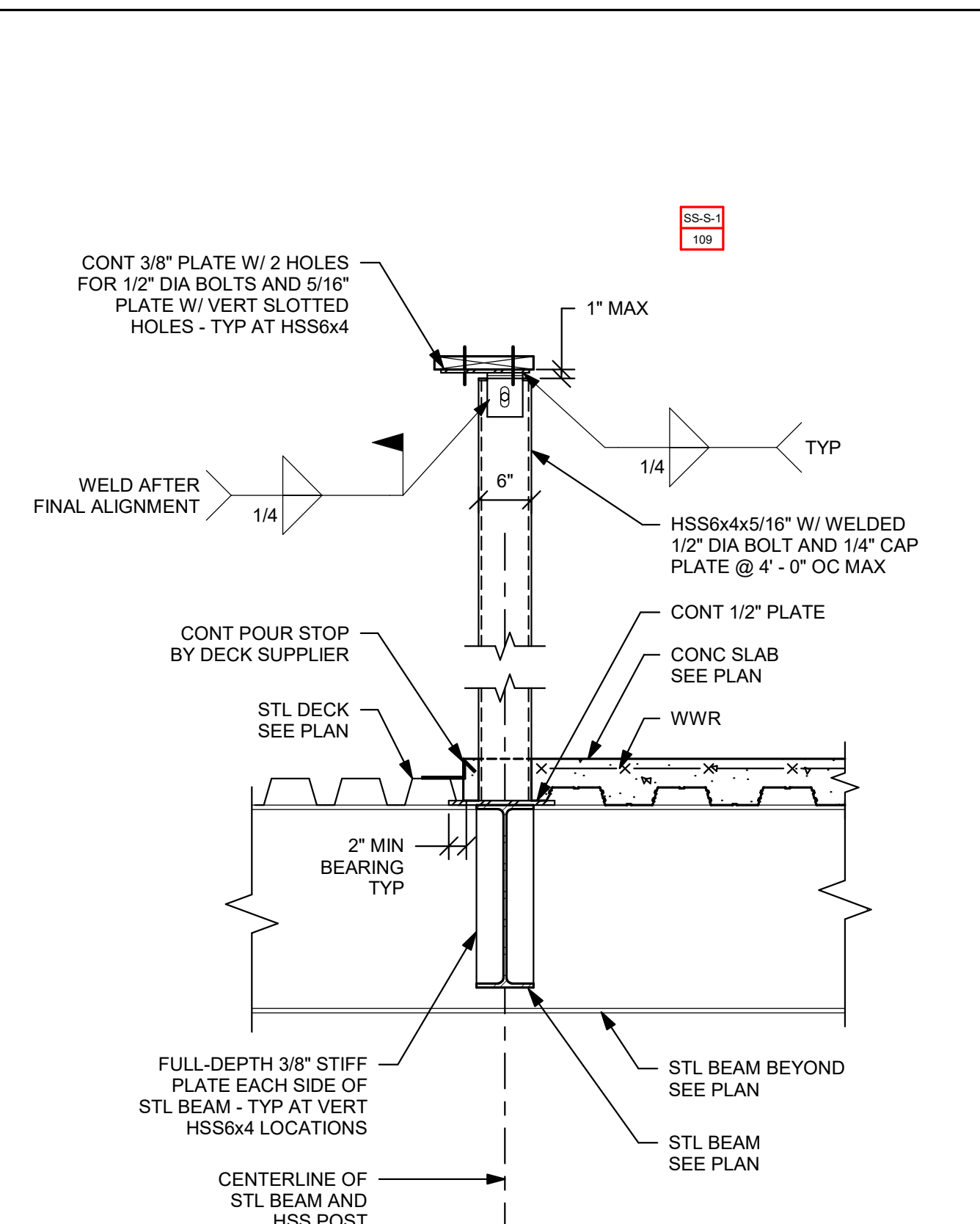
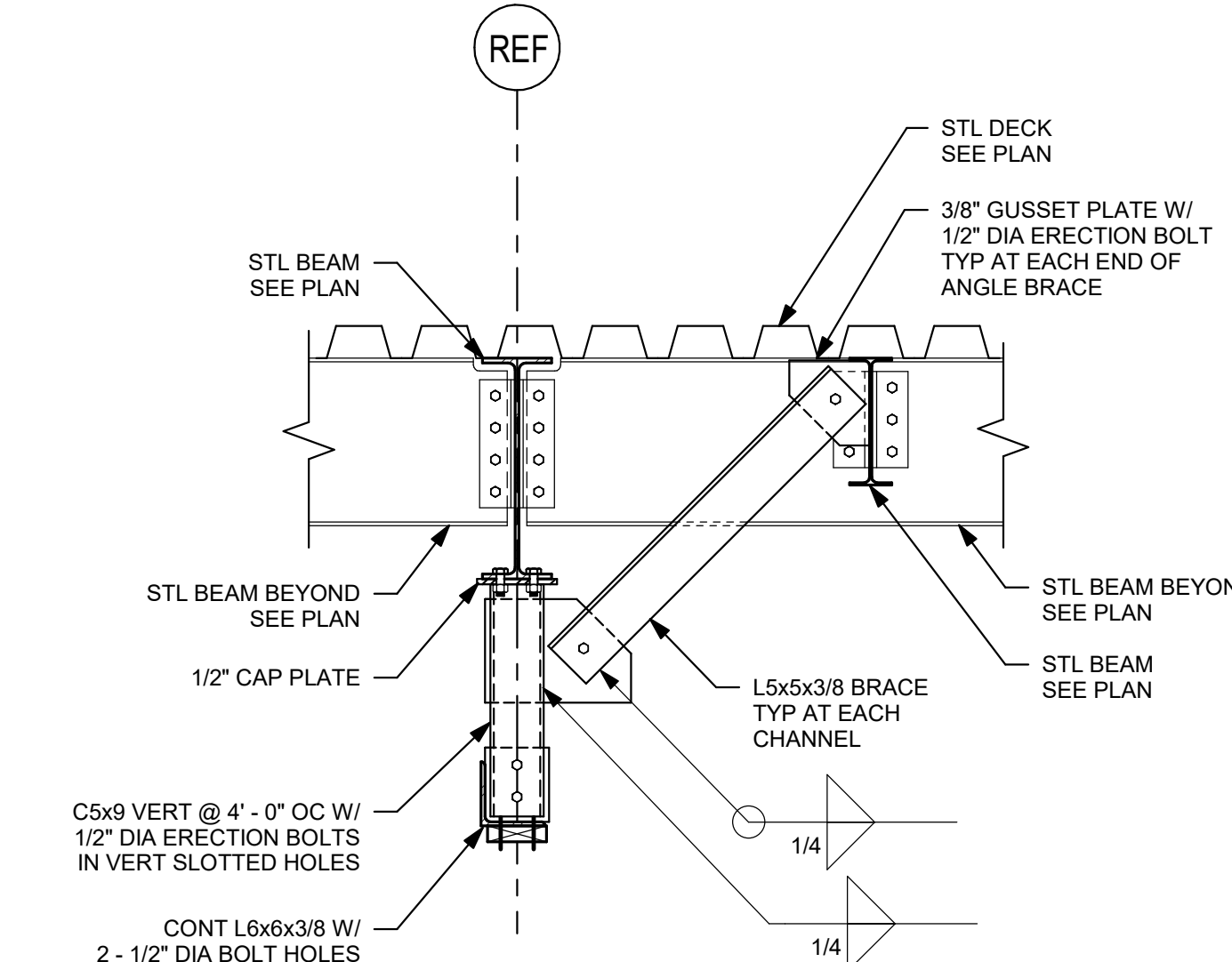
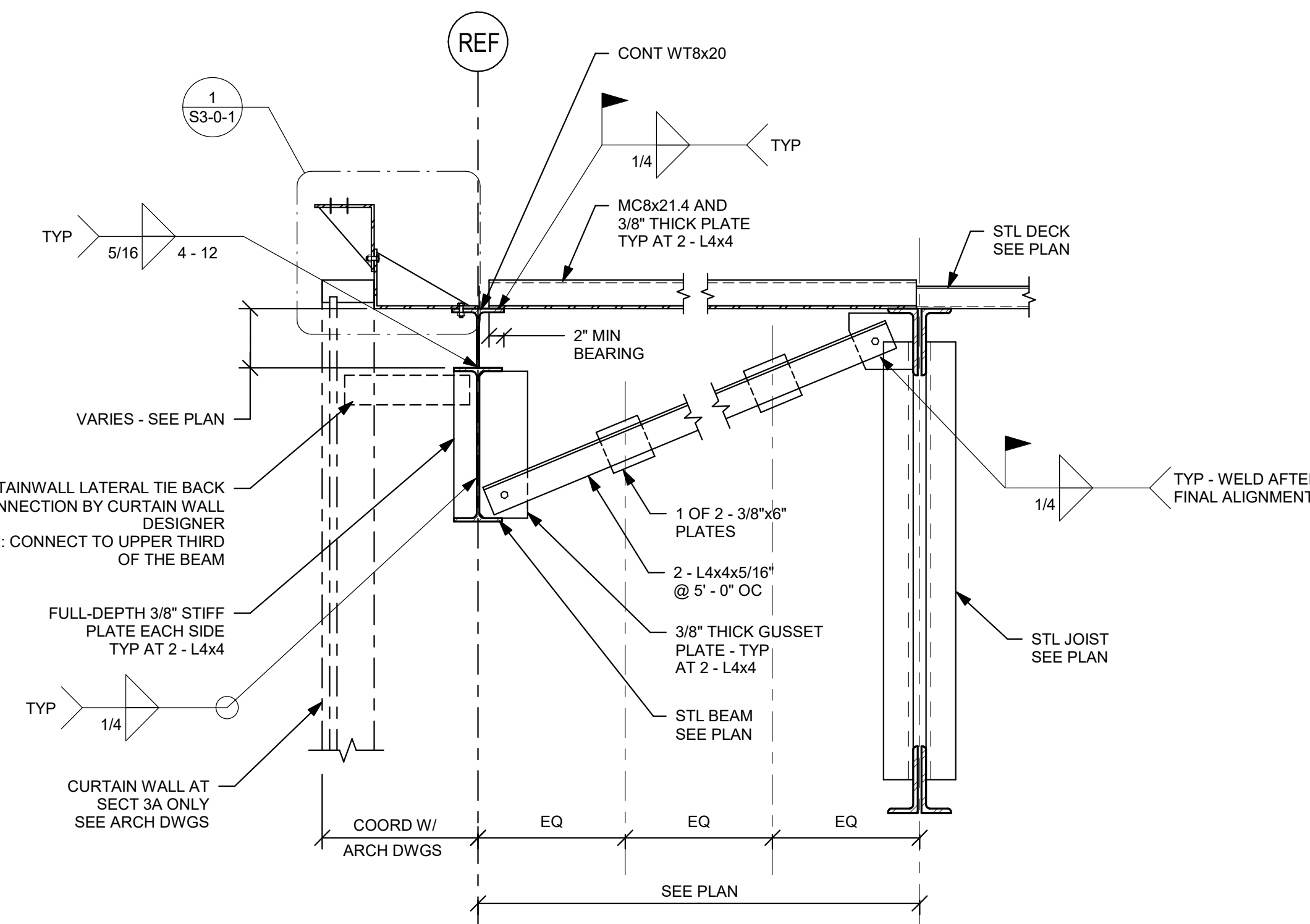


NOTES:

- COORDINATE ALL ROOF-EDGE ANGLE AND BENT PLATE SIZES AND EXTENTS WITH ARCHITECTURAL DETAILS.
- PROVIDE 3/8" STIFFENER PLATES @ 4'-0" OC AS SHOWN WHERE ROOF-EDGE BENT PLATE DIMENSIONS 'A' AND/OR 'B' ARE EQUAL TO OR GREATER THAN 9".
- PROVIDE 3/8" STIFFENER PLATES @ 4'-0" OC AS SHOWN WHERE ROOF-EDGE BENT PLATE DIMENSION 'C' IS EQUAL TO OR GREATER THAN 1'-0".
- EXTEND ROOF EDGE ASSEMBLIES AROUND CORNERS AS REQUIRED PER DETAIL 10 ON DRAWING S0-0-7.
- PROVIDE 2 HOLES FOR 1/2" DIAMETER BOLTS @ 32" OC FOR WOOD BLOCKING ALONG UPPER BENT PLATE.
- PROVIDE 1/2" DIAMETER ERECTION BOLTS @ 4'-0" OC TYPICAL AT EACH BENT PLATE.



NOTE:
SEE DETAIL 1 ON S0-0-5 FOR
ADDITIONAL INFORMATION



$$3/4'' = 1' - 0''$$
$$3/4^{\circ} = 1^{\circ} - 0^{\circ}$$
$$3(4^x) = 1^x - 0$$
$$3/4^{\circ} = 1' - 0''$$
$$3/4^{\circ} = 1^{\circ} - 0^{\circ}$$
$$3(4^*) = 1^* - 0$$
$$3/4^{\circ} = 1^{\circ} - 0^{\circ}$$
$$3/4^{\circ} = 1^{\circ} - 0^{\circ}$$
$$3/4^* = 1^* - 0$$

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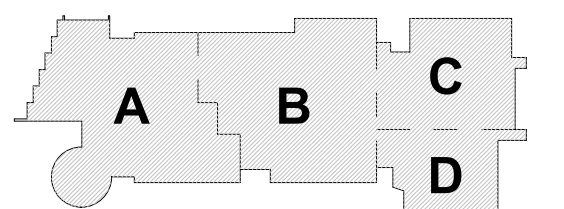
03/31/2023 EARLY STRUCTURAL BID PACKAGE

REVISION LIST
SS-S-1 4/14/2023 STRUCTURAL STEEL
ADDENDUM 1

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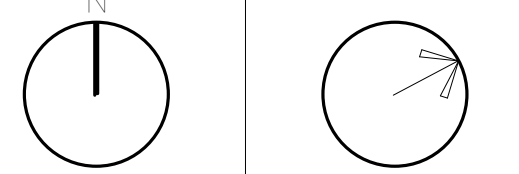
BID SET

August 28th, 2023



KEY PLAN

PROJECT NORTH MAGNETIC NORTH



SECTIONS

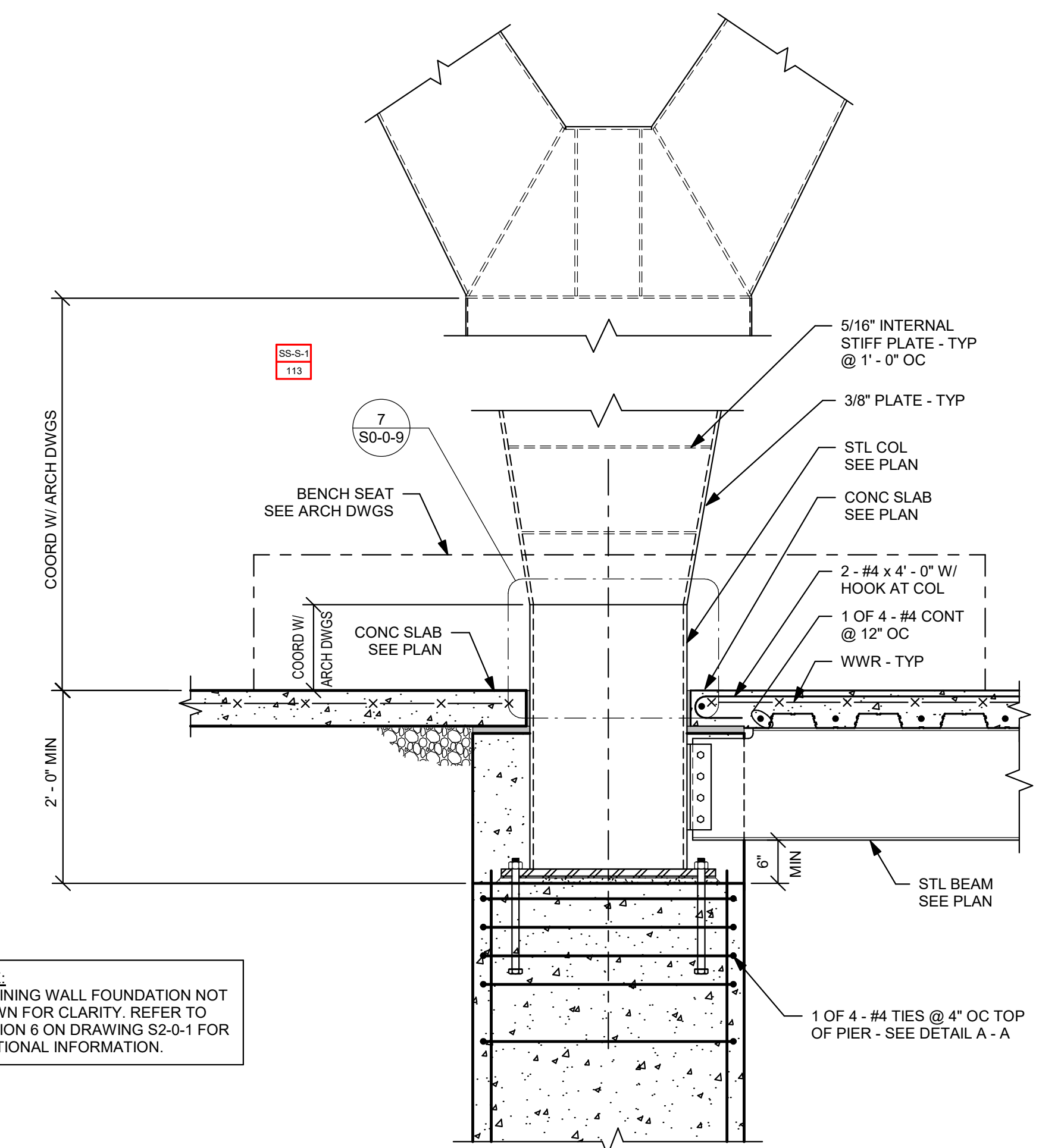
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Job No.: 20202

Drawn By: EDG

Date: August 28th, 2023

S3-0-3

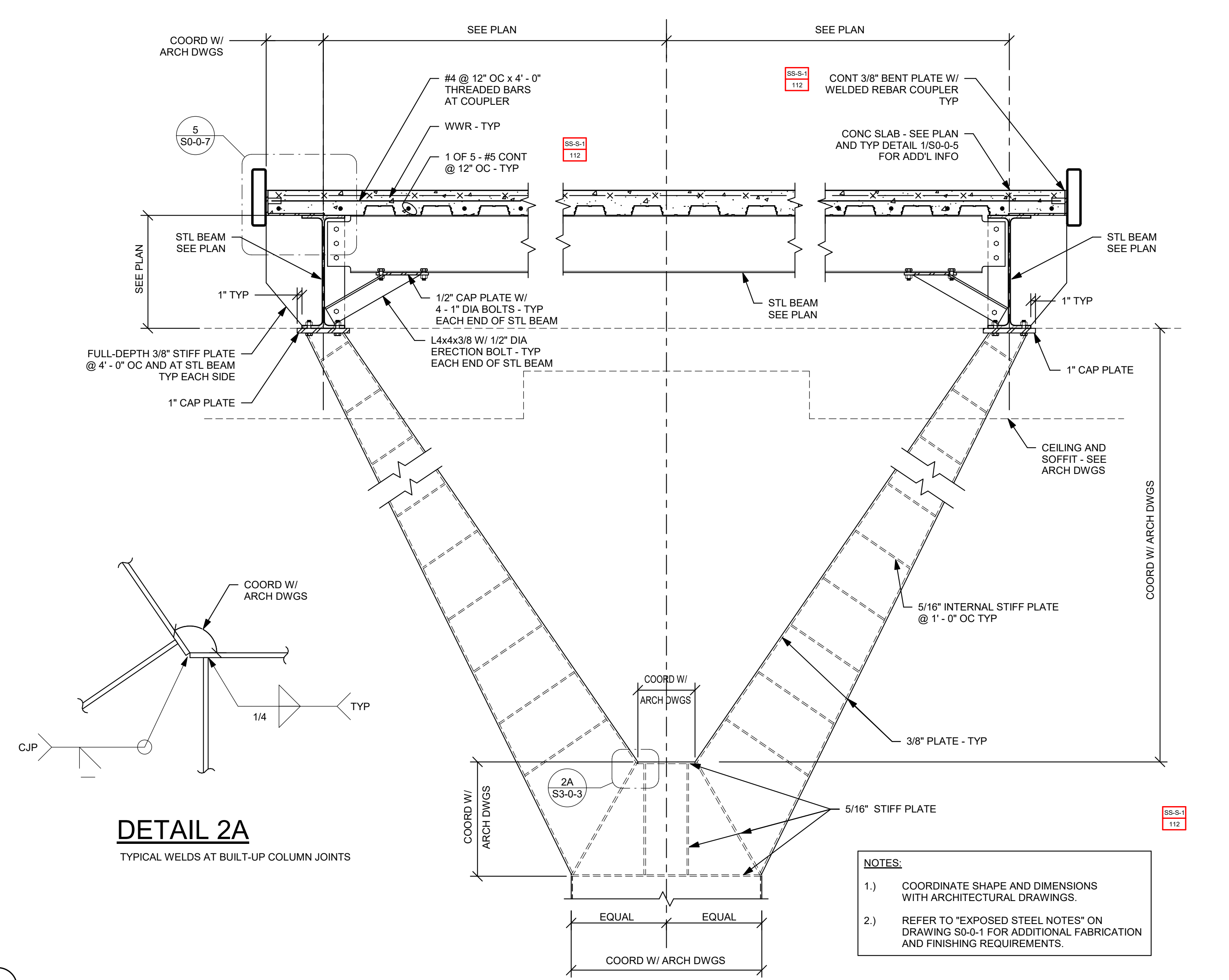


TYPICAL BUILT - UP COLUMN SECTION

NOTES:

- COORDINATE SHAPE AND DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
- REFER TO "EXPOSED STEEL NOTES" ON DRAWING S3-0-1 FOR ADDITIONAL FABRICATION AND FINISHING REQUIREMENTS.

NOTE: RETAINING WALL FOUNDATION NOT SHOWN FOR CLARITY. REFER TO SECTION 6 ON DRAWING S2-0-1 FOR ADDITIONAL INFORMATION.

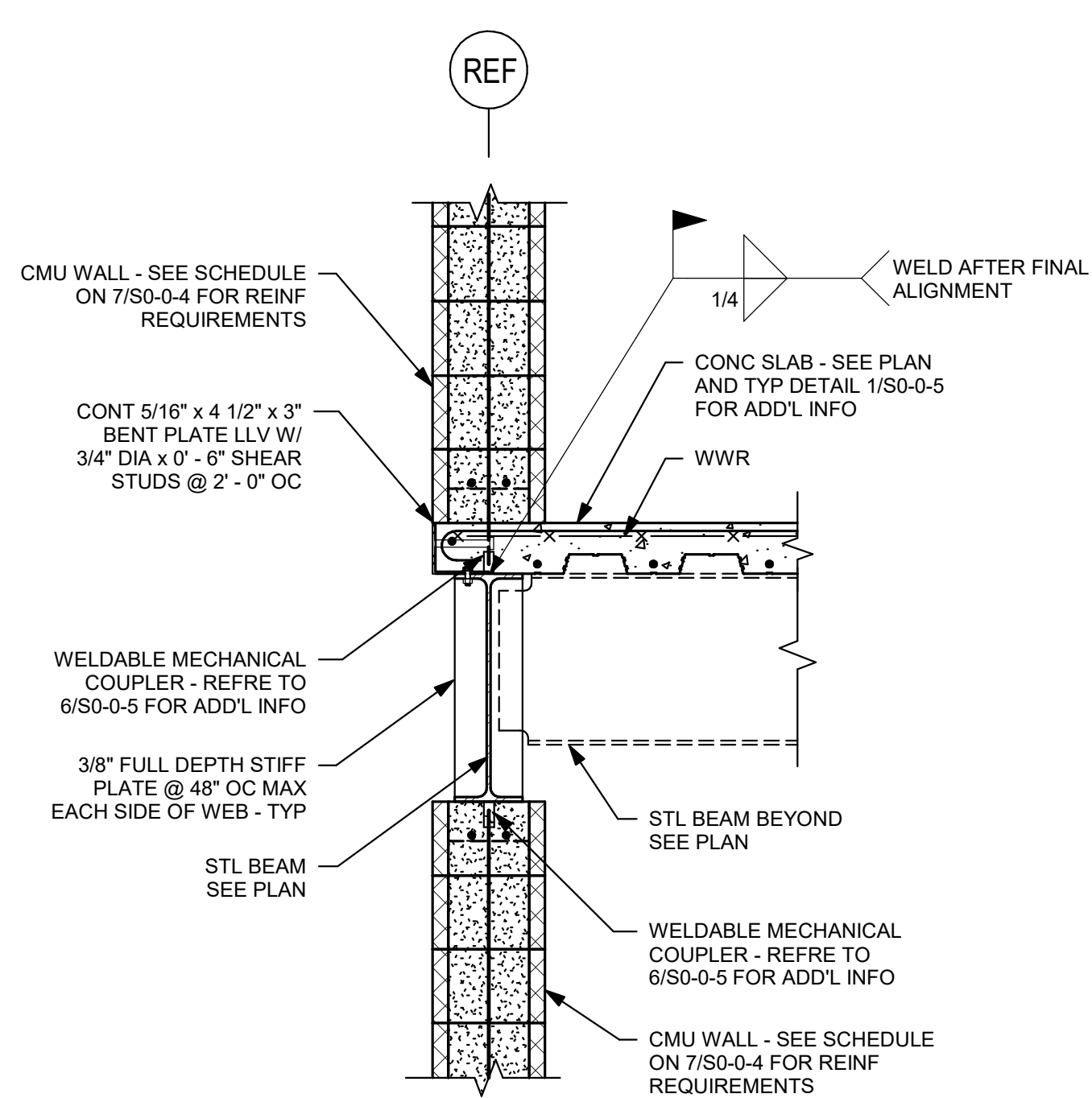


DETAIL 2A

TYPICAL WELDS AT BUILT-UP COLUMN JOINTS

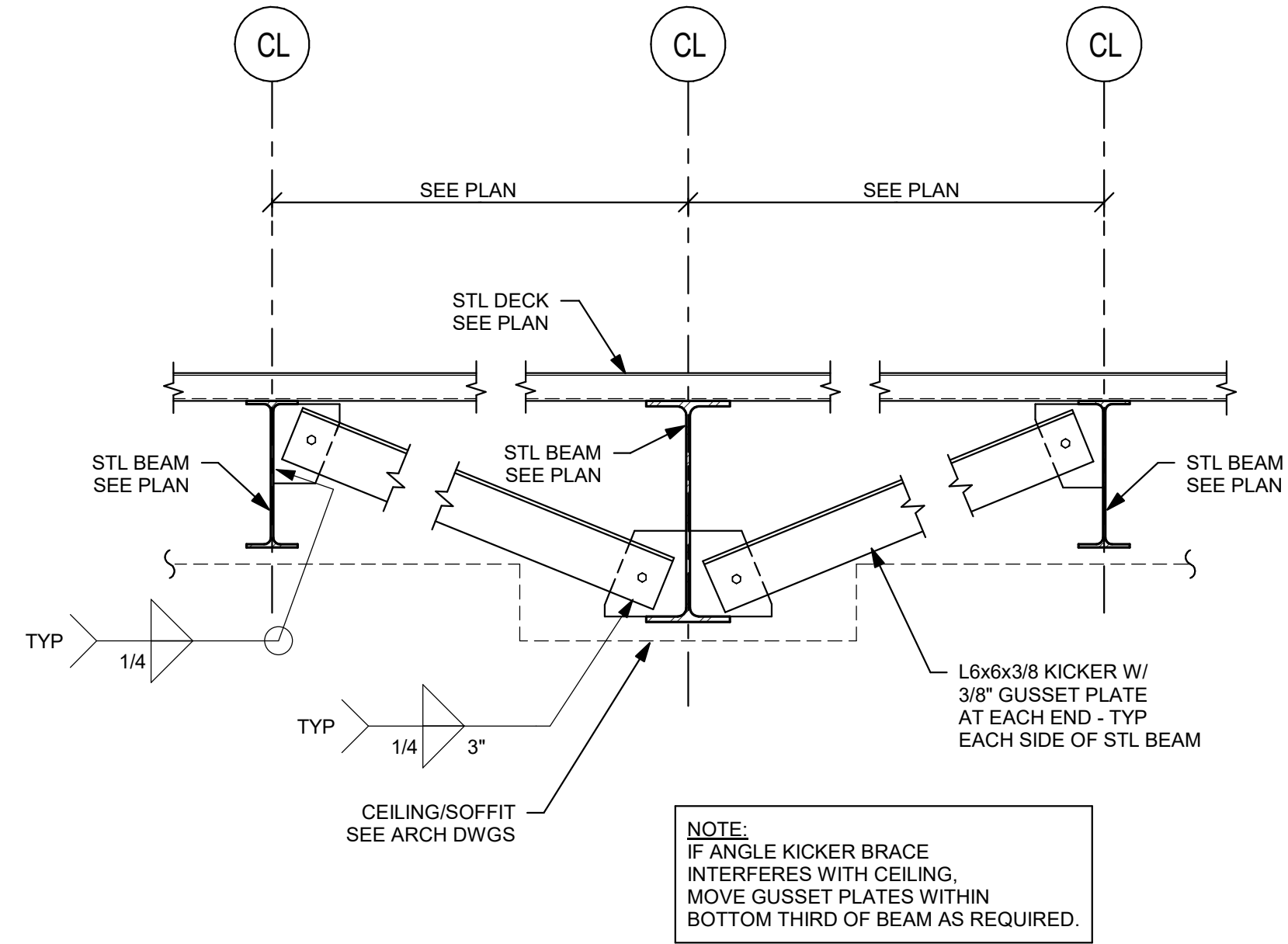
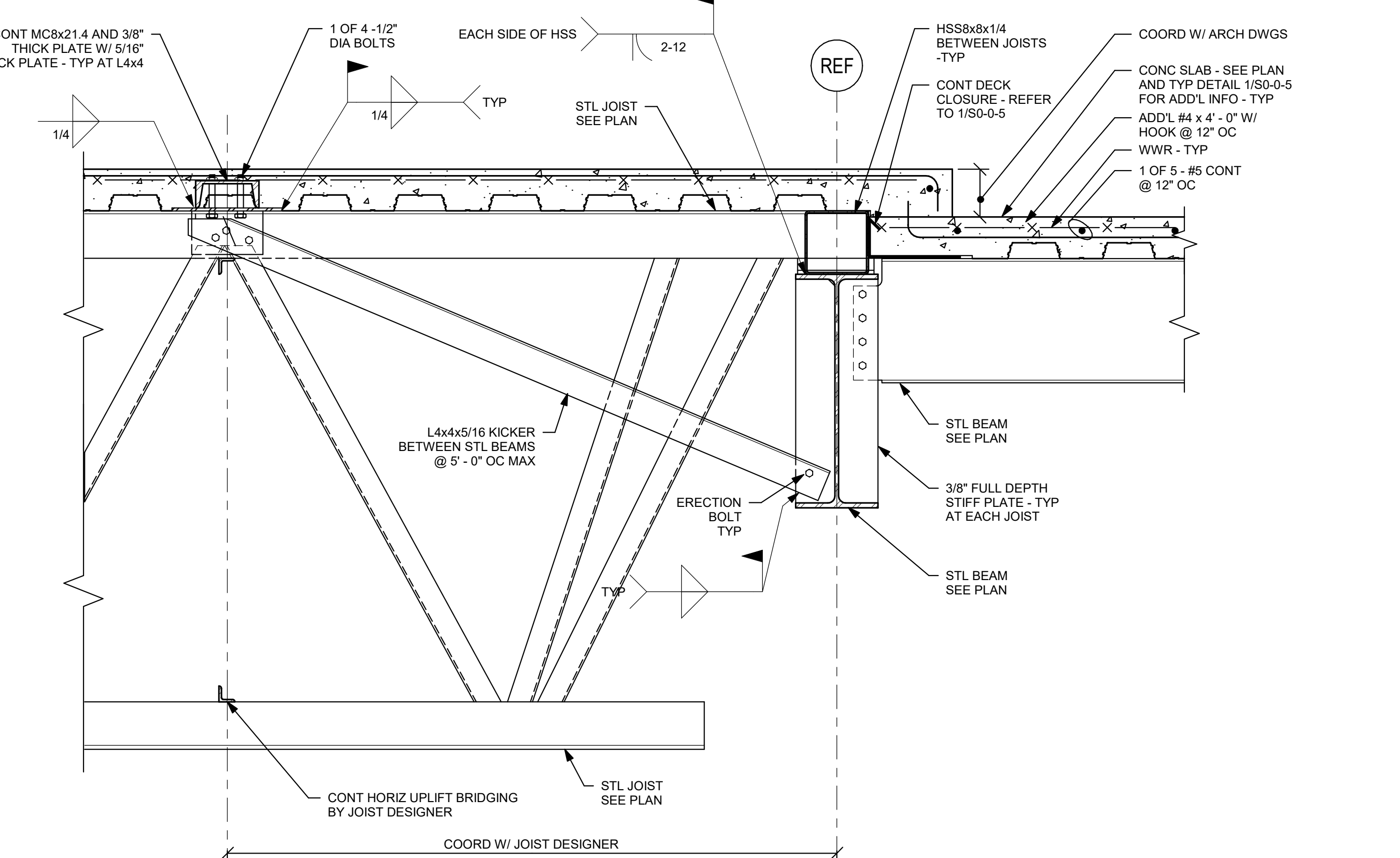
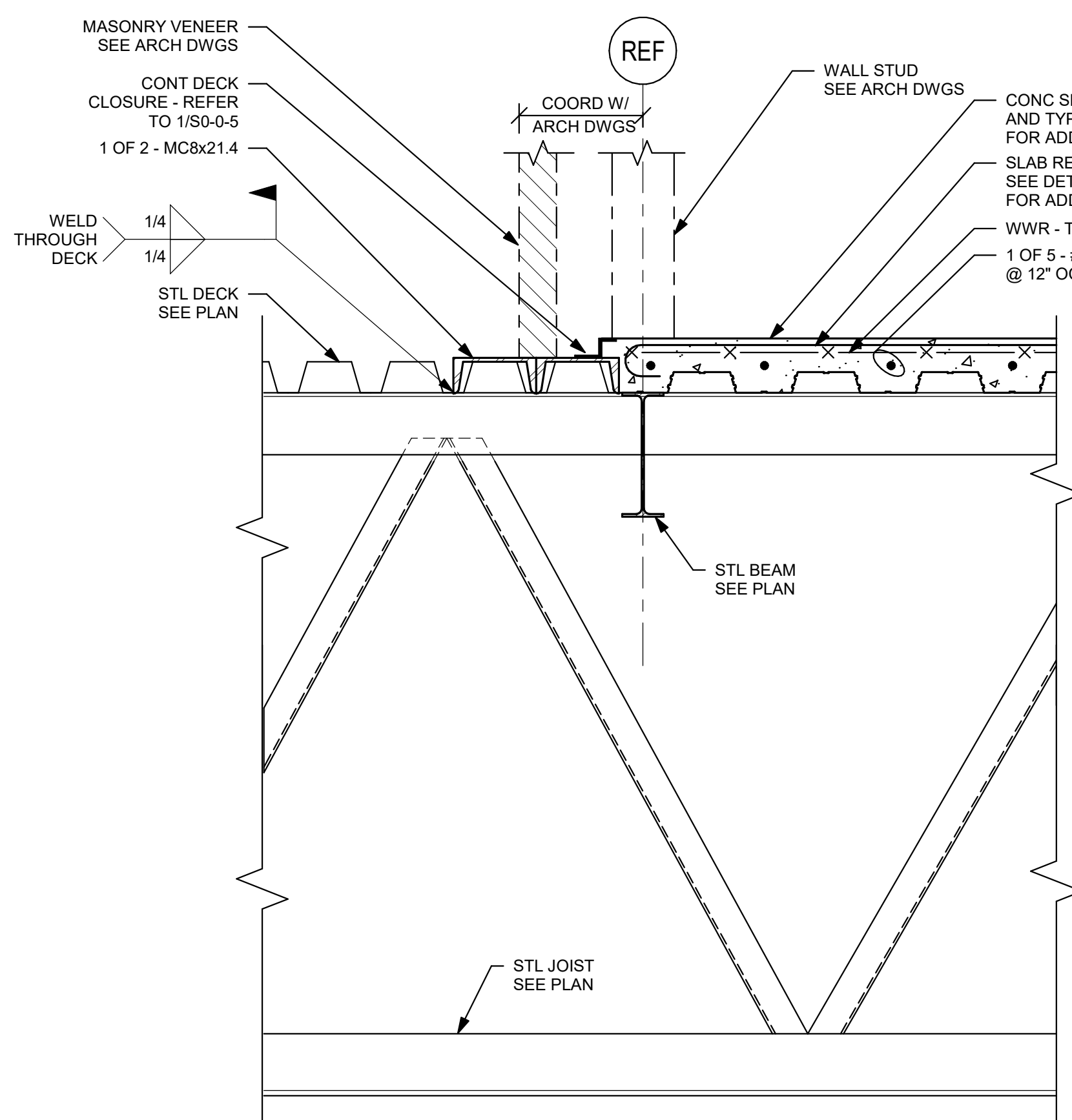
NOTES:

- COORDINATE SHAPE AND DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
- REFER TO "EXPOSED STEEL NOTES" ON DRAWING S3-0-1 FOR ADDITIONAL FABRICATION AND FINISHING REQUIREMENTS.

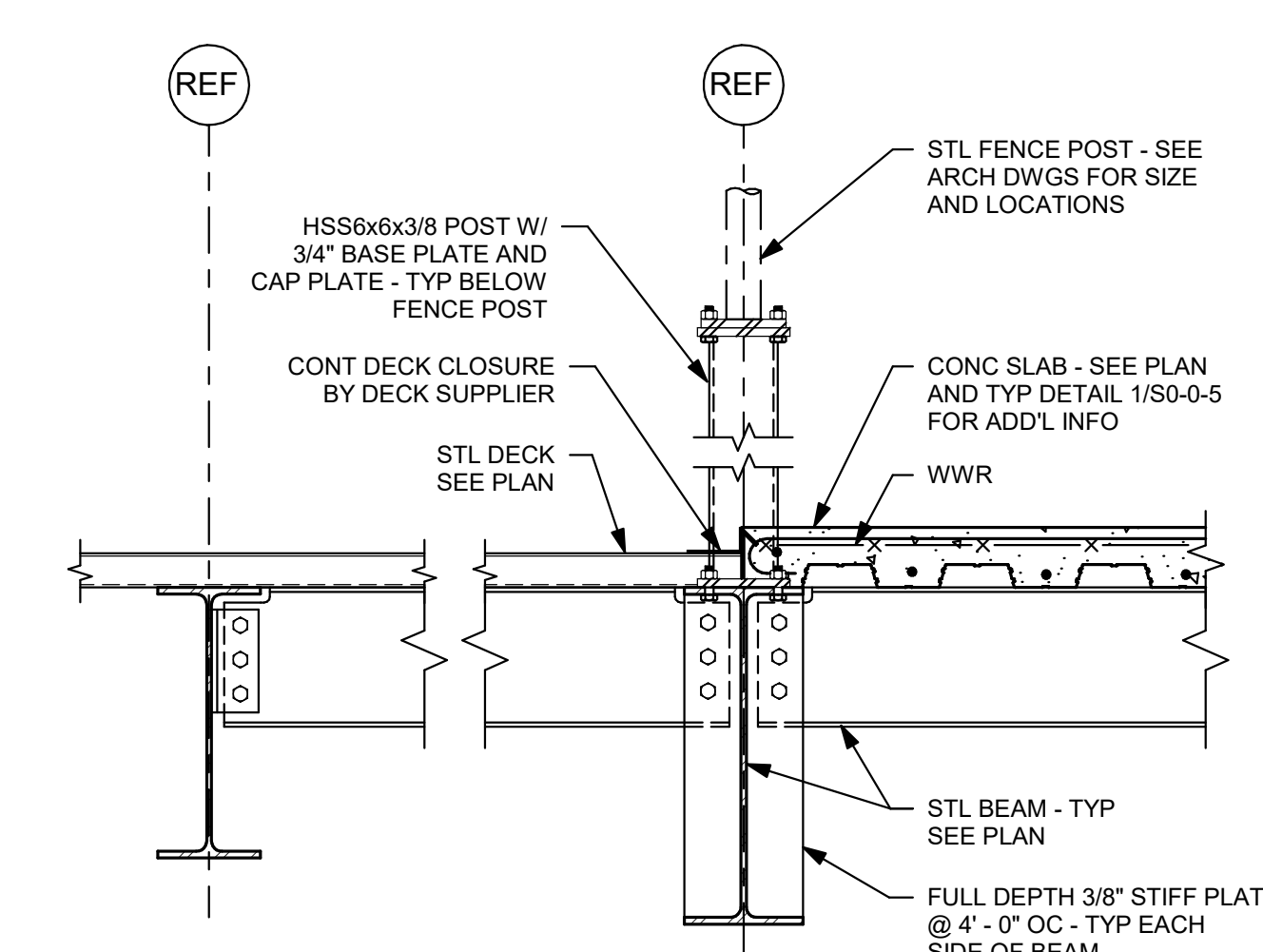
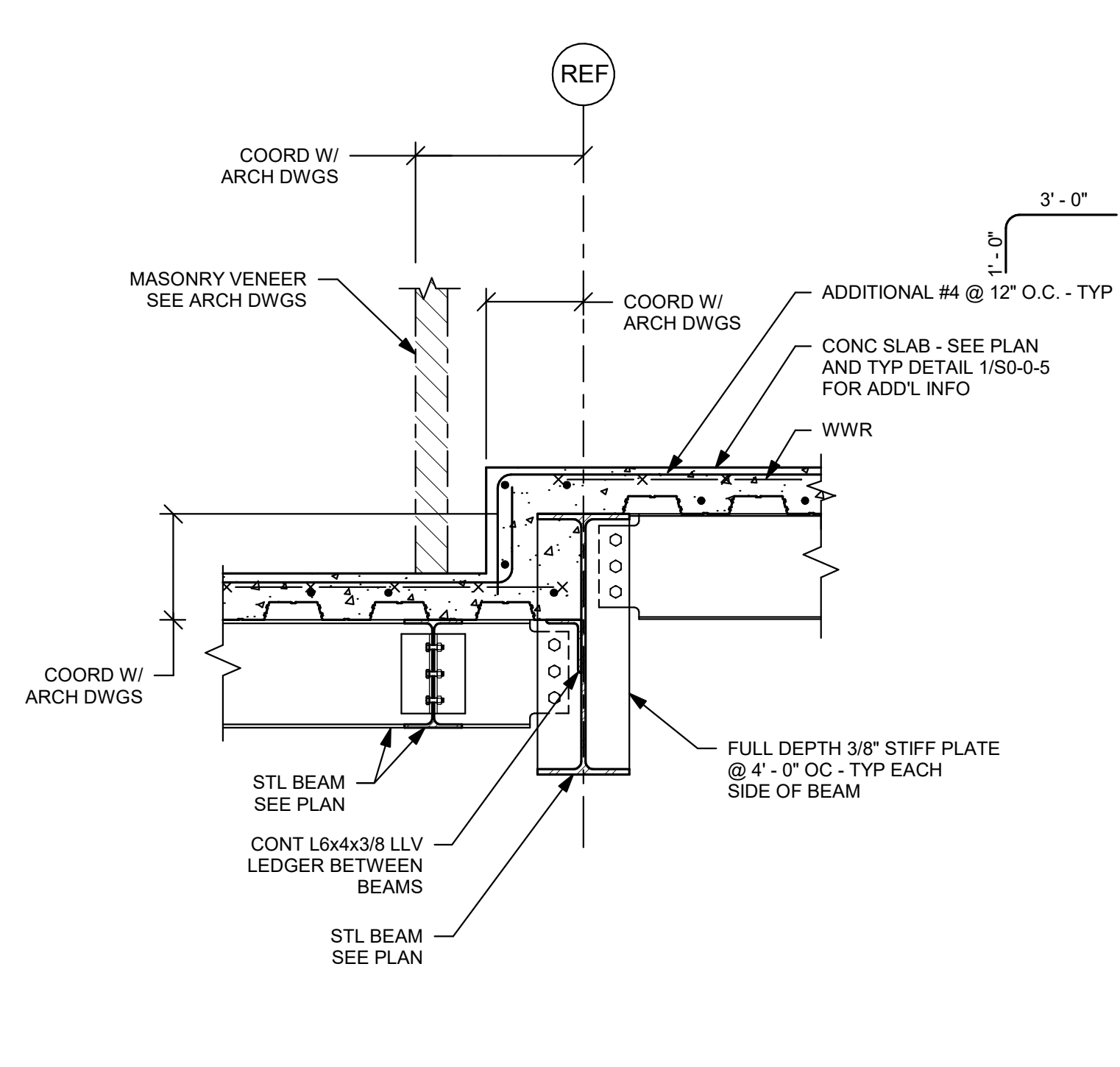
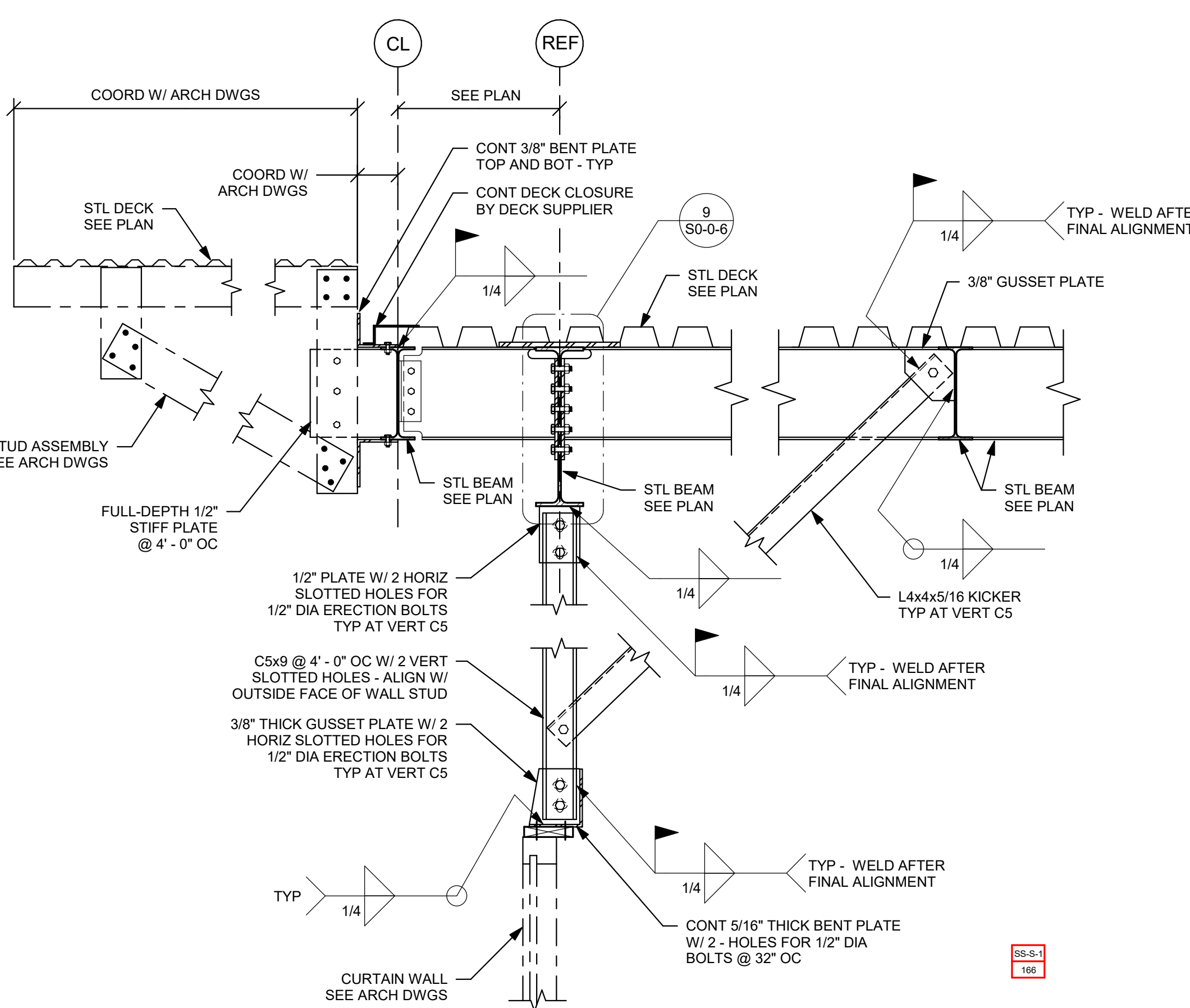


NOTES:

- COORDINATE SLAB-EDGE REINFORCING WITH DETAIL 1 ON DRAWING S3-0-5.
- SEE PLAN FOR NUMBER OF SHEAR STUDS REQUIRED AT STEEL BEAMS.



NOTE: IF ANGLE KICKER BRACE INTERFERES WITH CEILING, MOVE GUSSET PLATES WITHIN BOTTOM THIRD OF BEAM AS REQUIRED.



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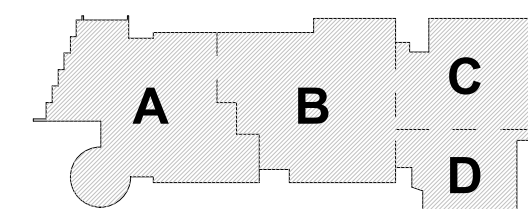
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August 28th, 2023



KEY PLAN

PROJECT NORTH
MAGNETIC NORTH

SECTIONS

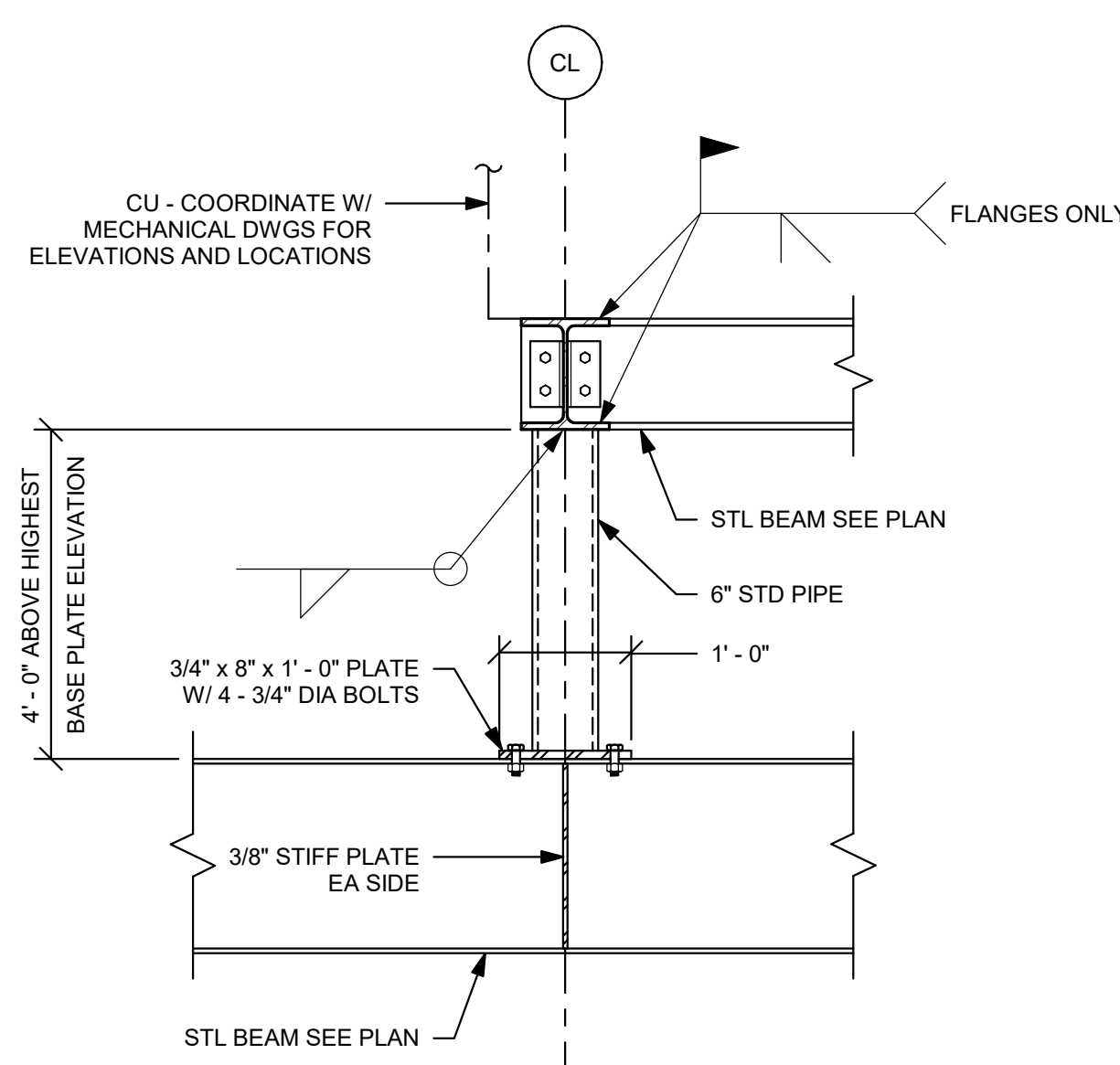
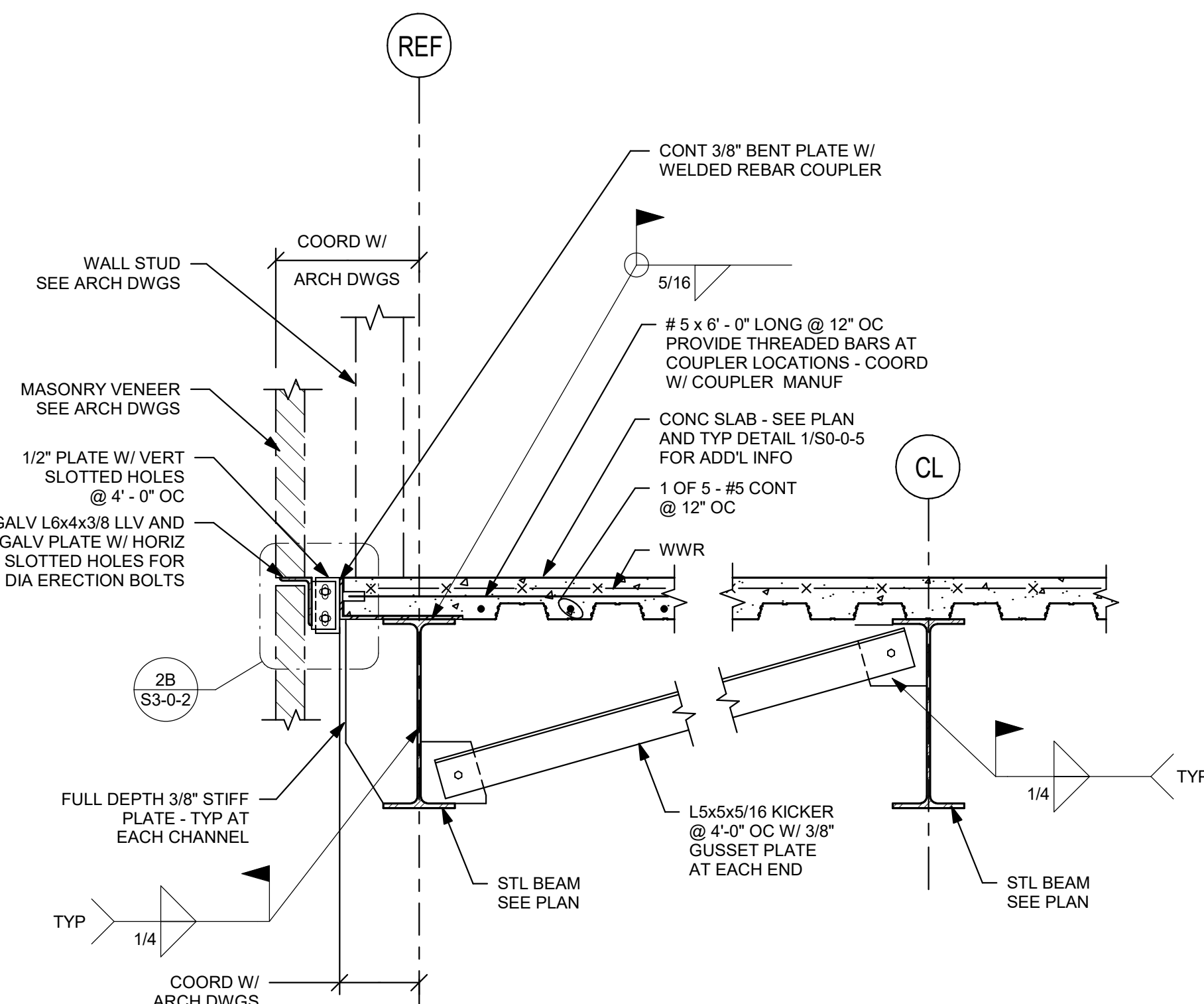
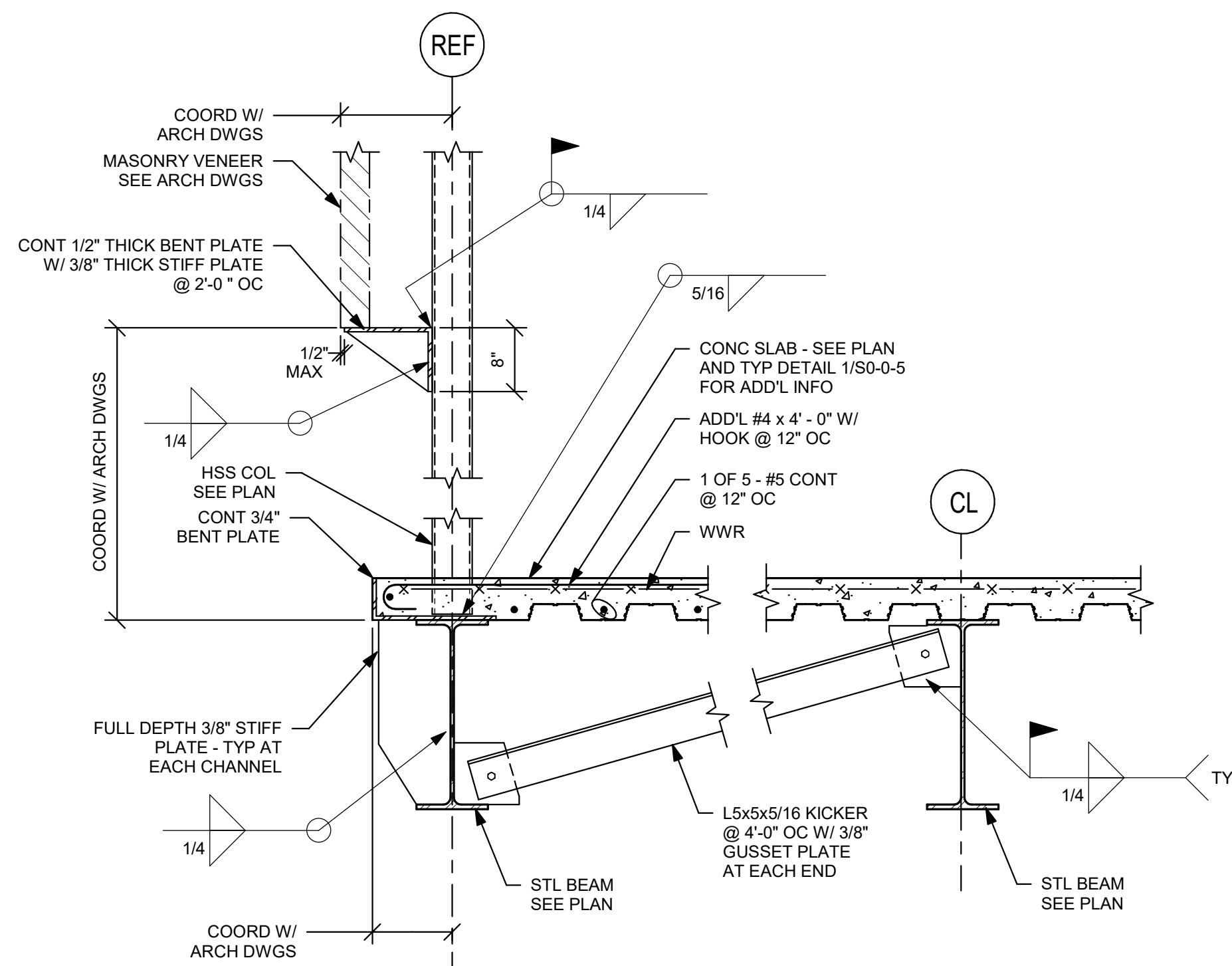
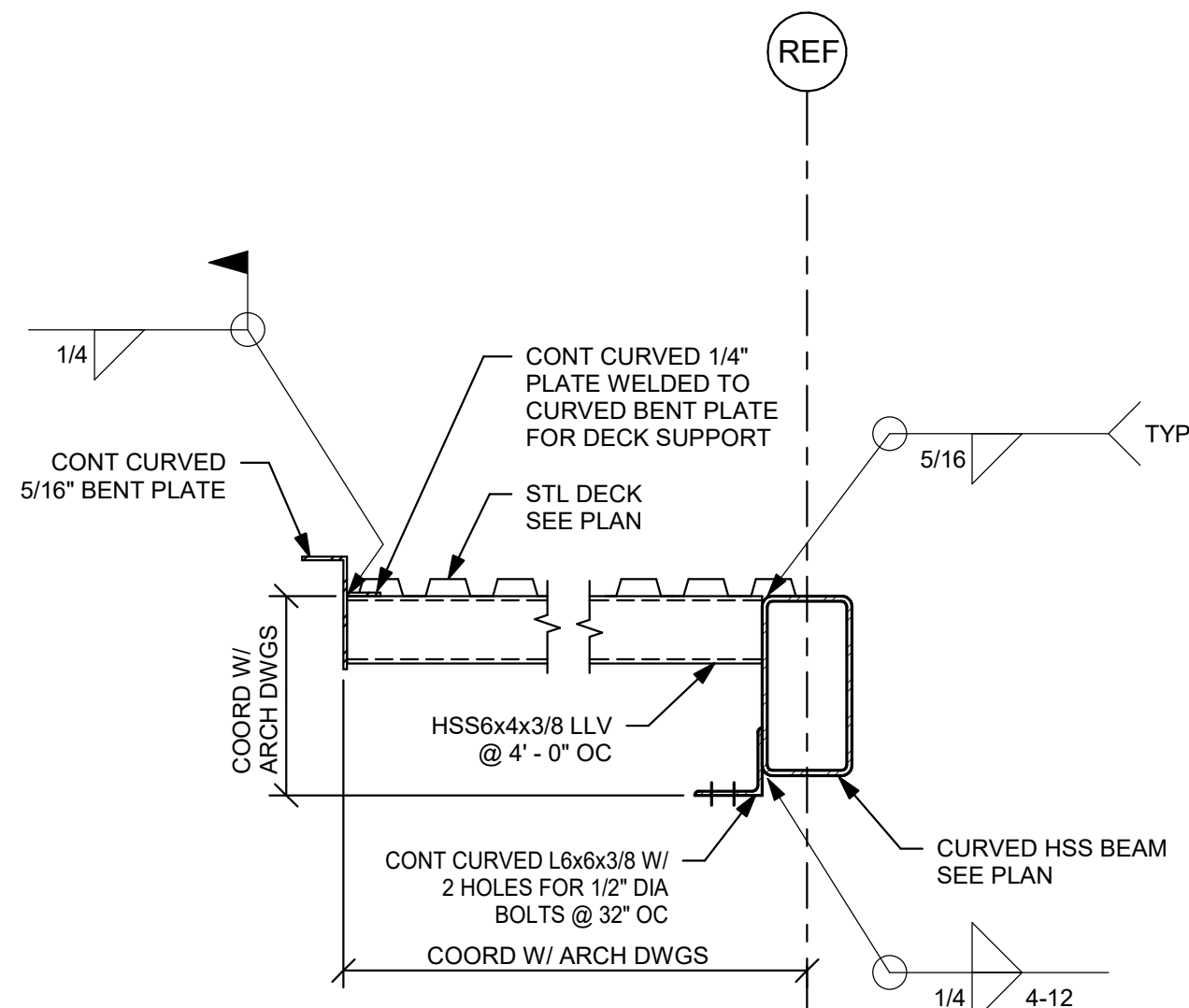
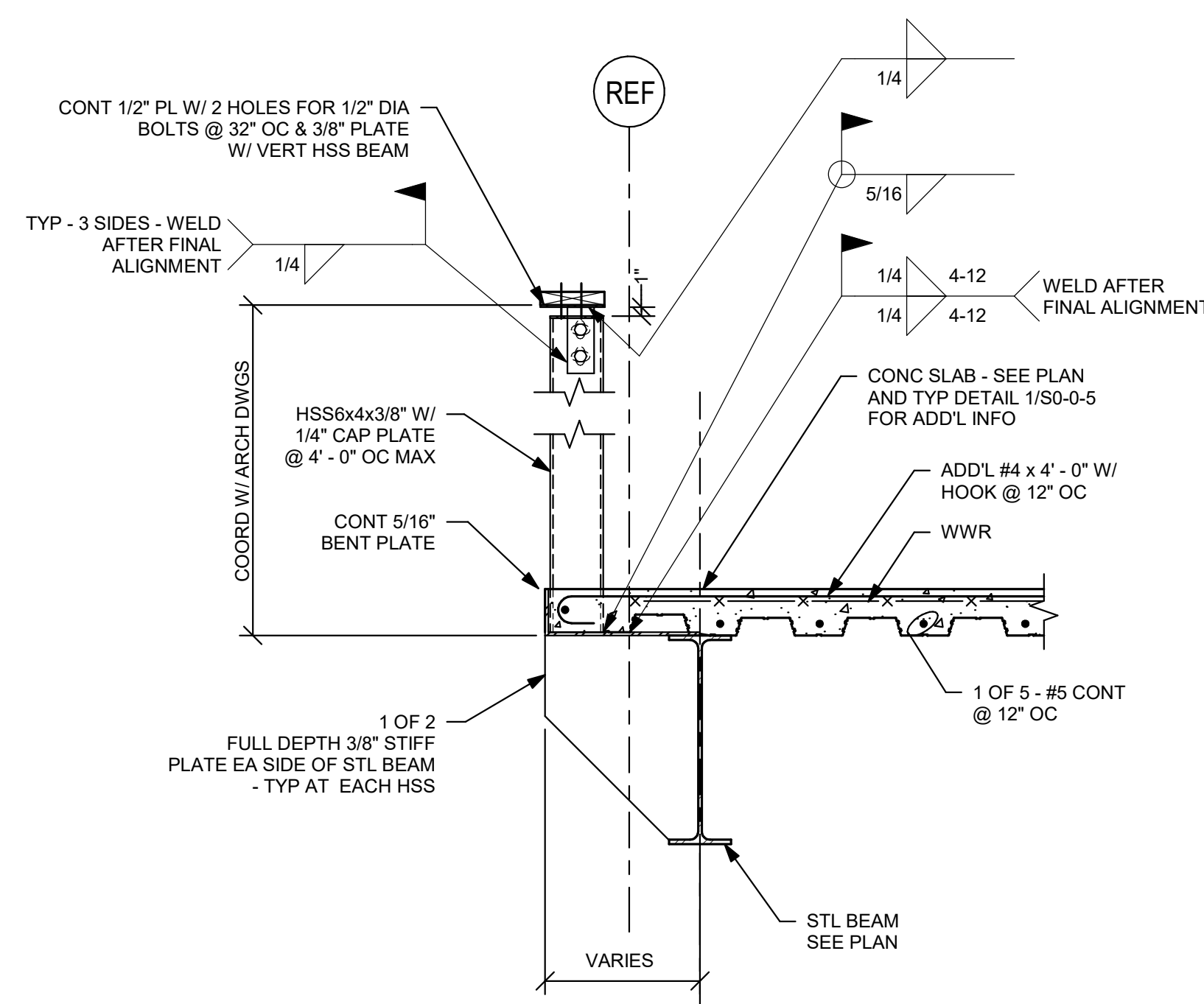
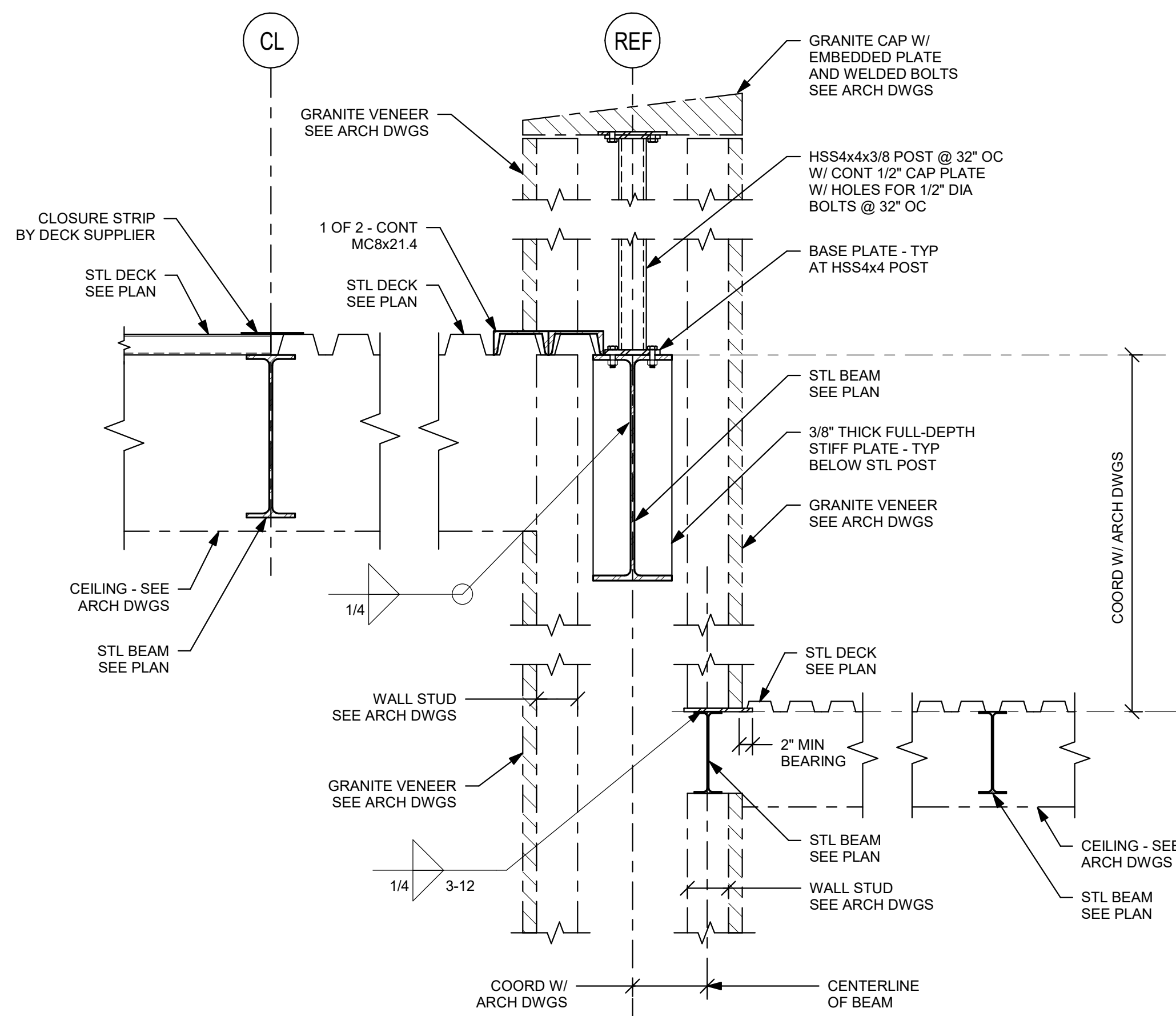
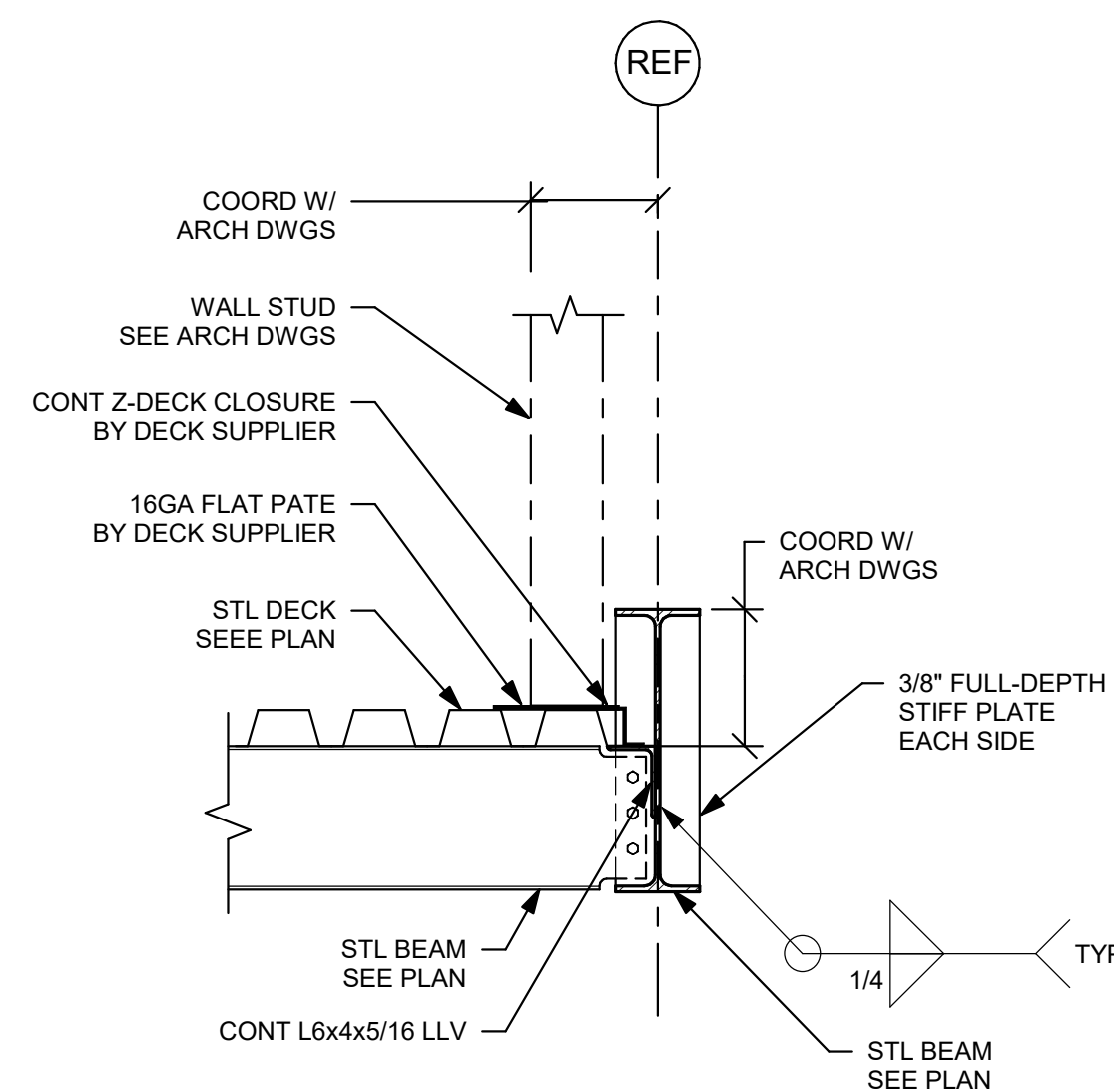
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Job No.: 20202

Drawn By: EDG

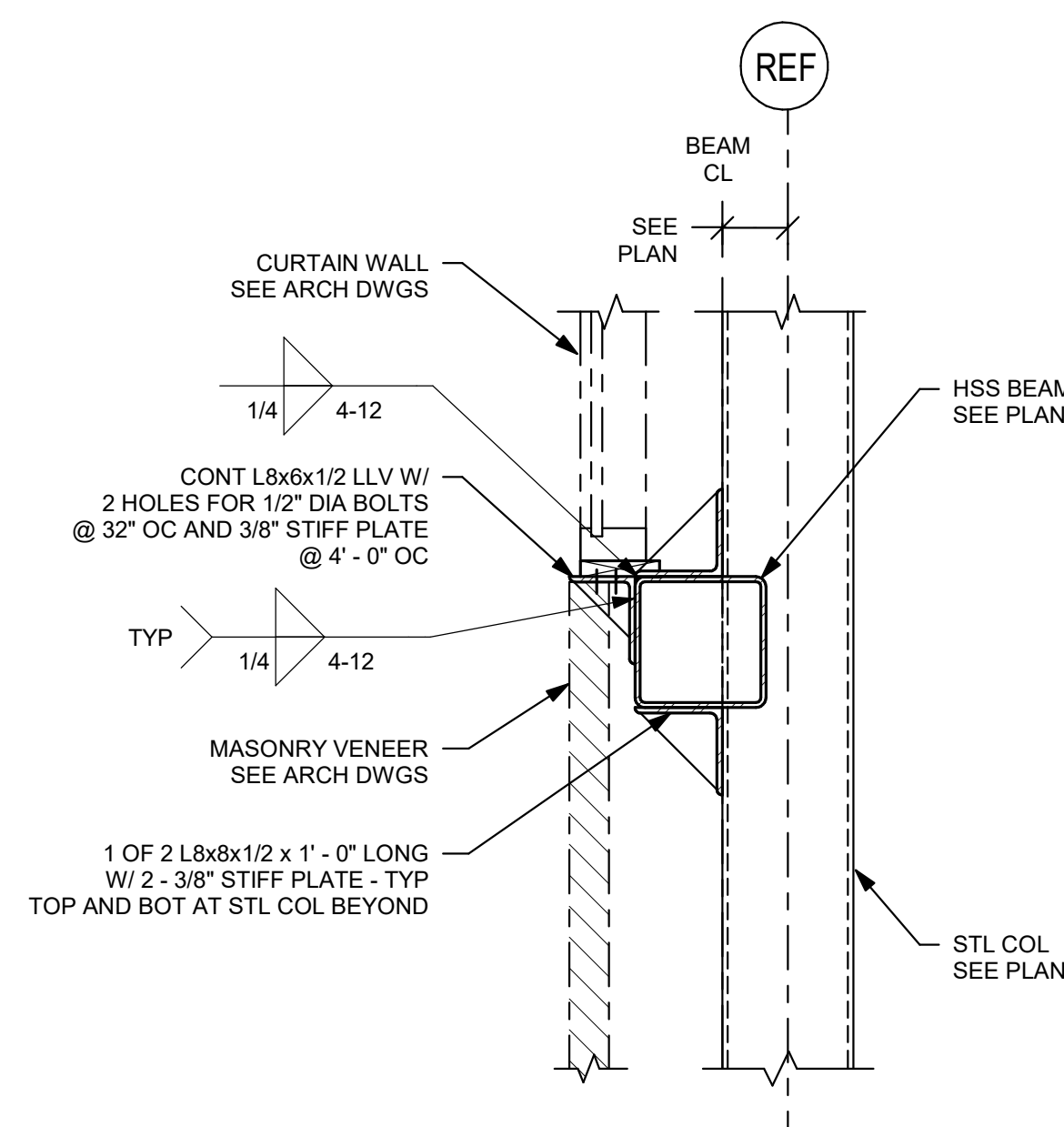
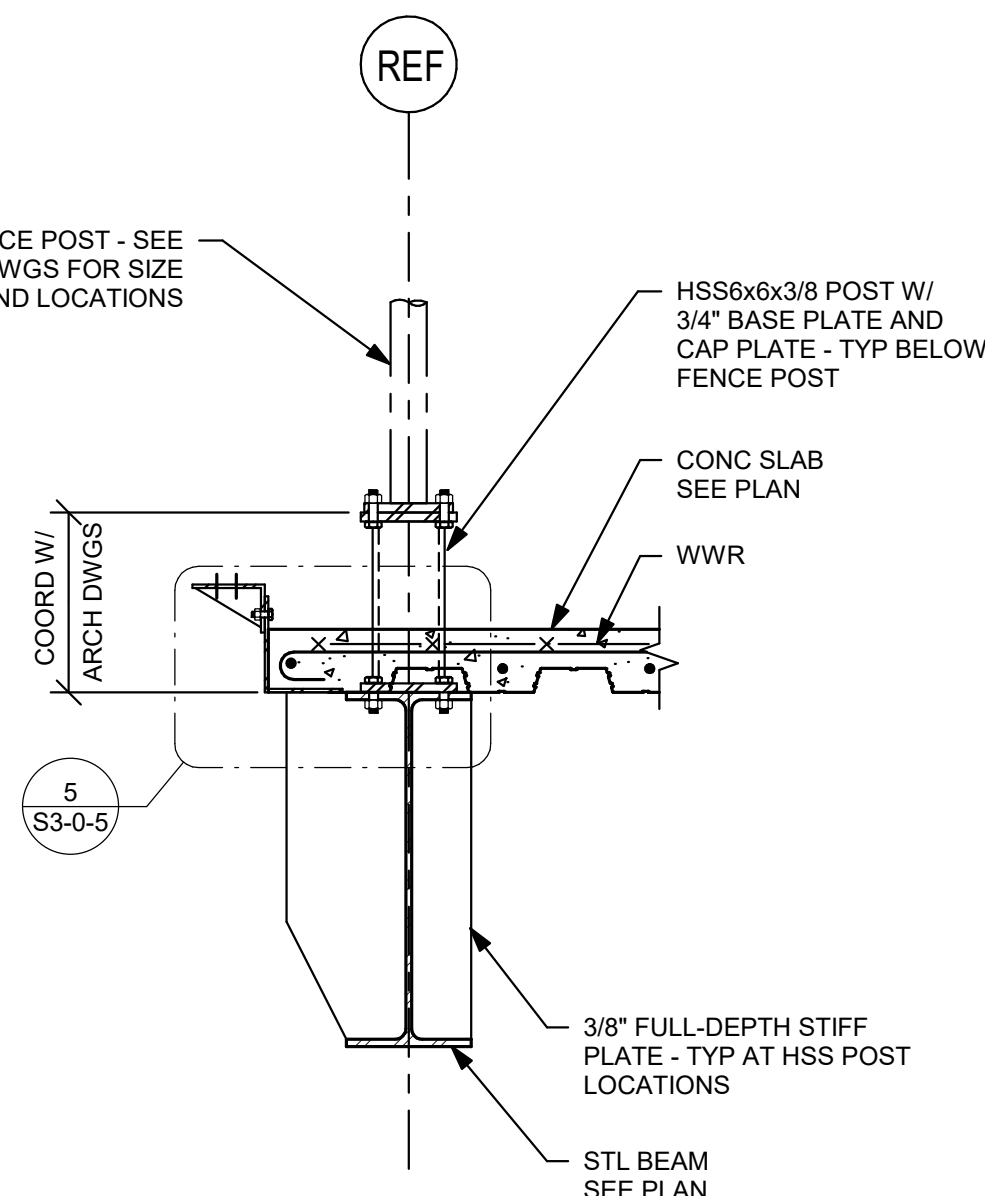
Date: August 28th, 2023

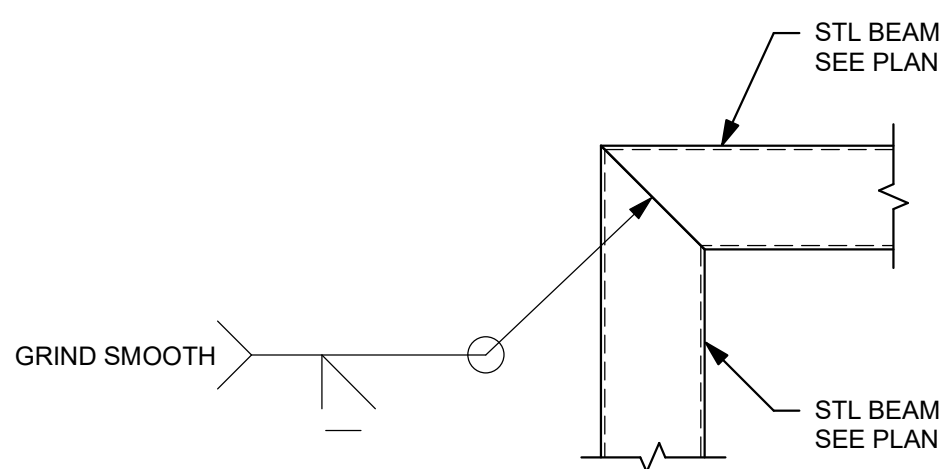
S3-0-4



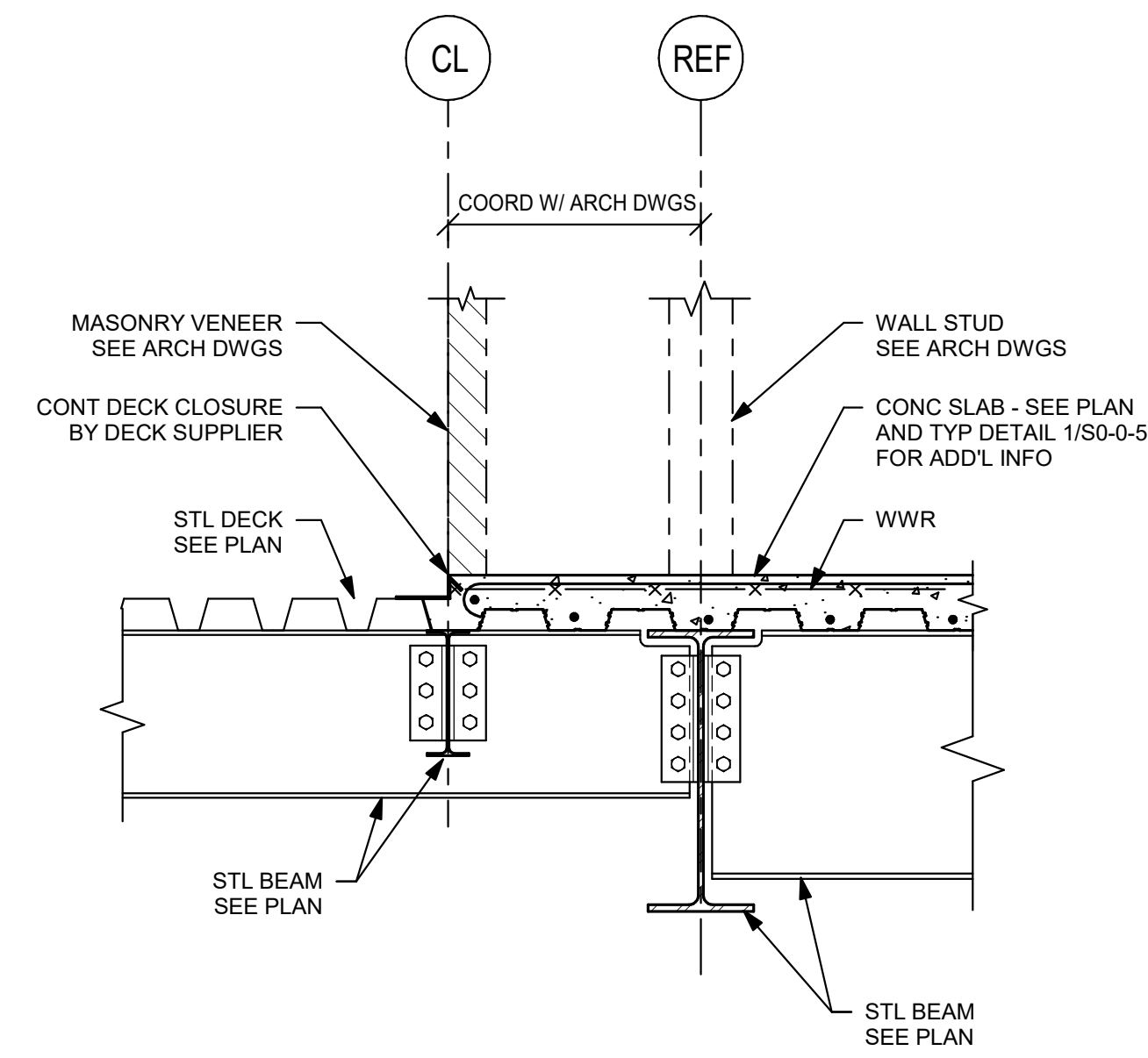
TYPICAL SUPPORT OF ROOF TOP MECHANICAL UNIT DETAIL

- NOTES:
- 1.) ALL EXPOSED STEEL TO BE HOT-DIP GALVANIZED.
 - 2.) TOUCH UP ALL FIELD WELDS WITH ZINC RICH PAINT

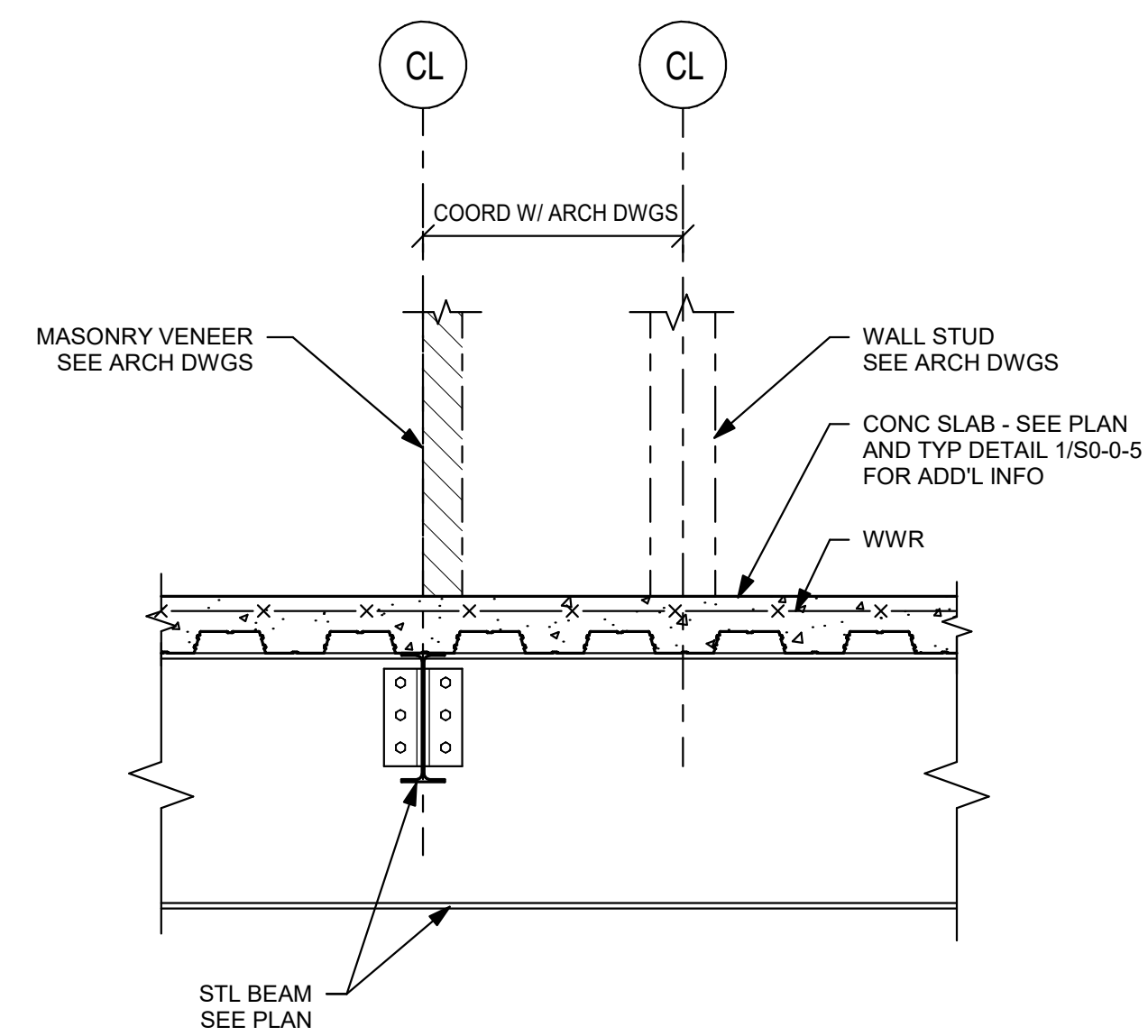




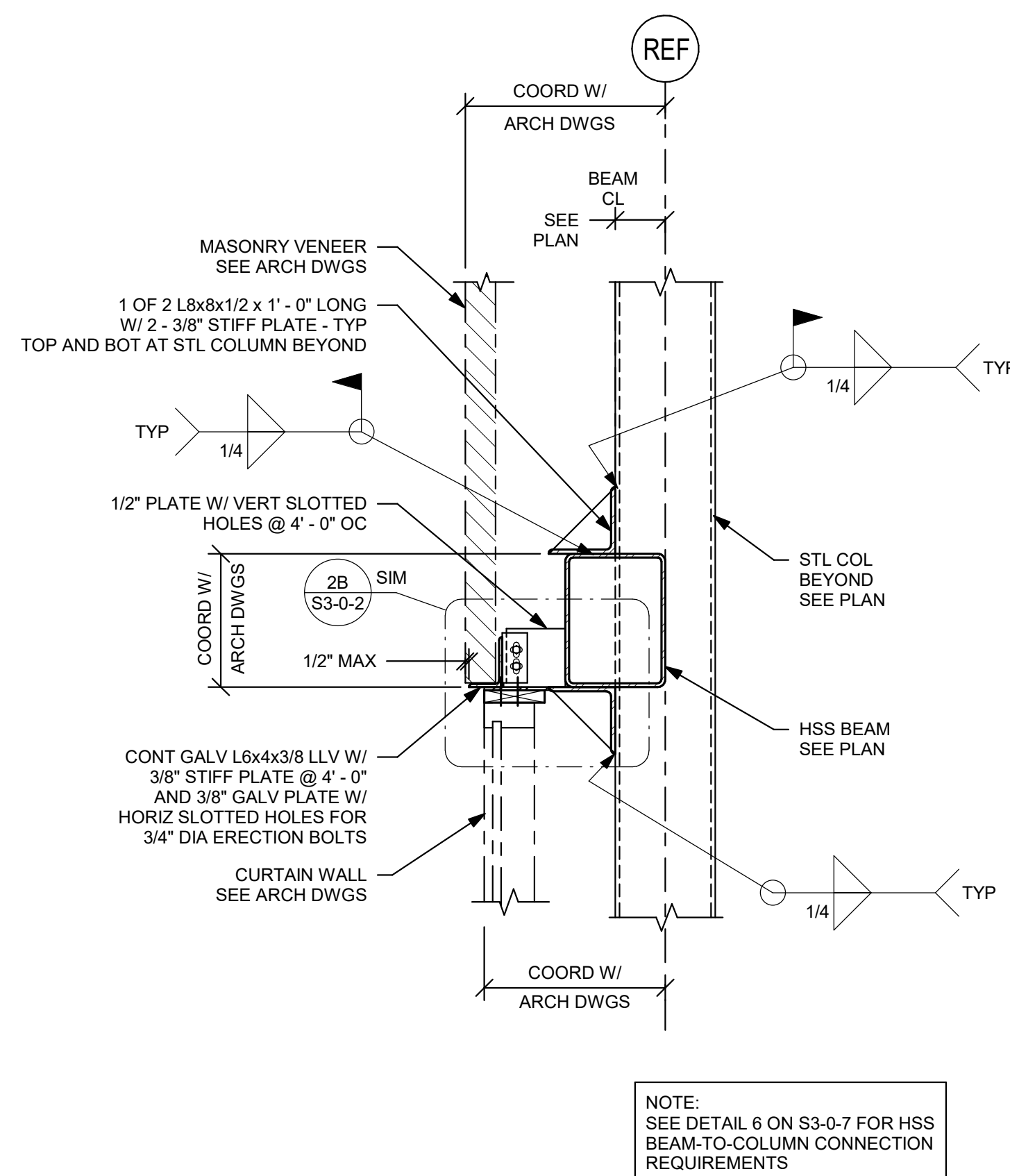
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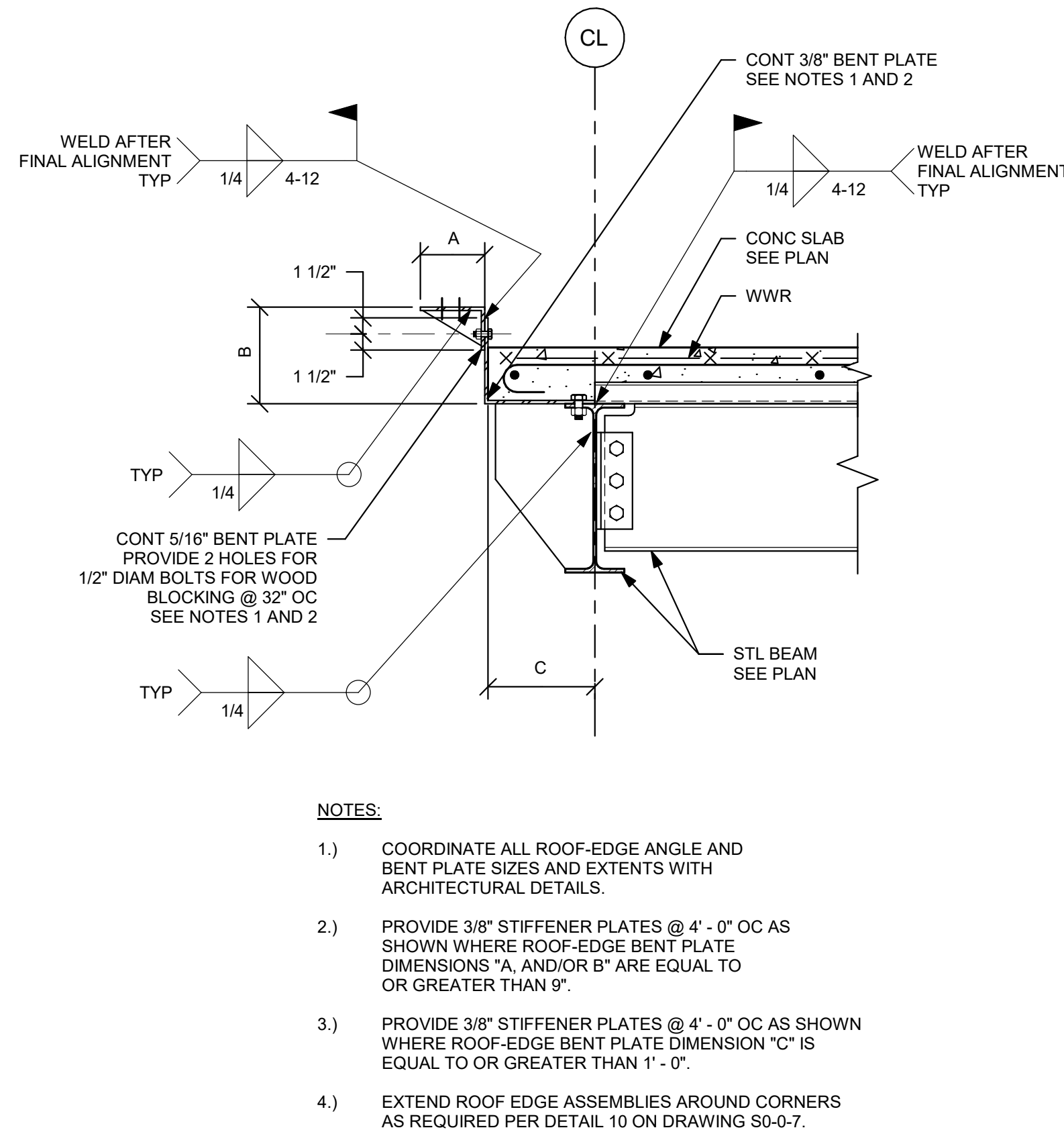
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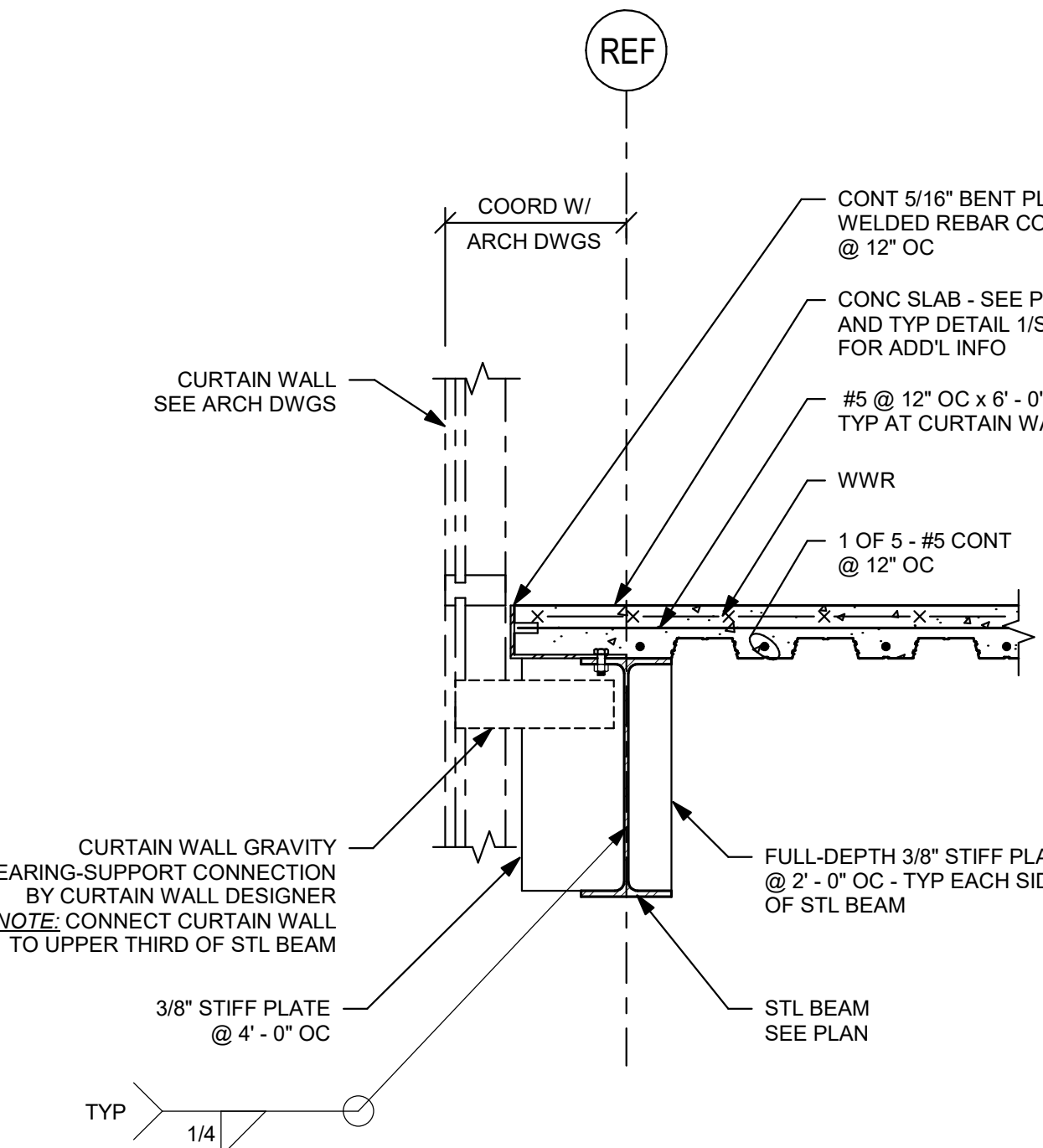
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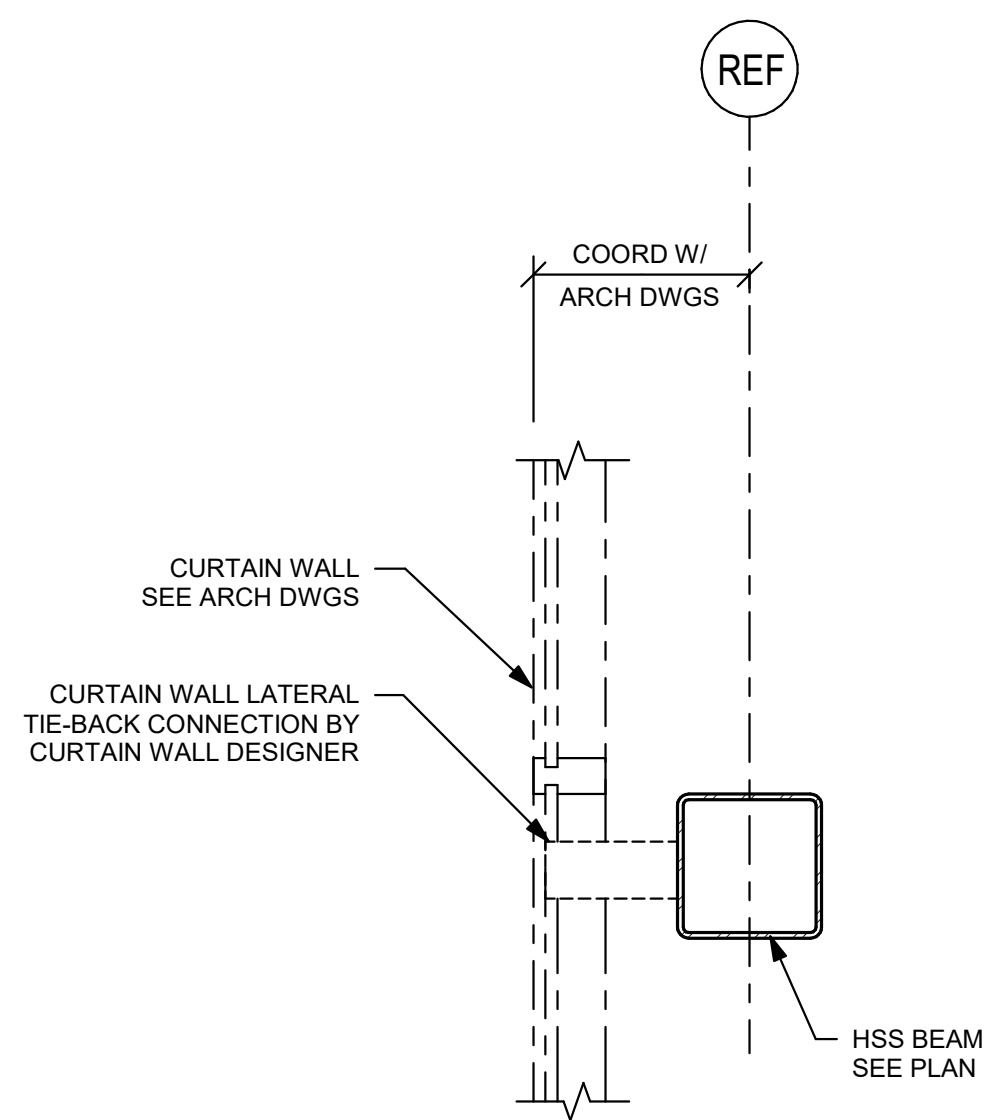
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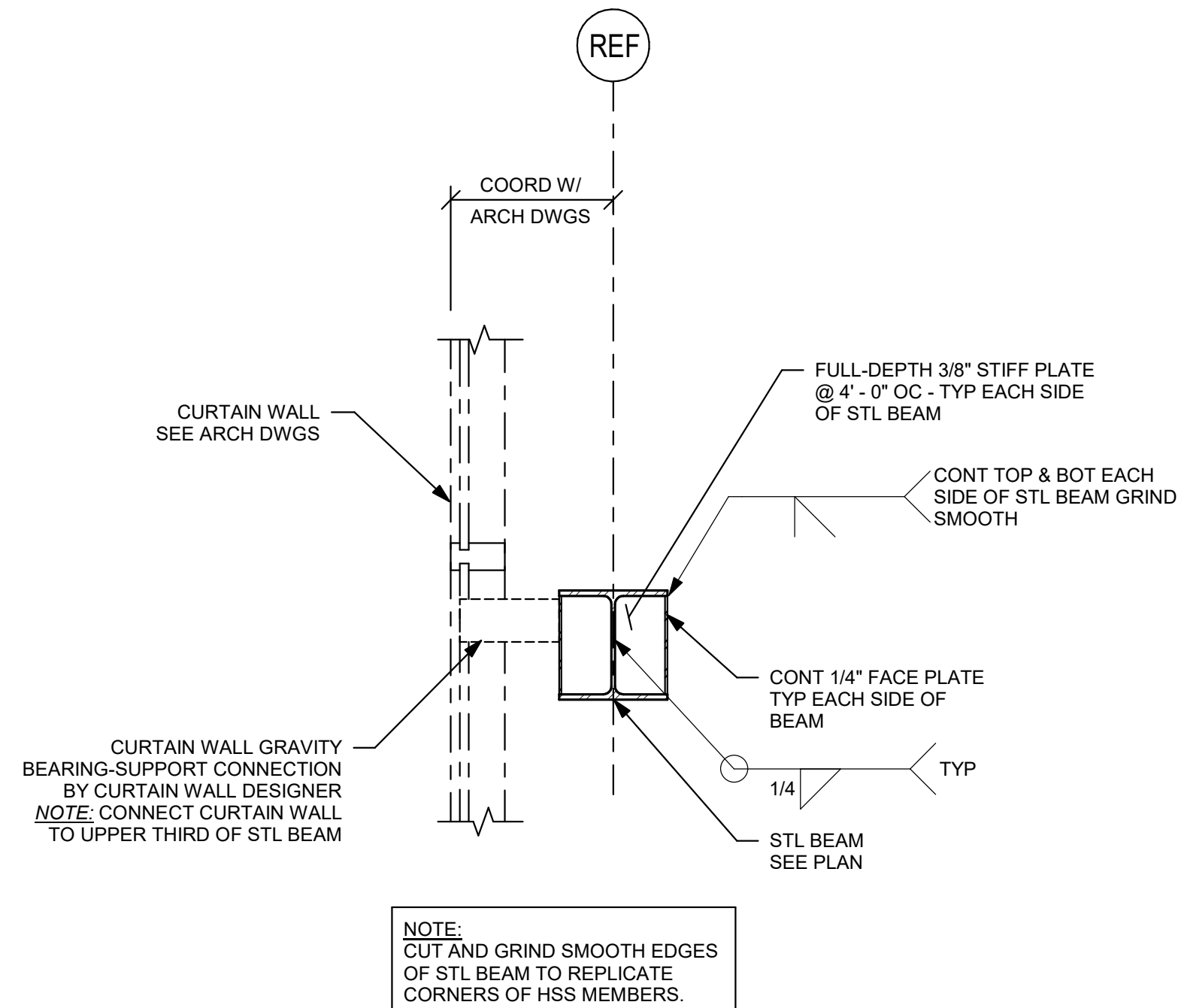
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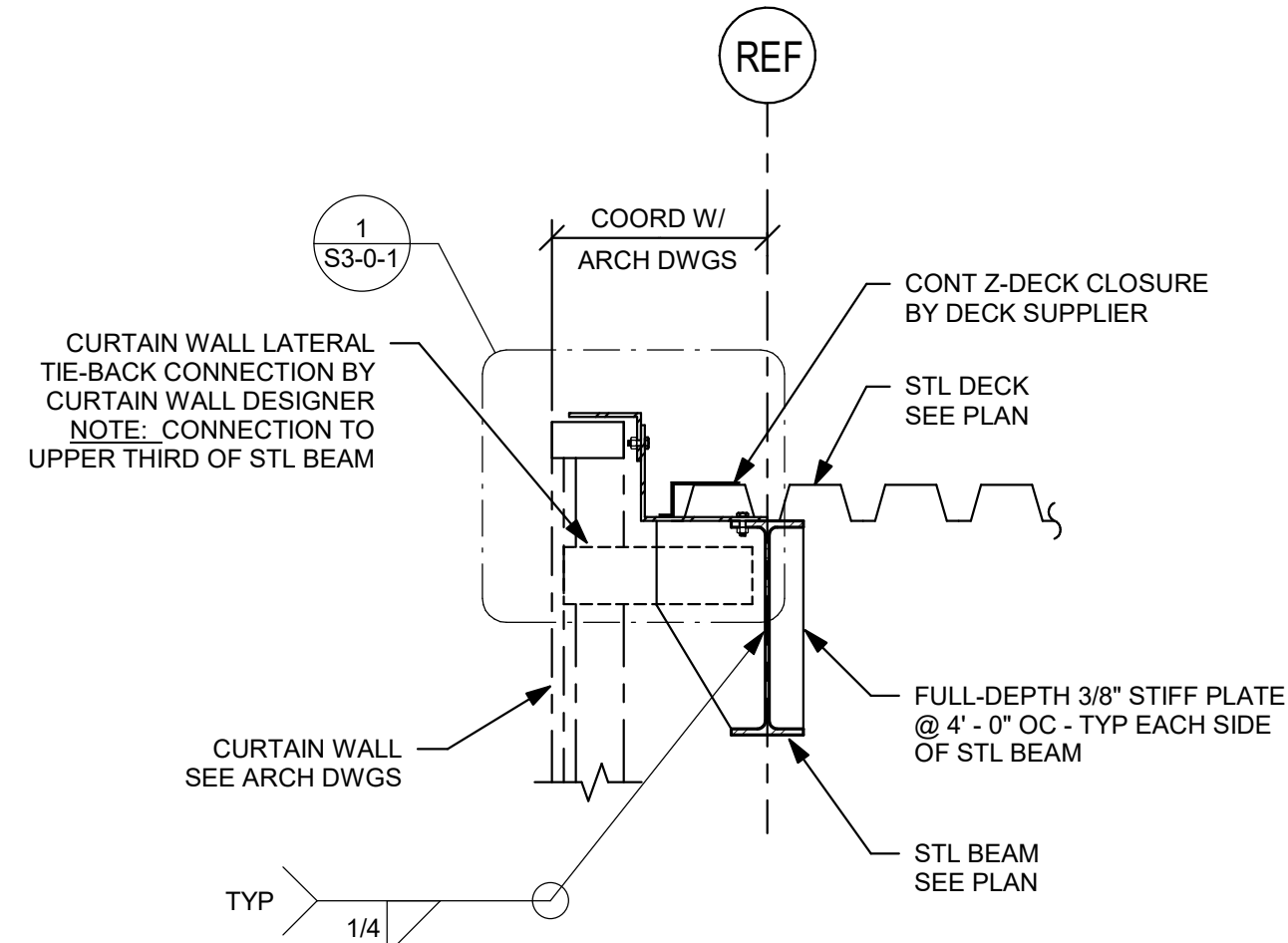
3/4" = 1' - 0"



3/4" = 1' - 0"



3/4" = 1' - 0"



3/4" = 1' - 0"

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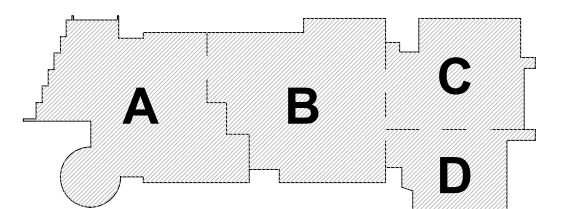
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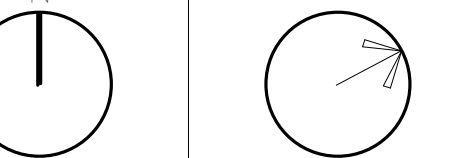
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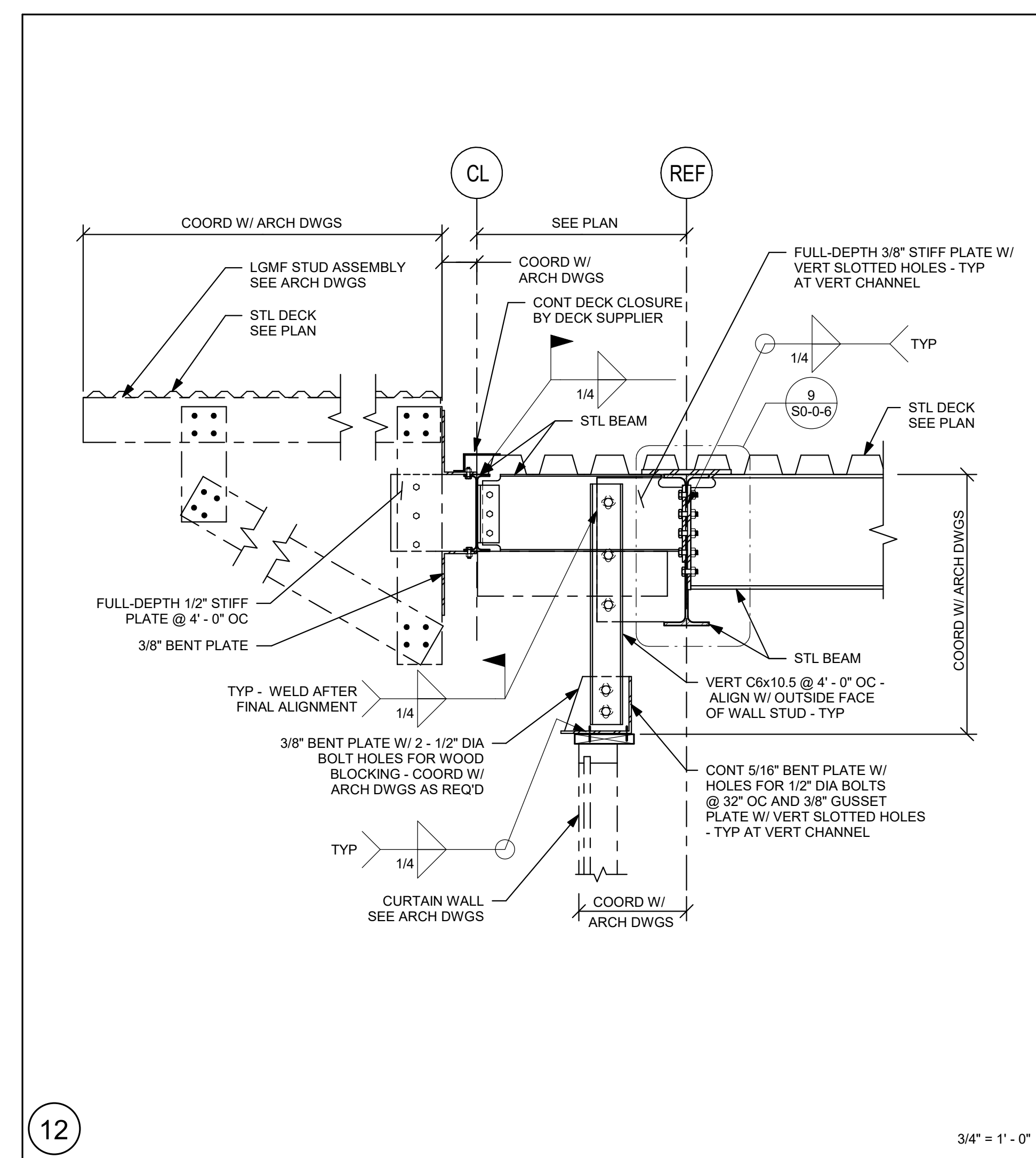
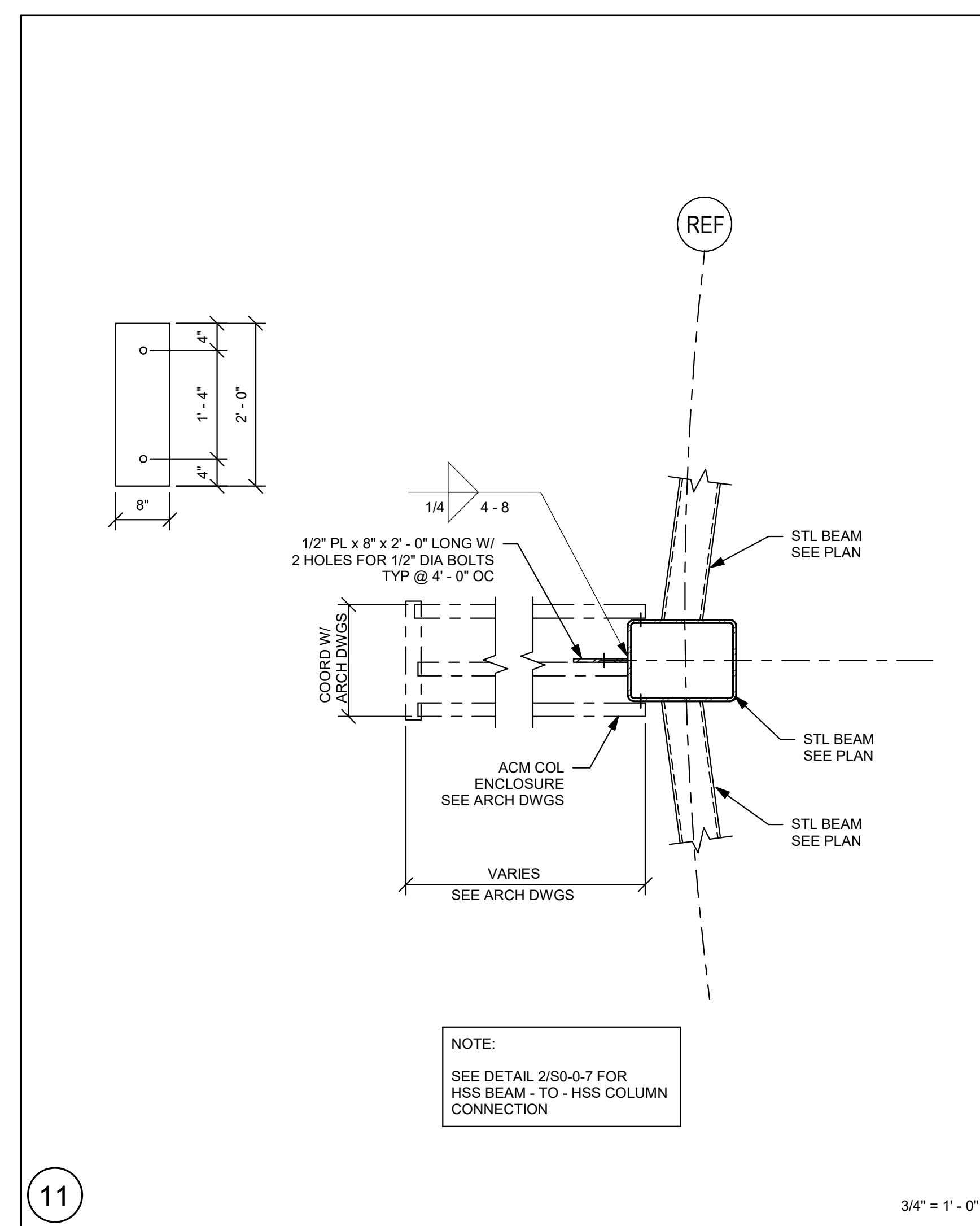
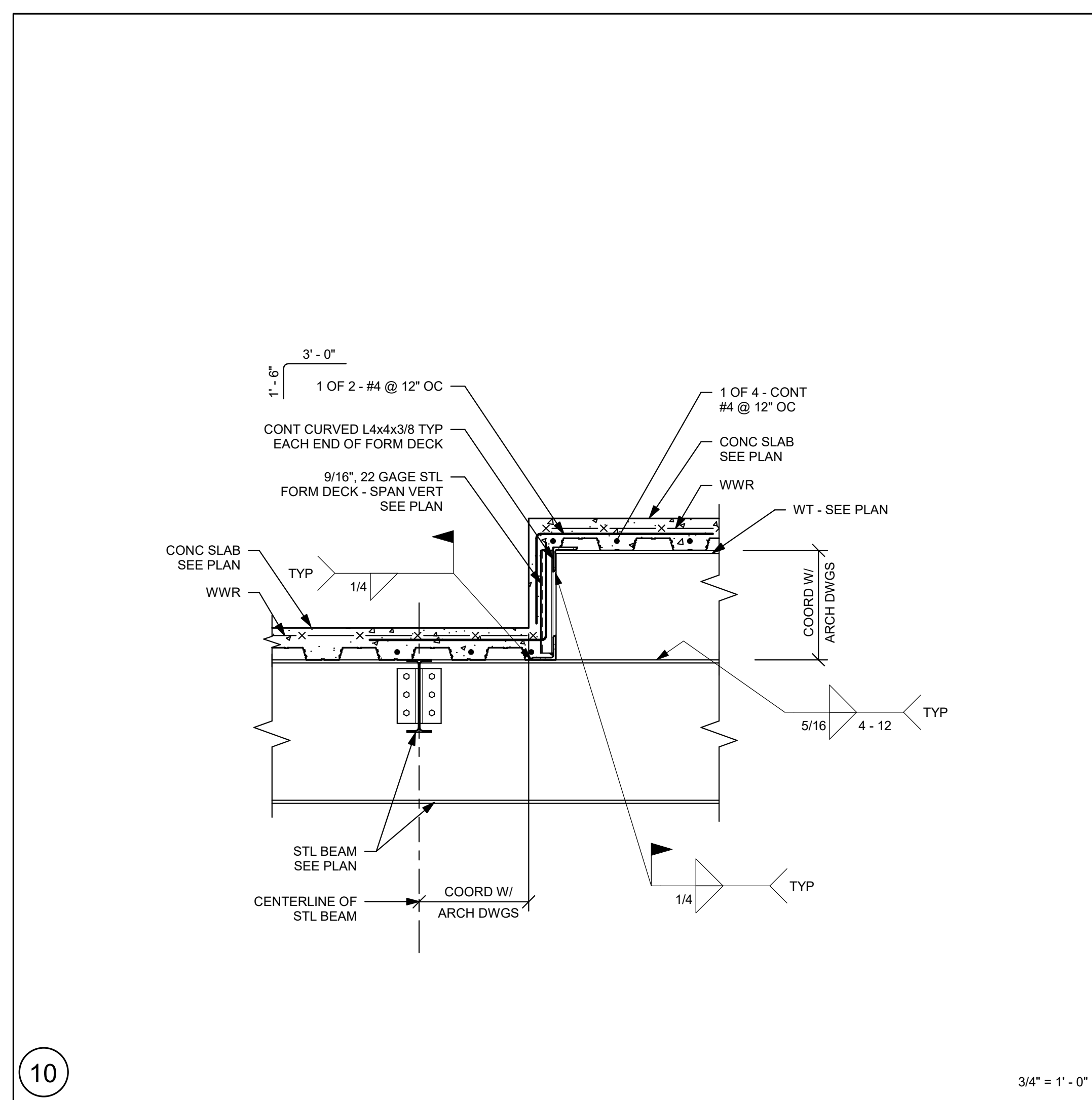
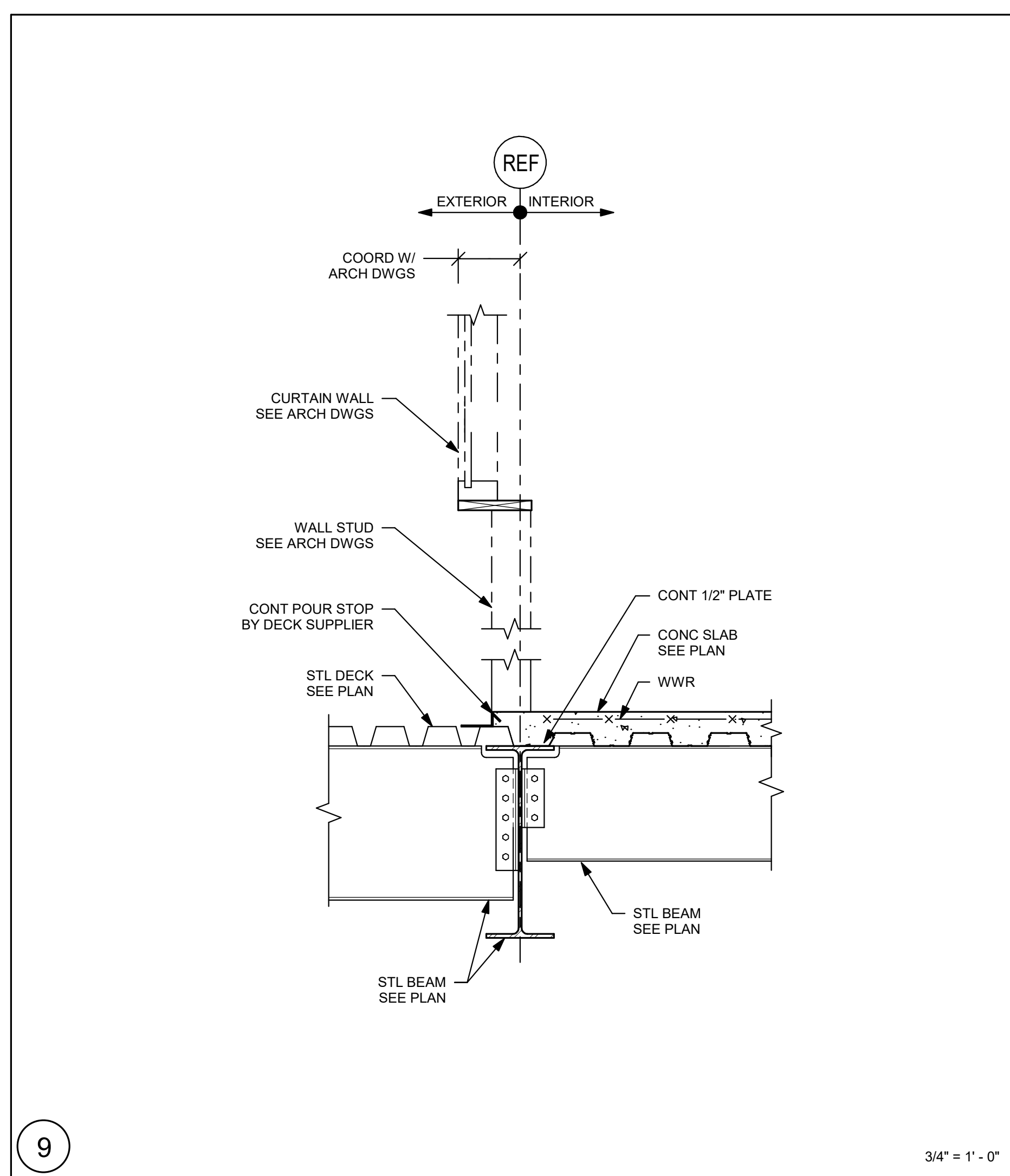
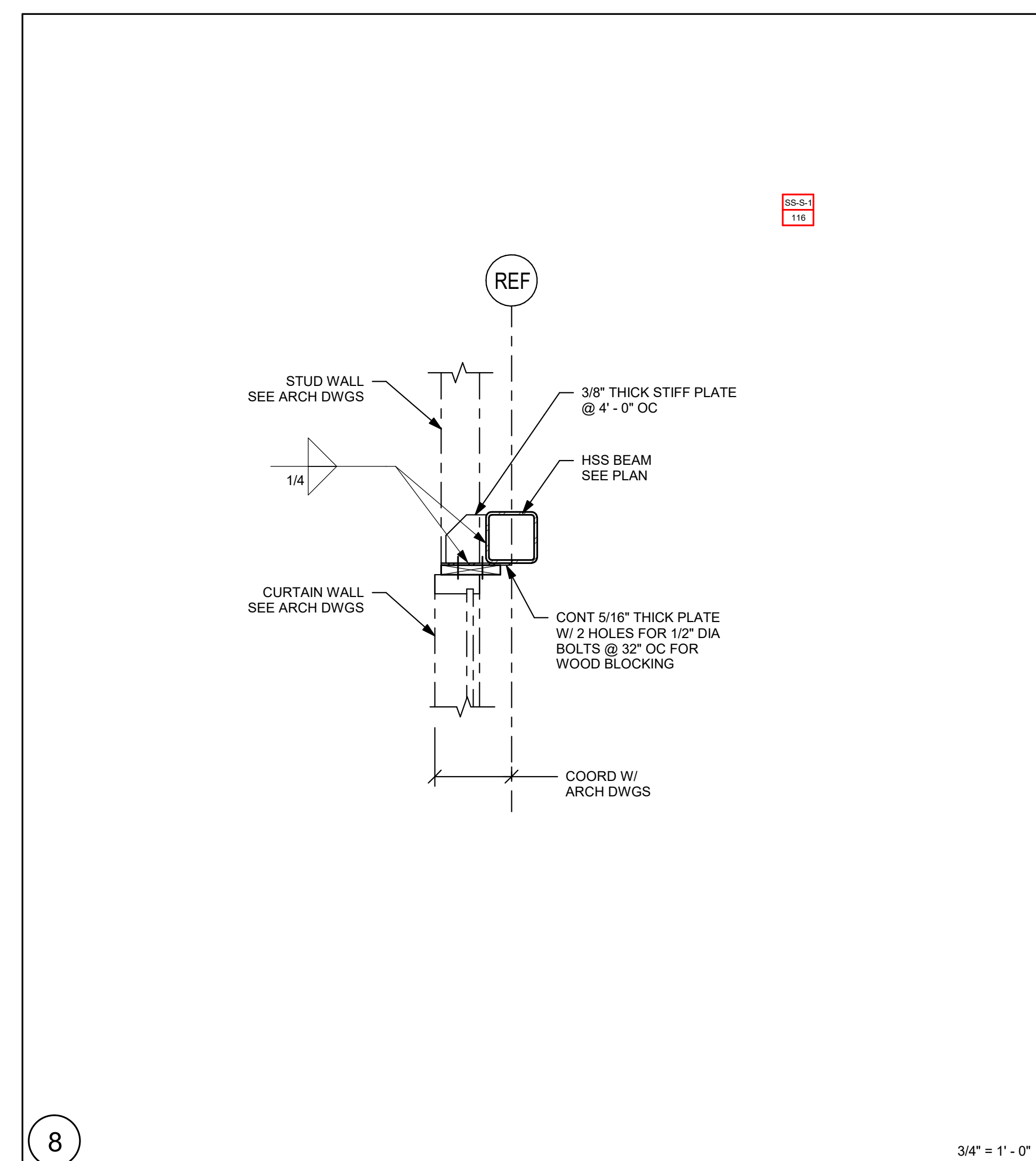
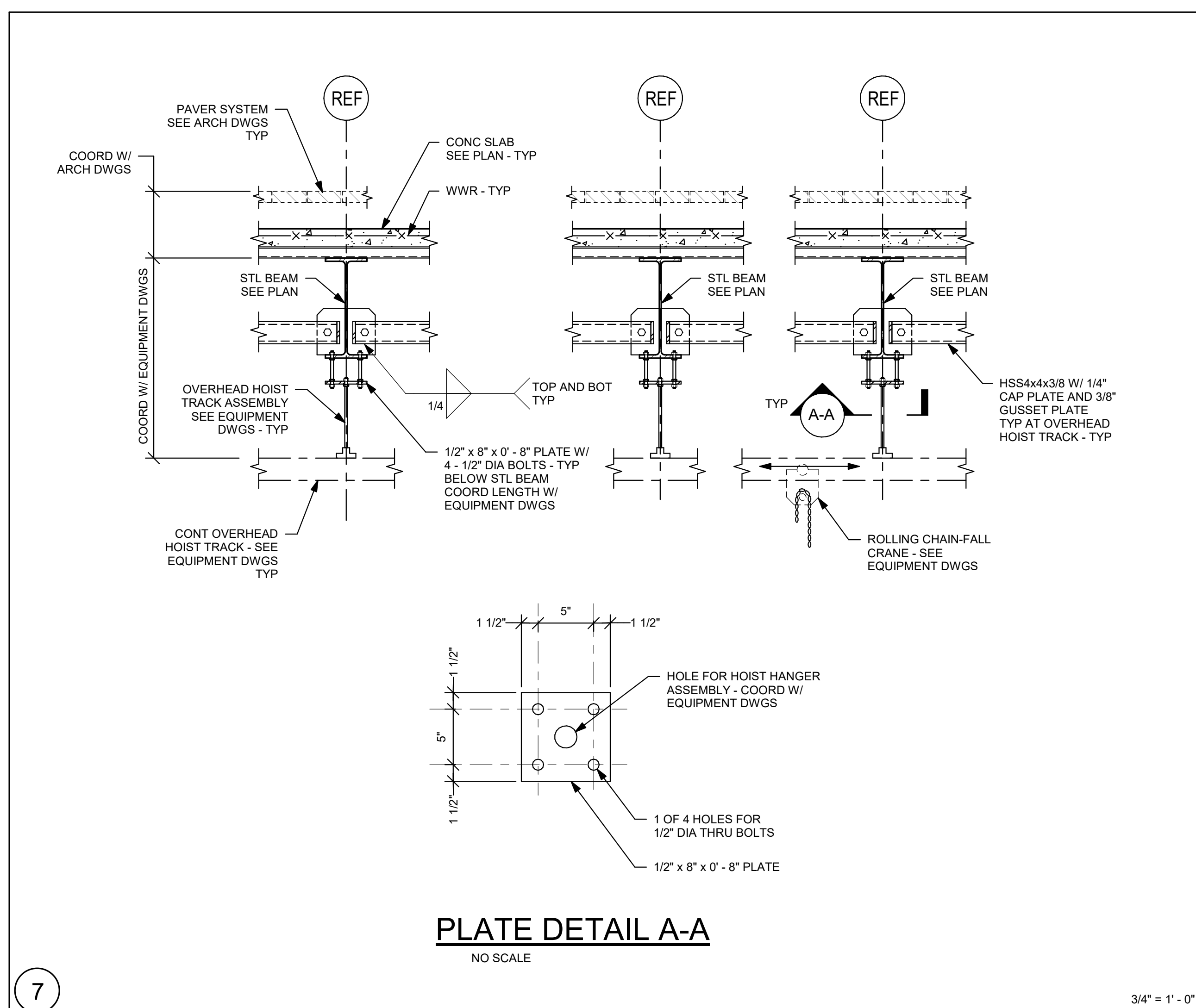
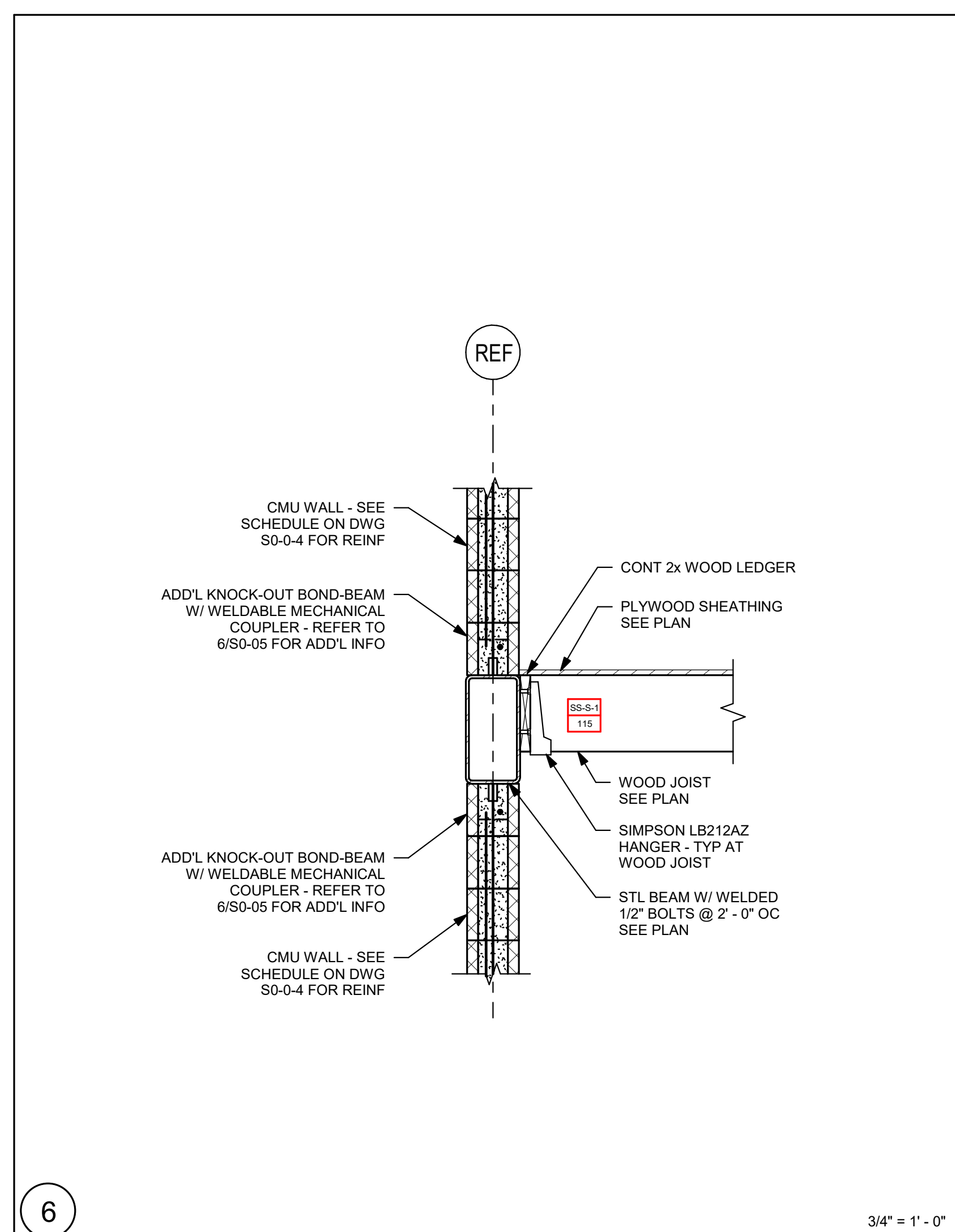
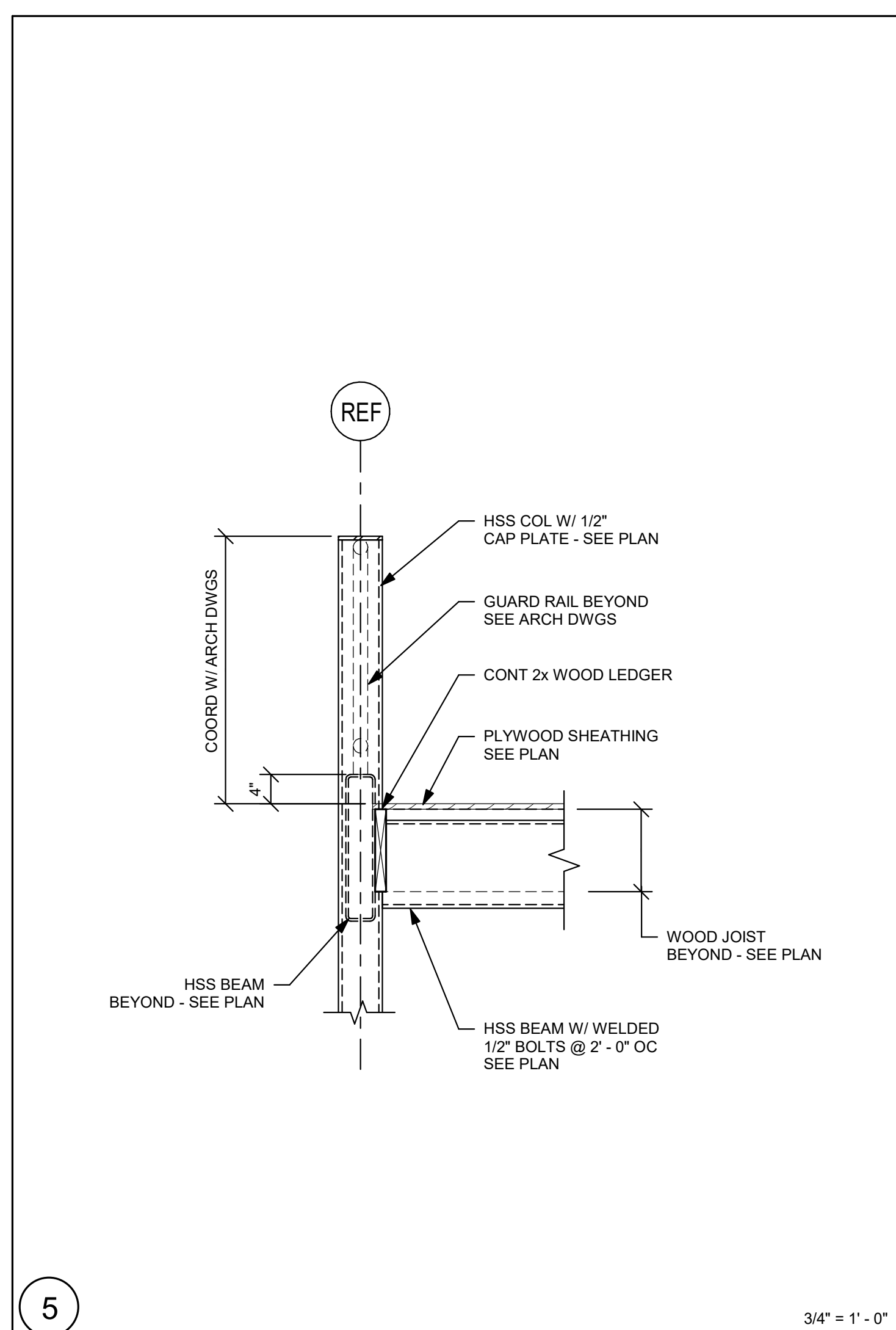
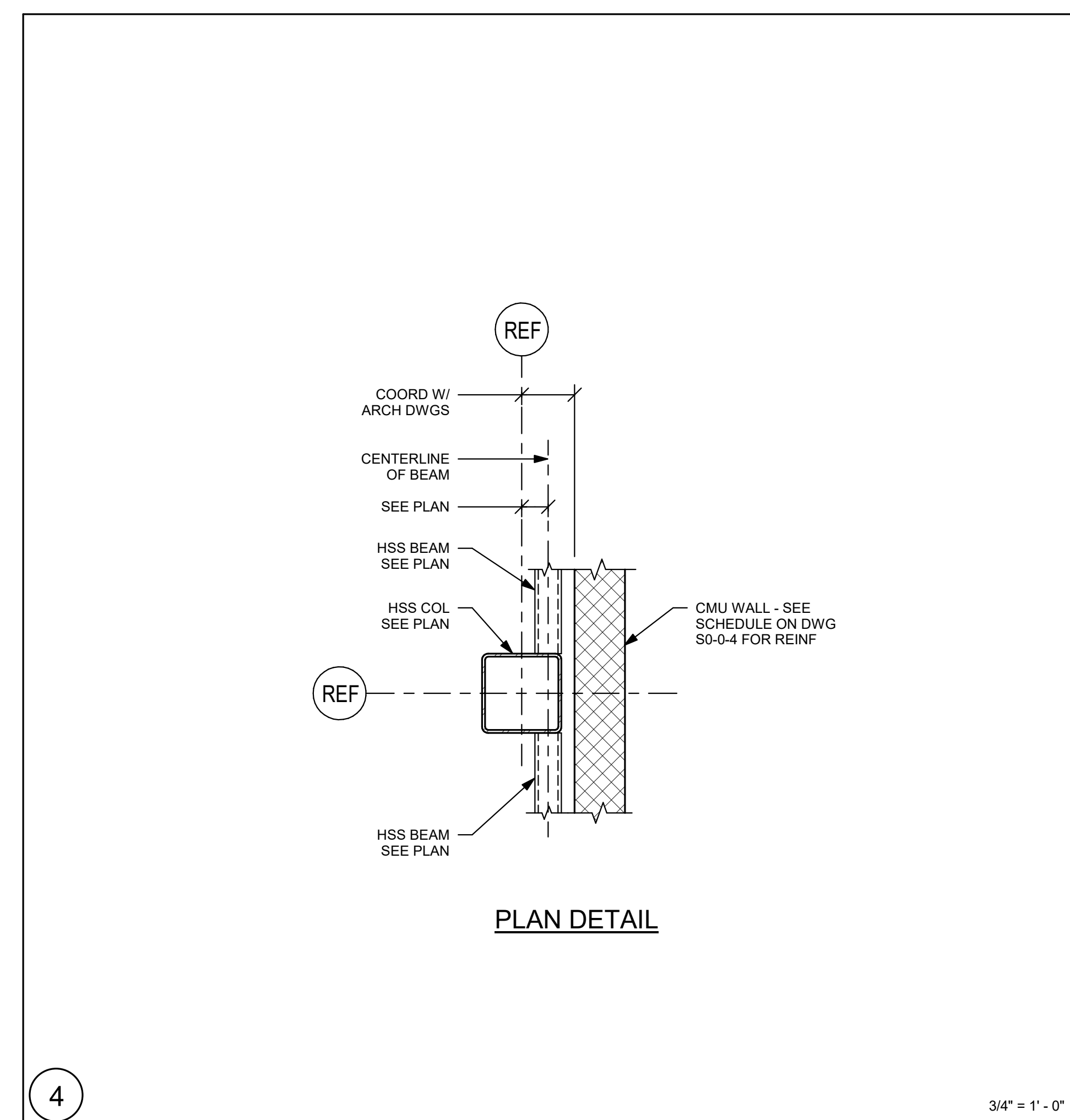
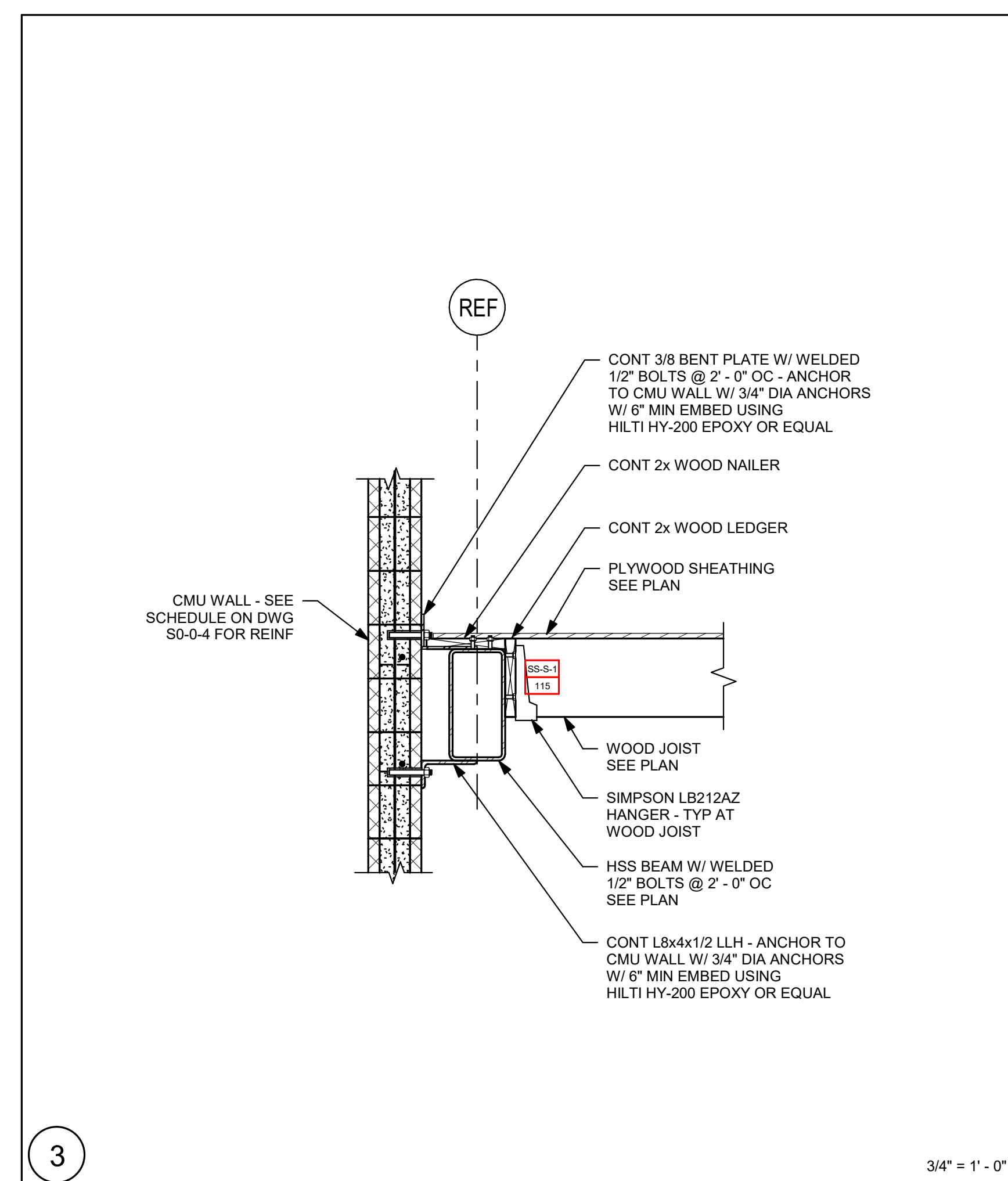
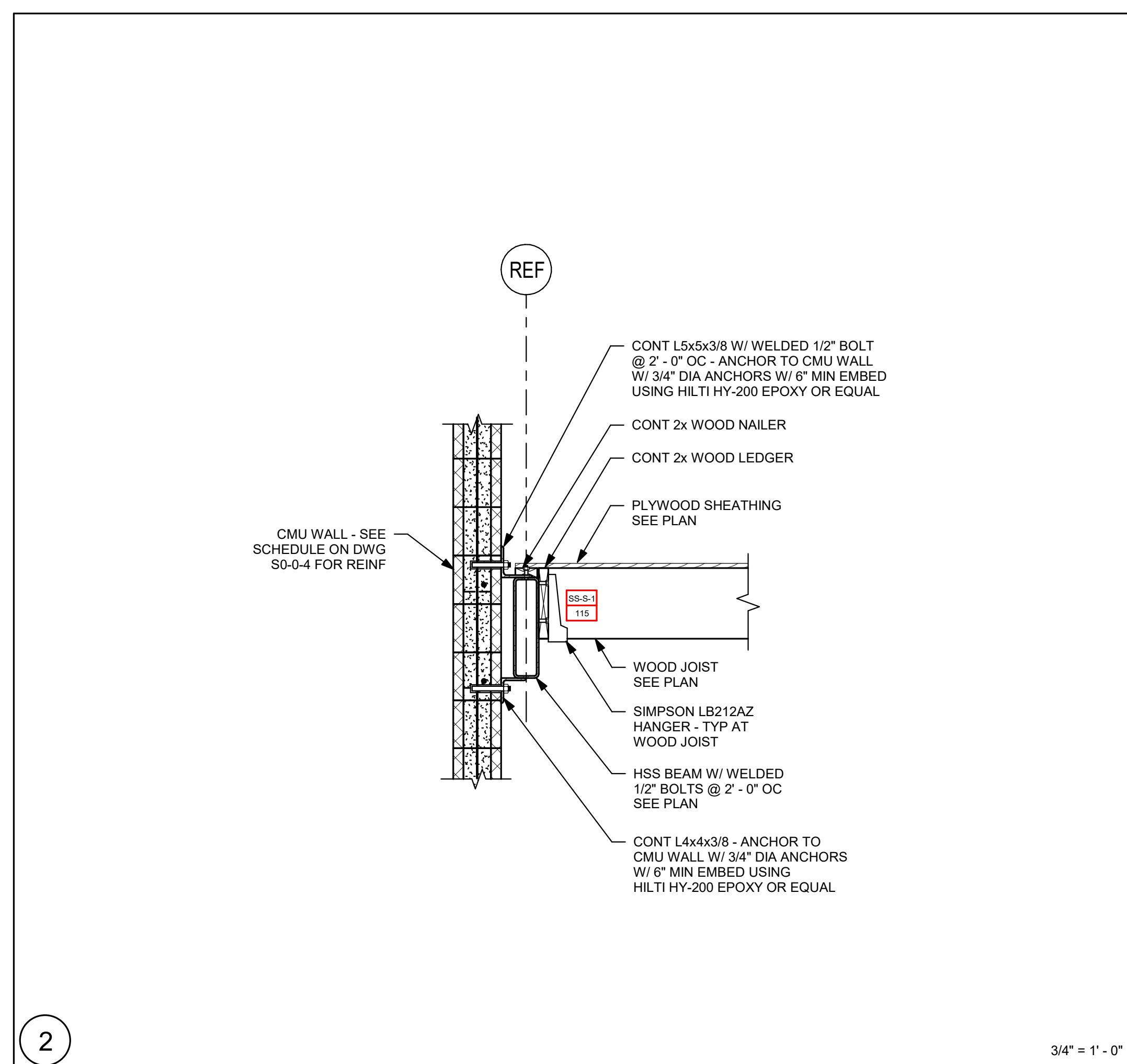
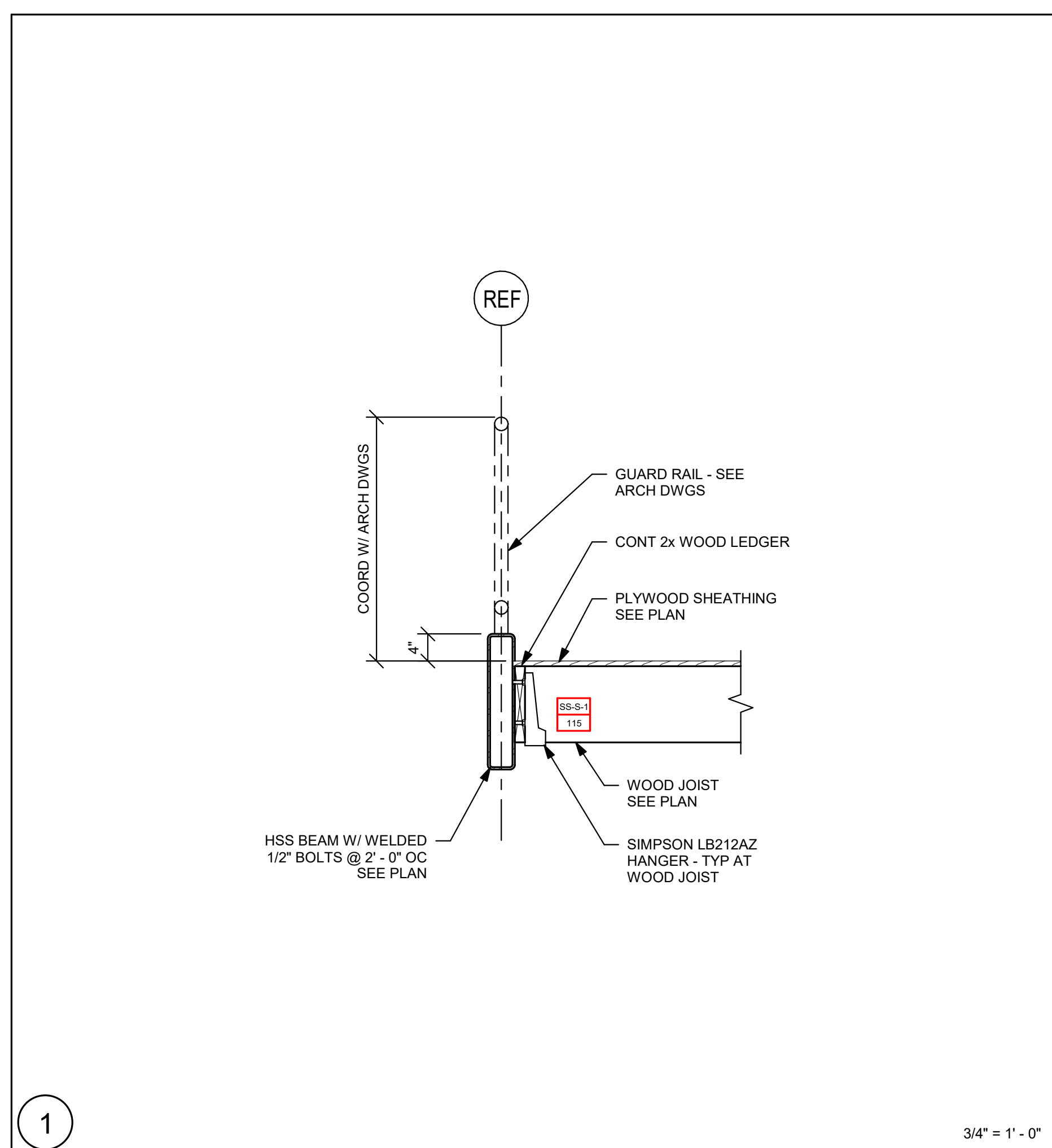
KEY PLAN

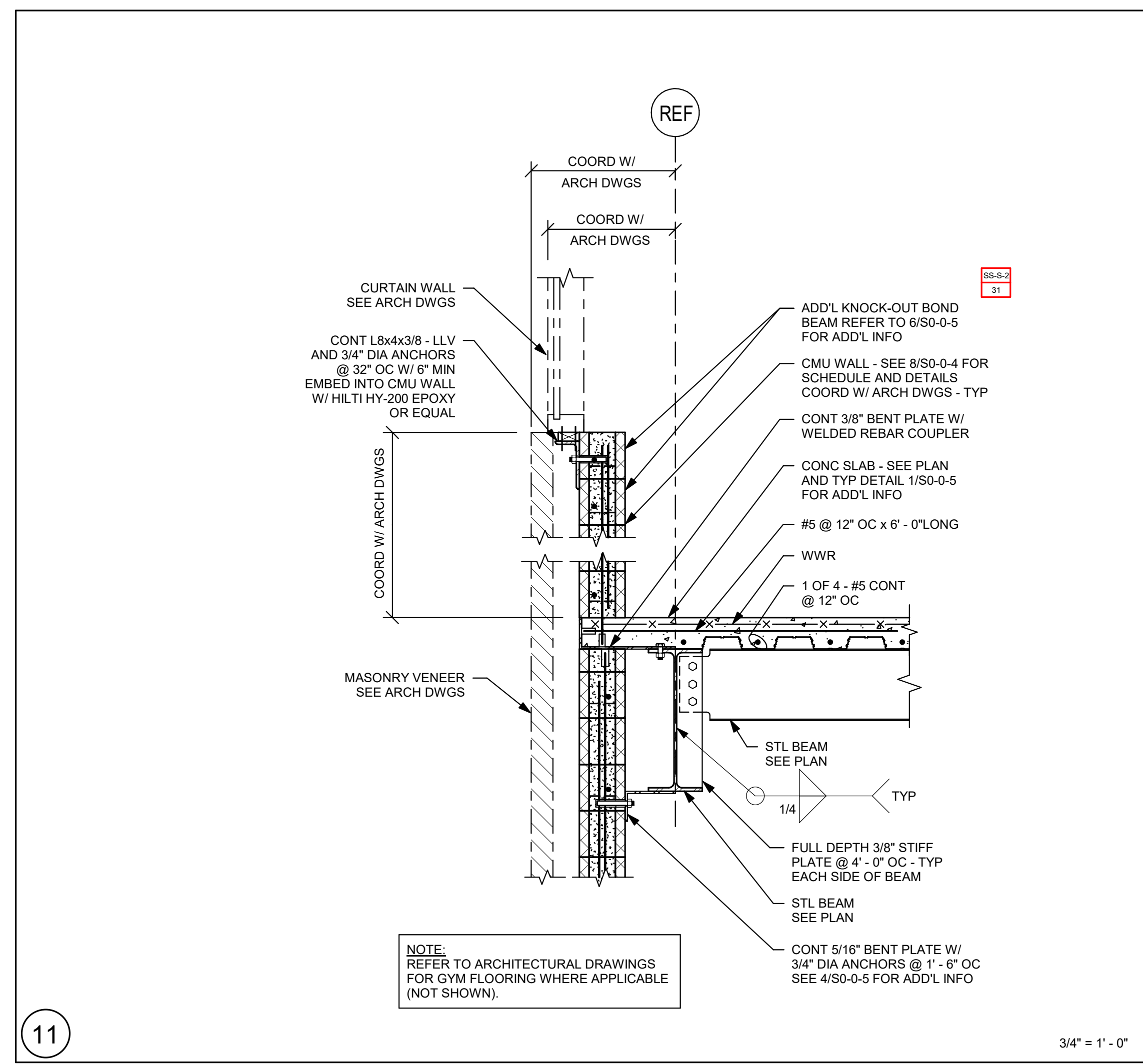
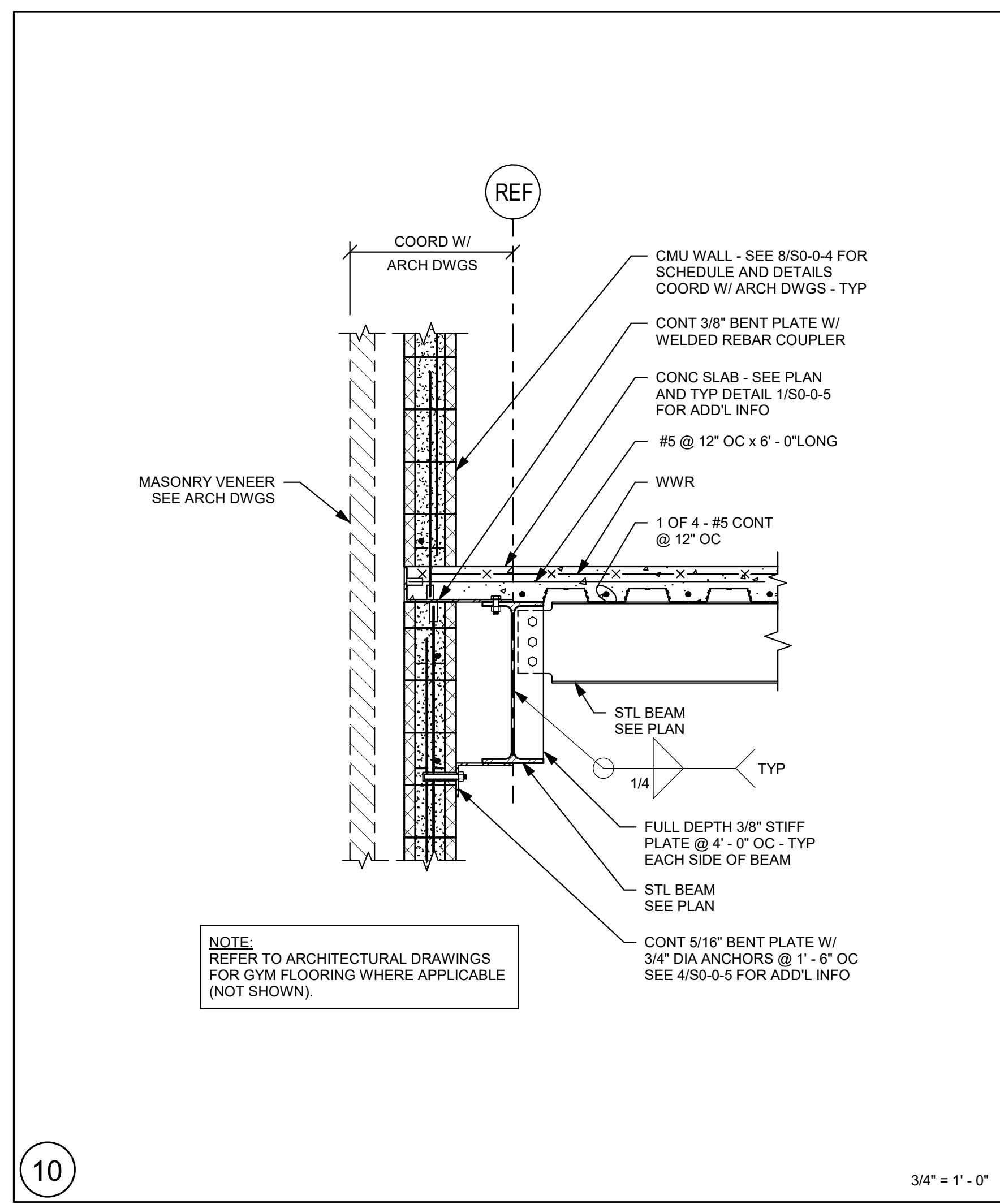
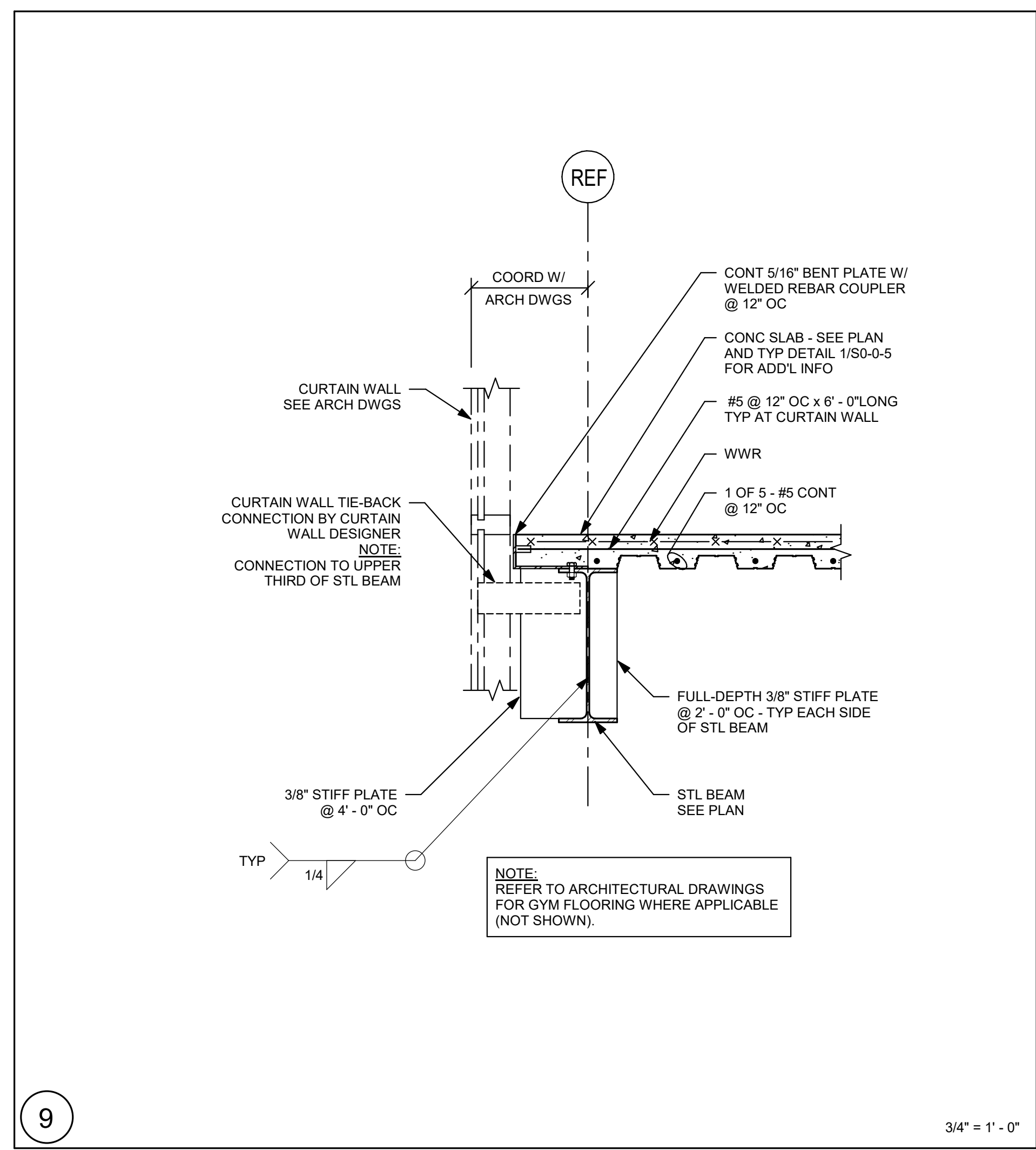
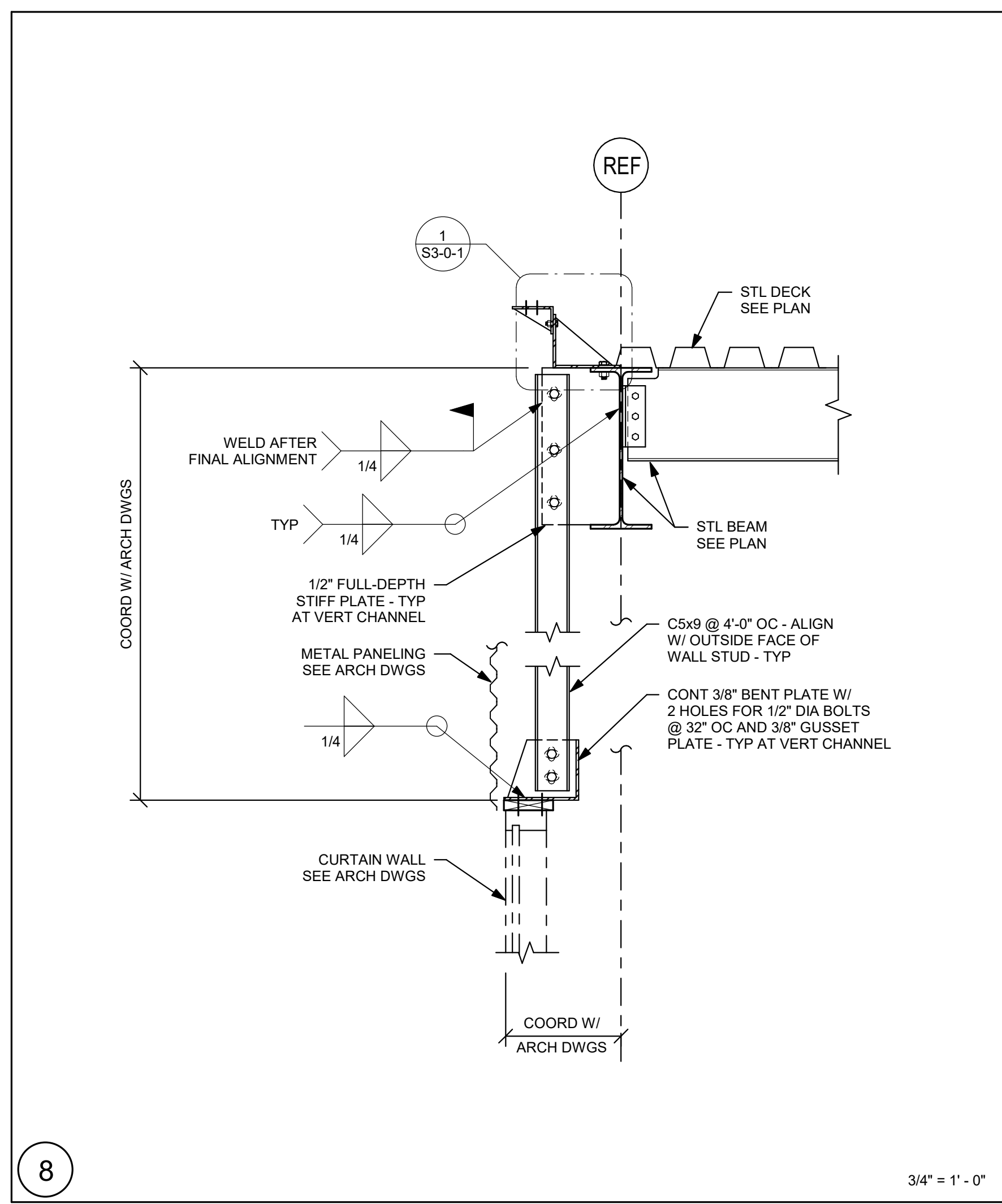
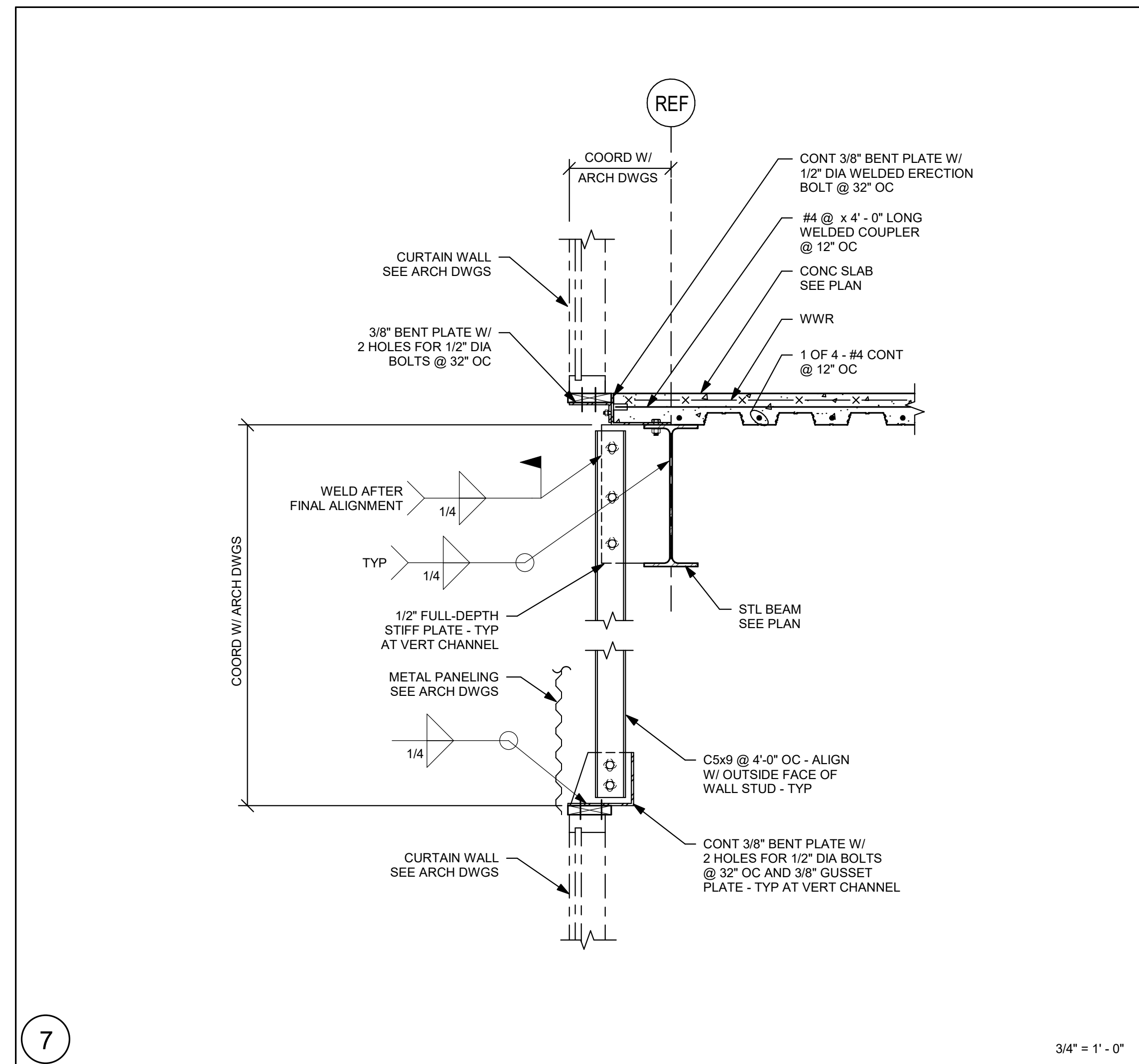
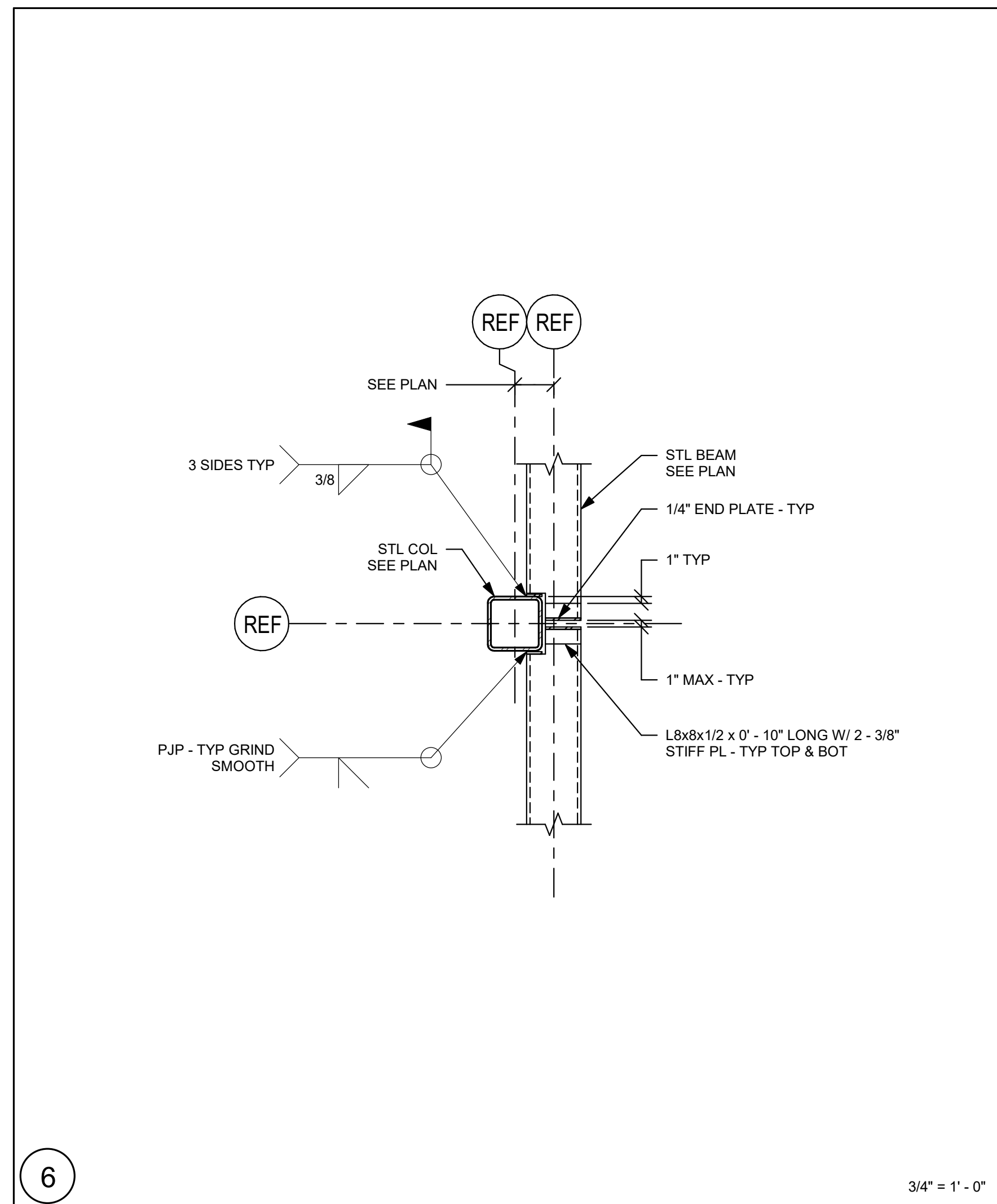
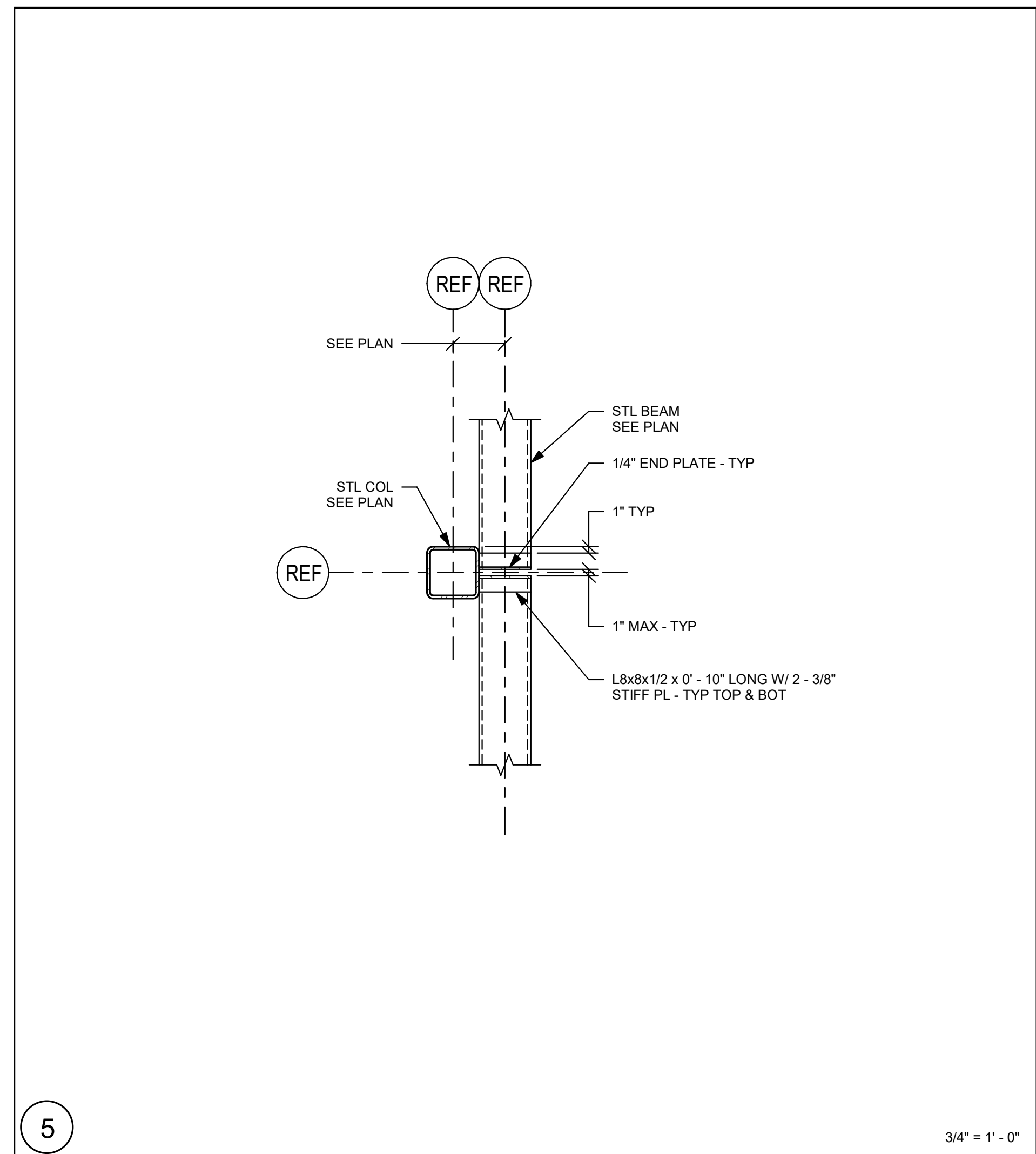
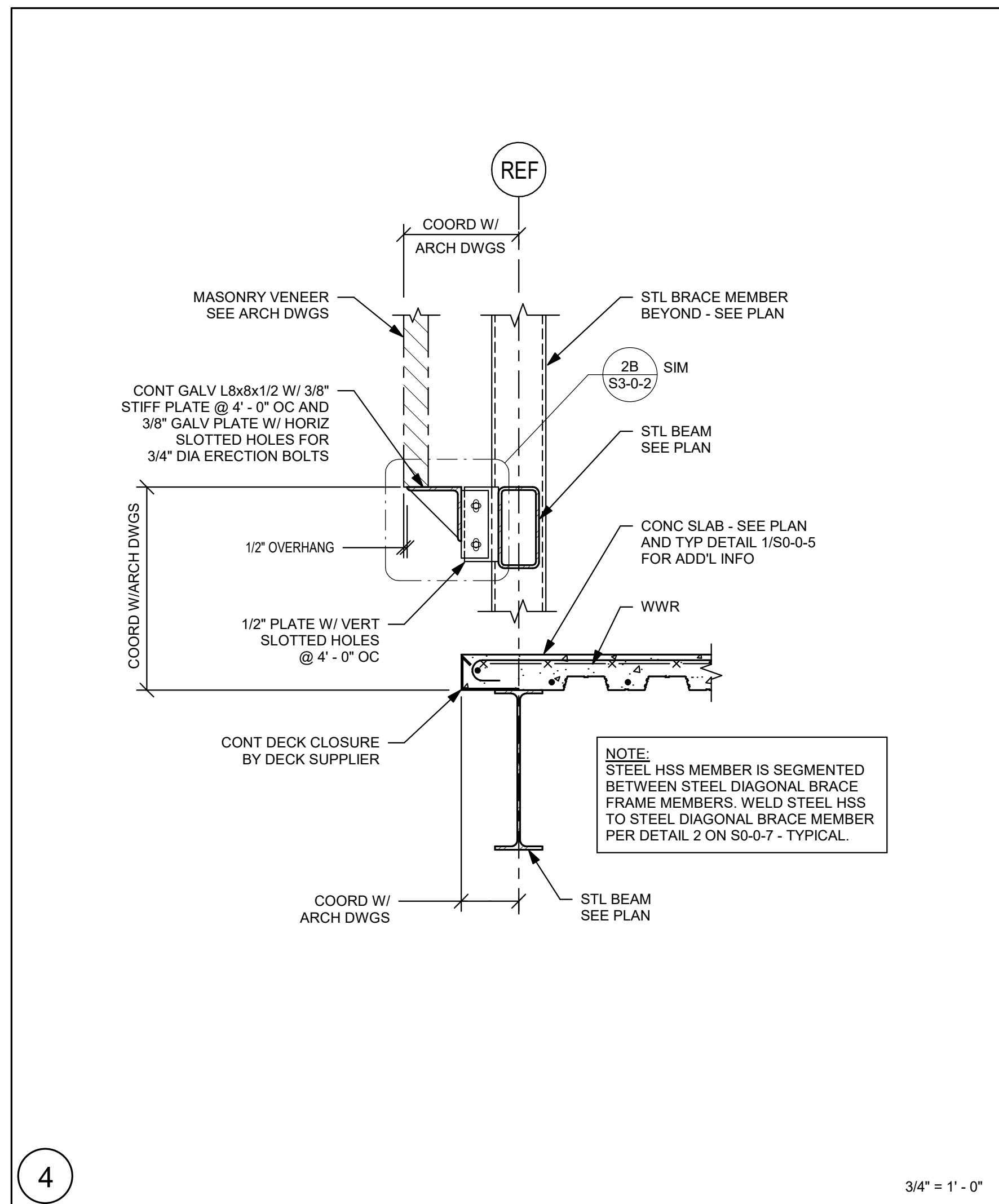
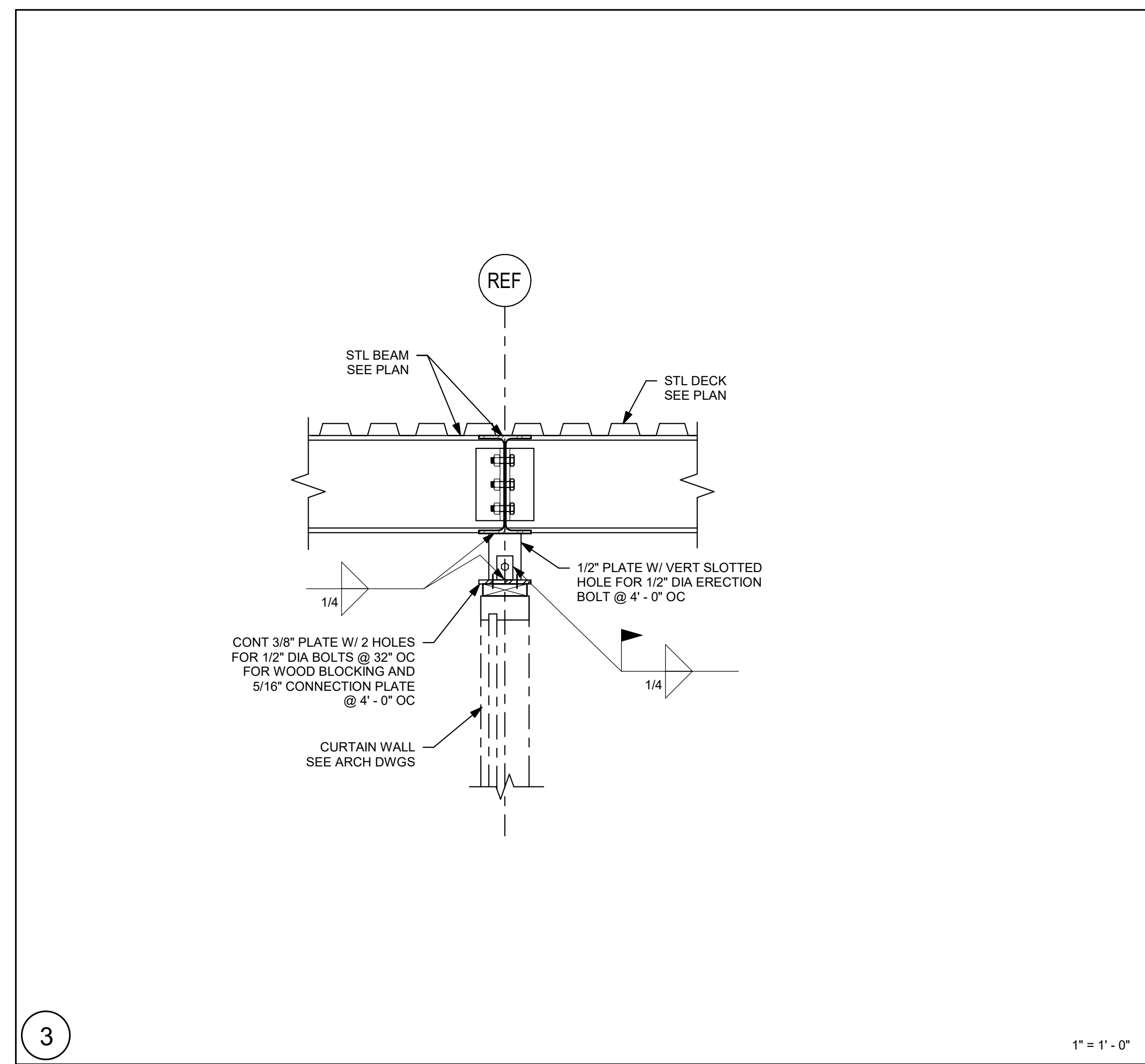
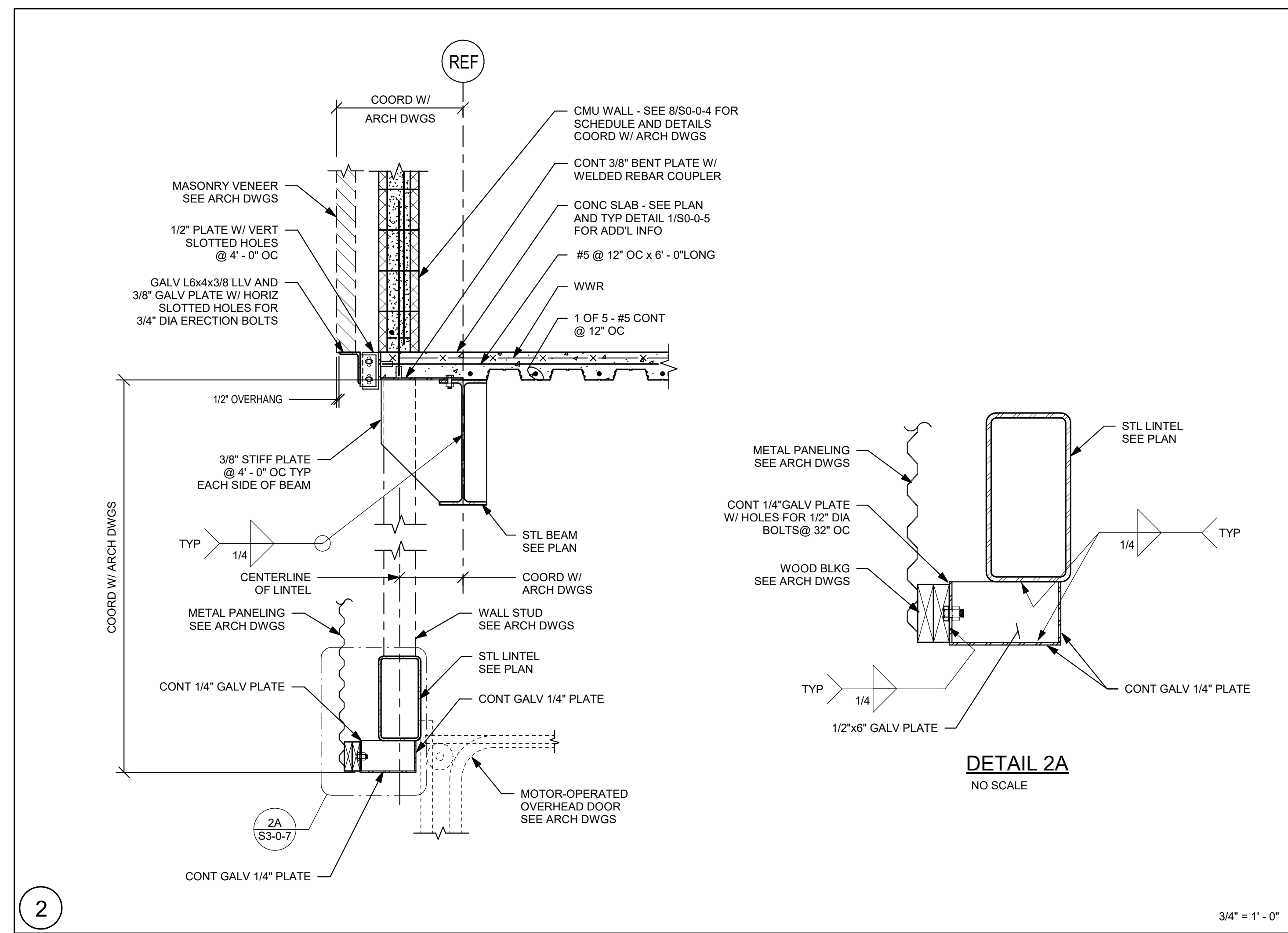
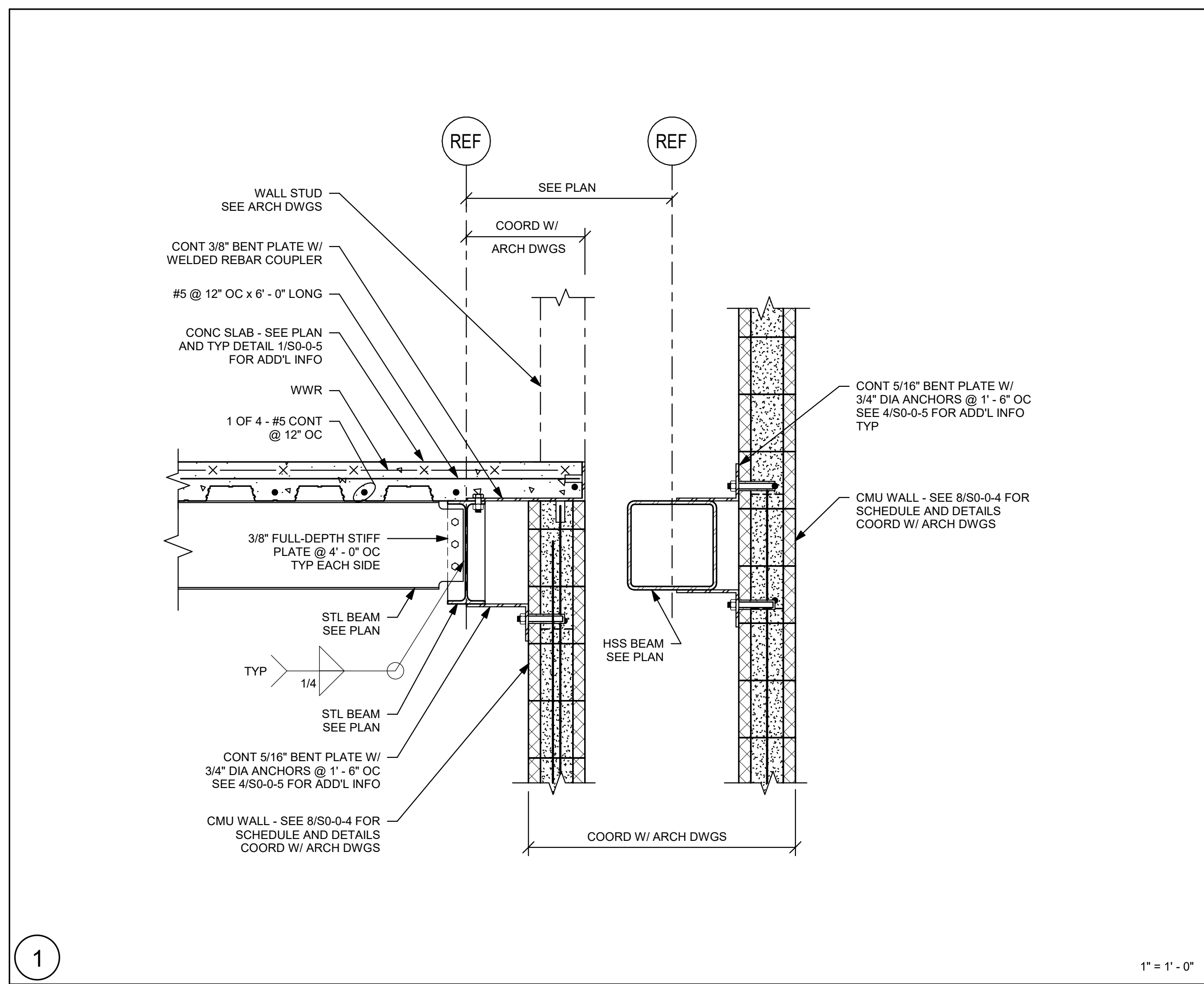
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Date: August 28th, 2023
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MAGNETIC NORTH

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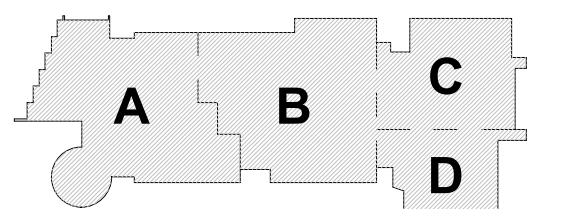
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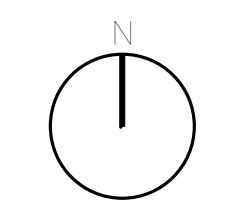
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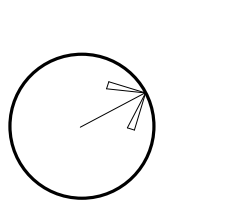


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MAGNETIC NORTH



SECTIONS

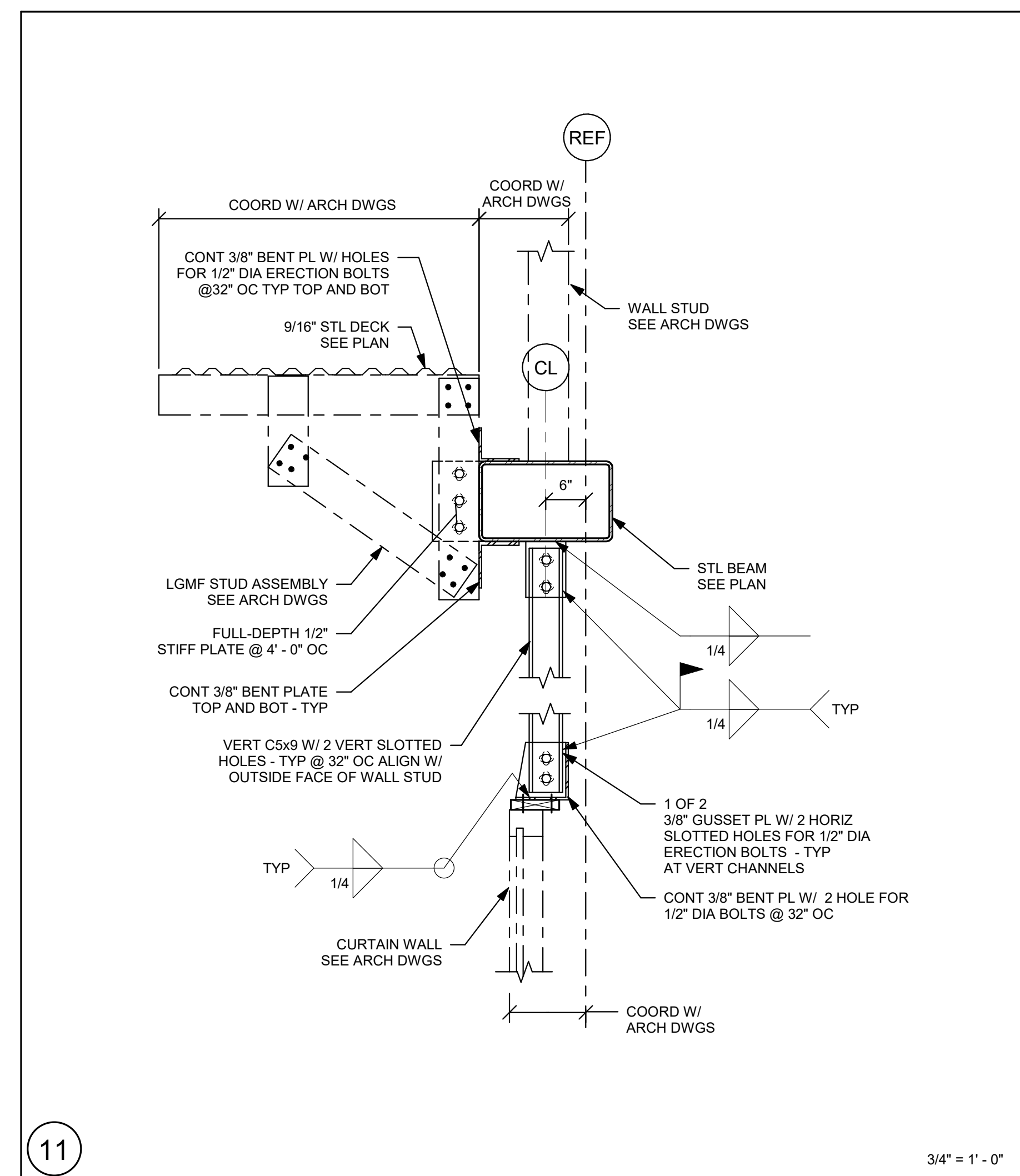
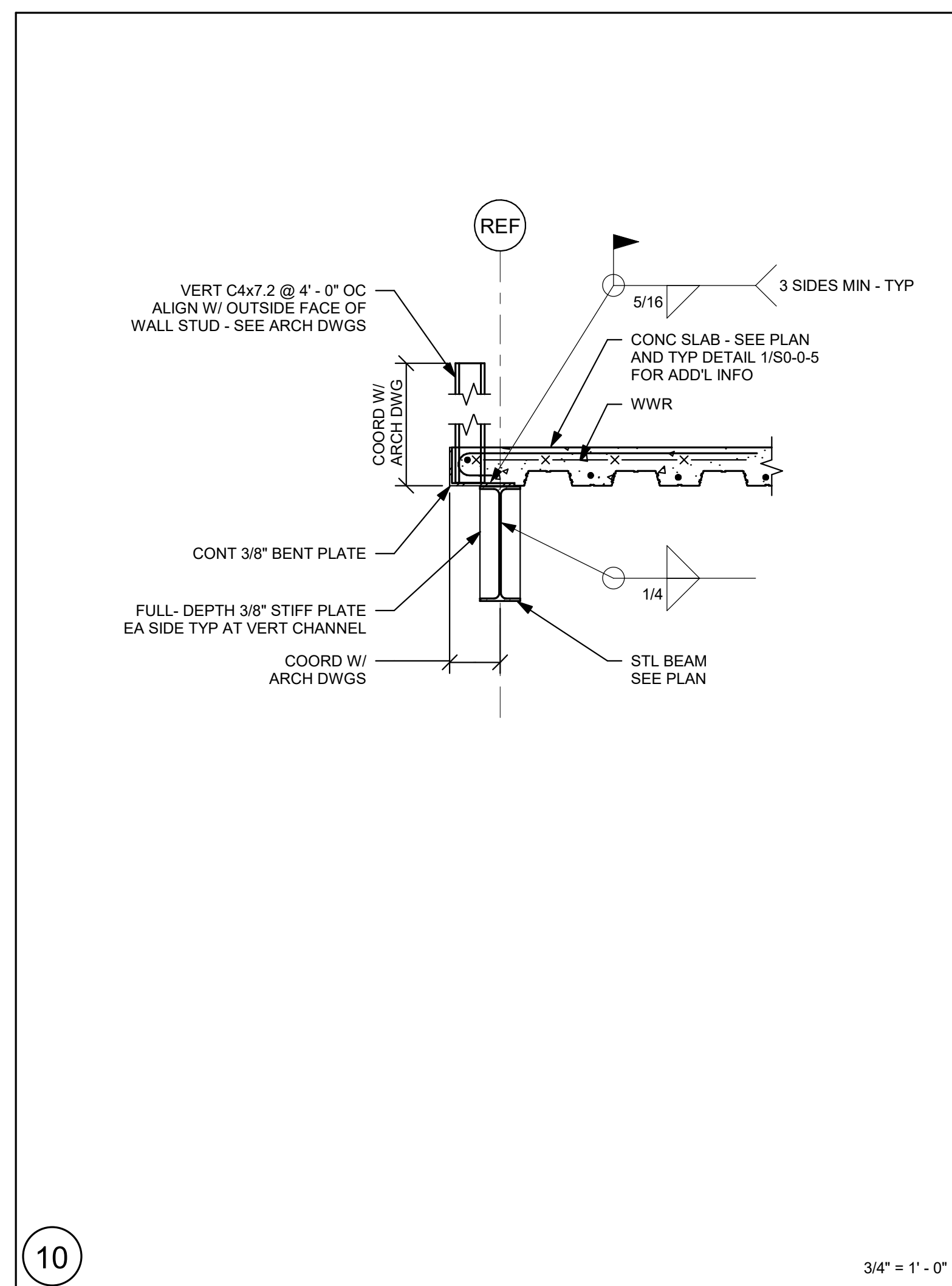
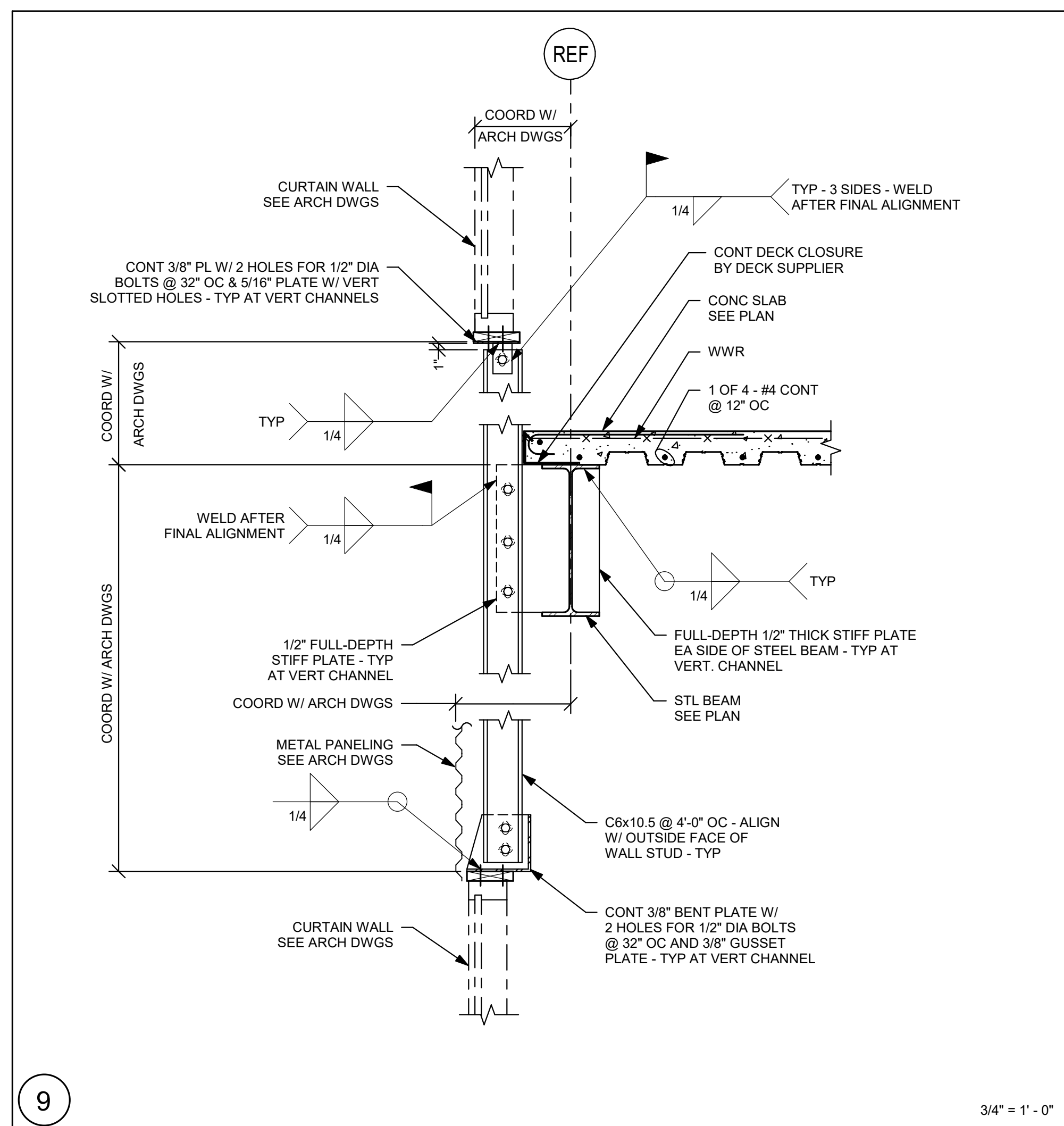
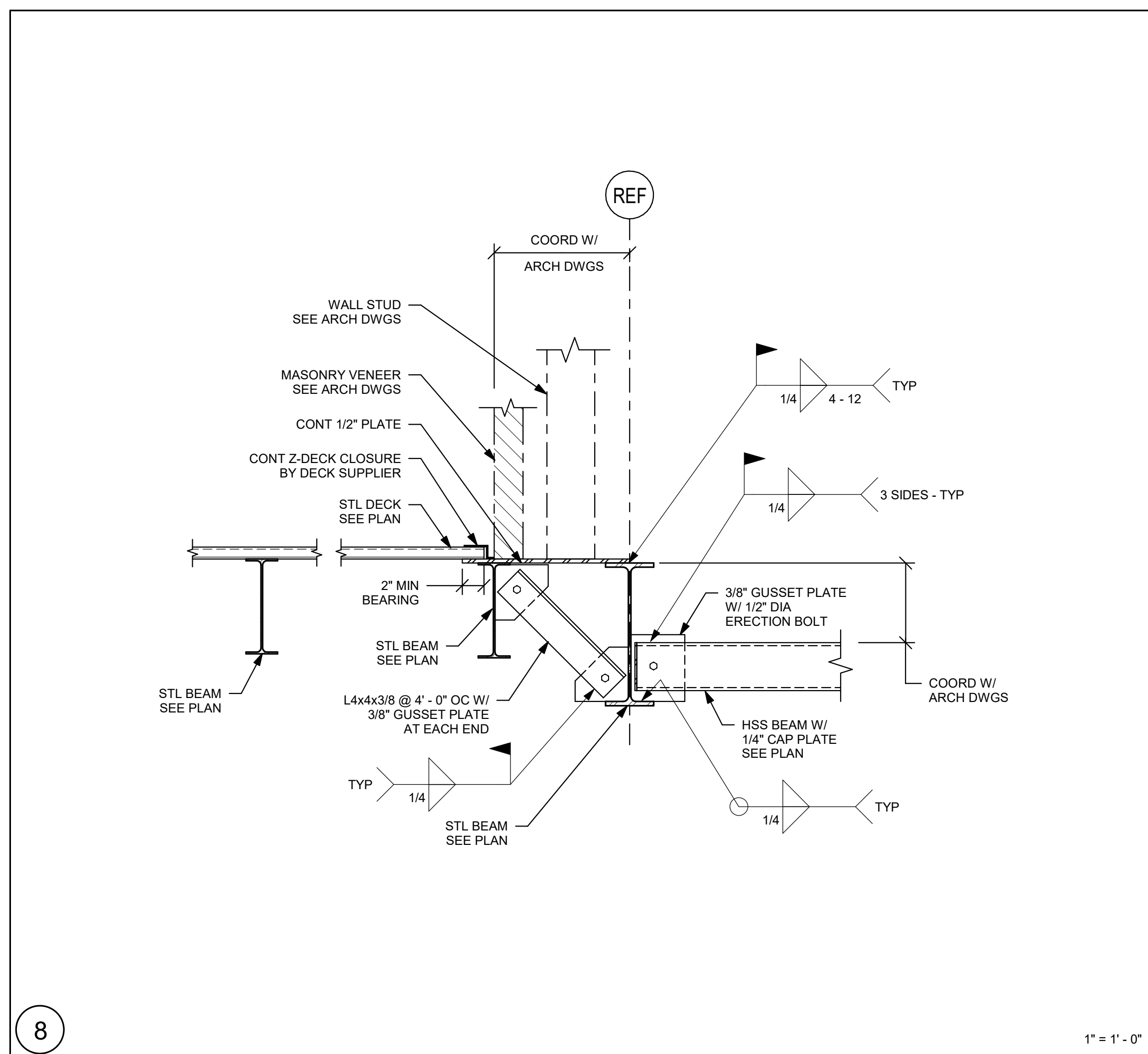
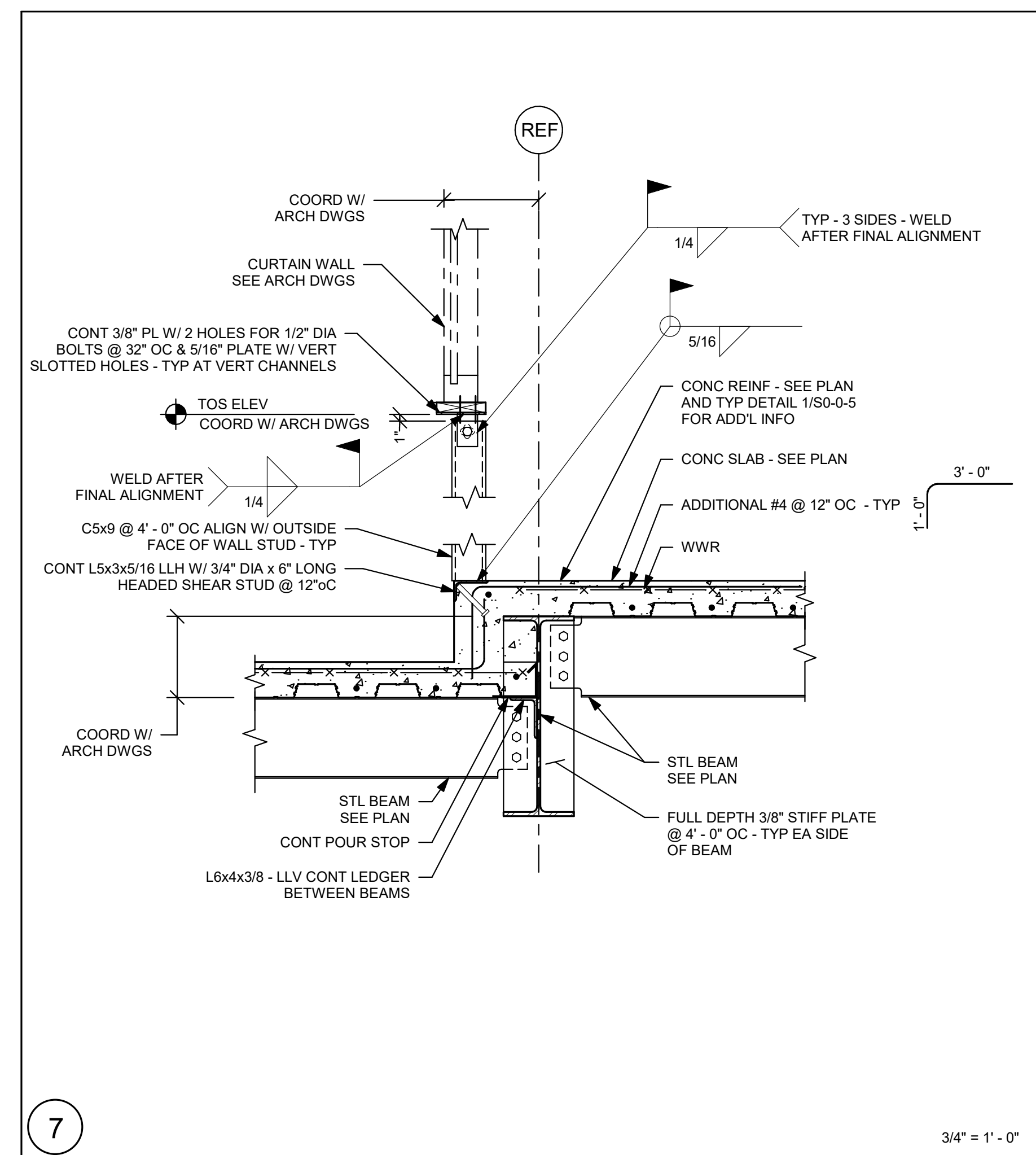
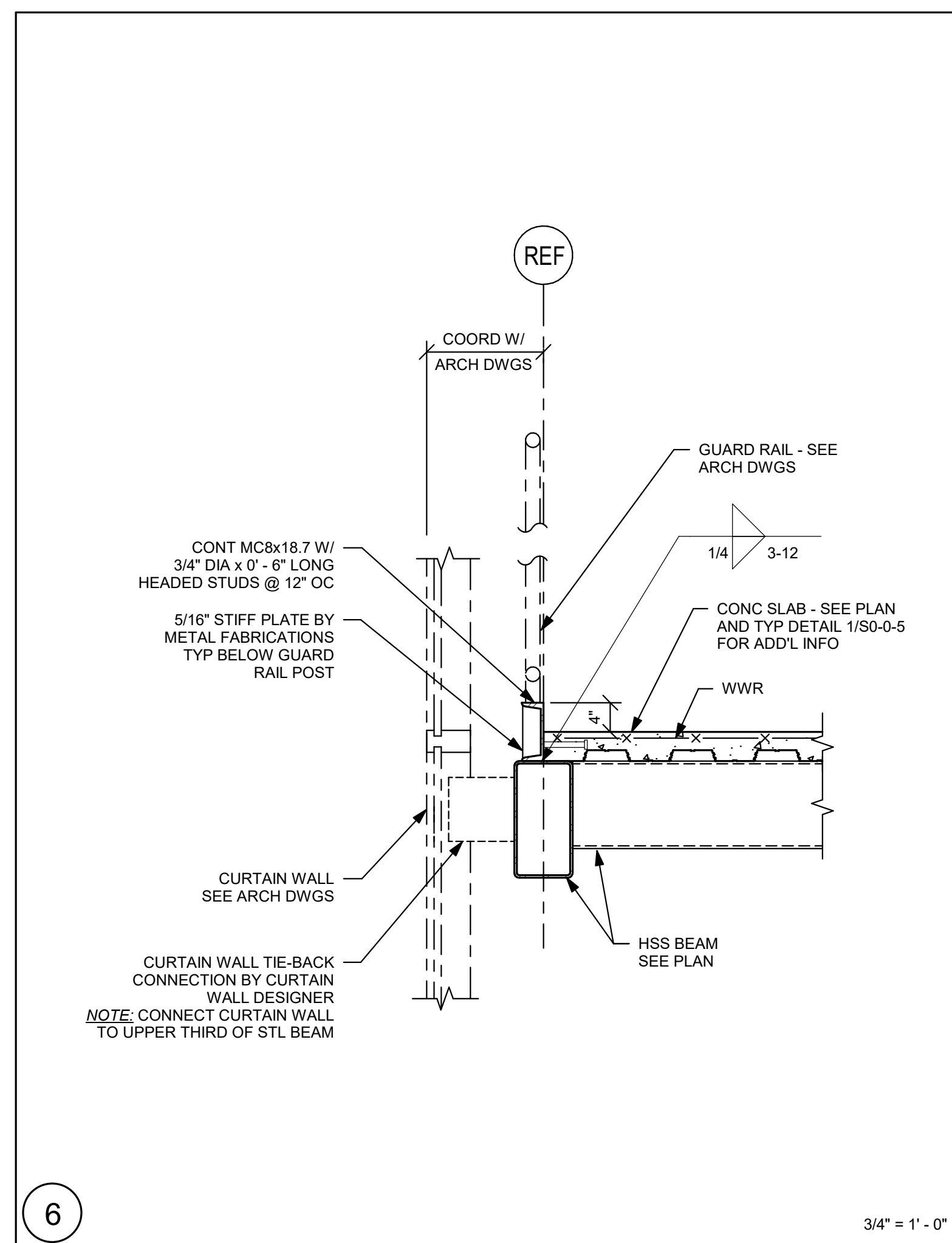
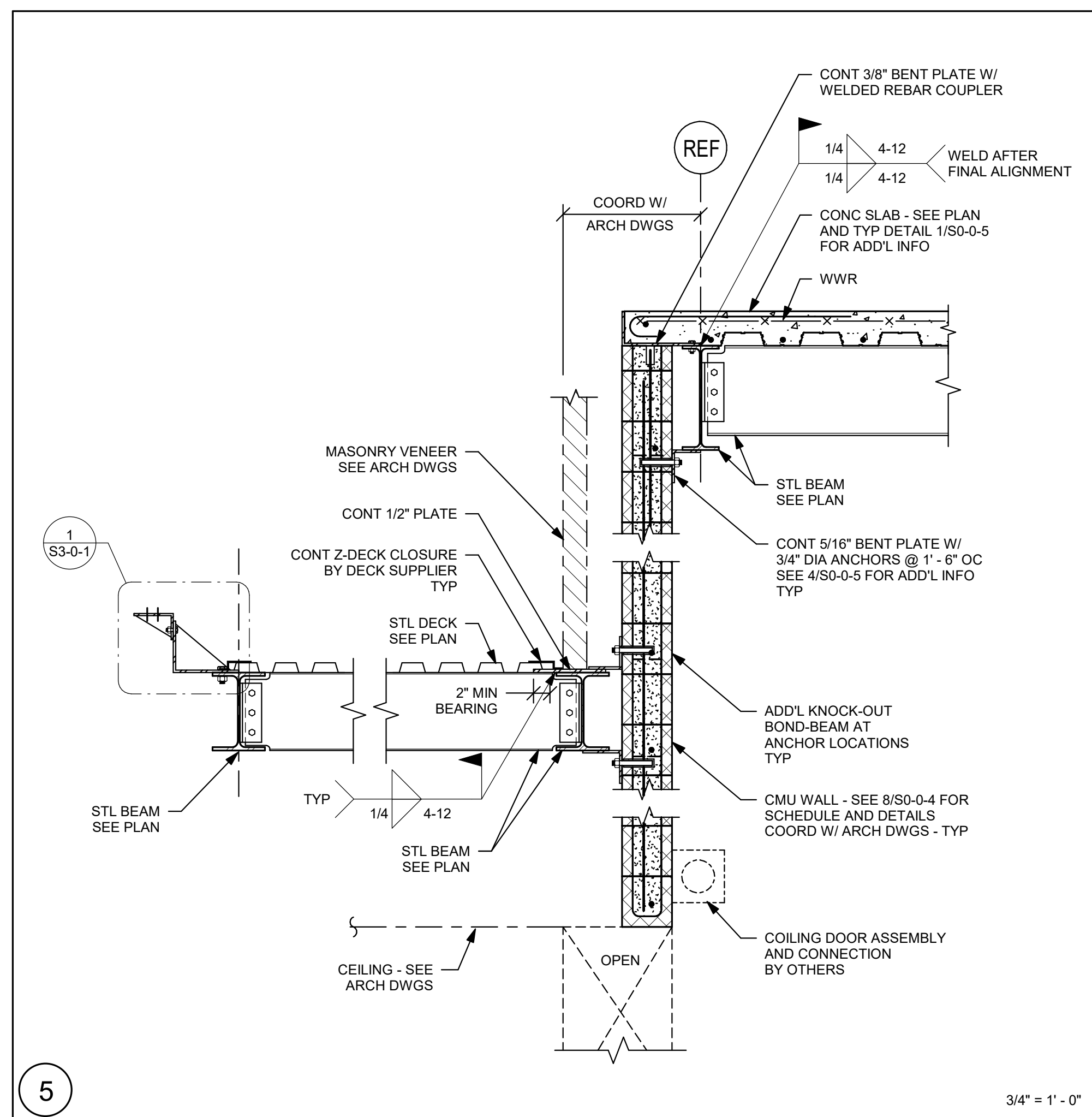
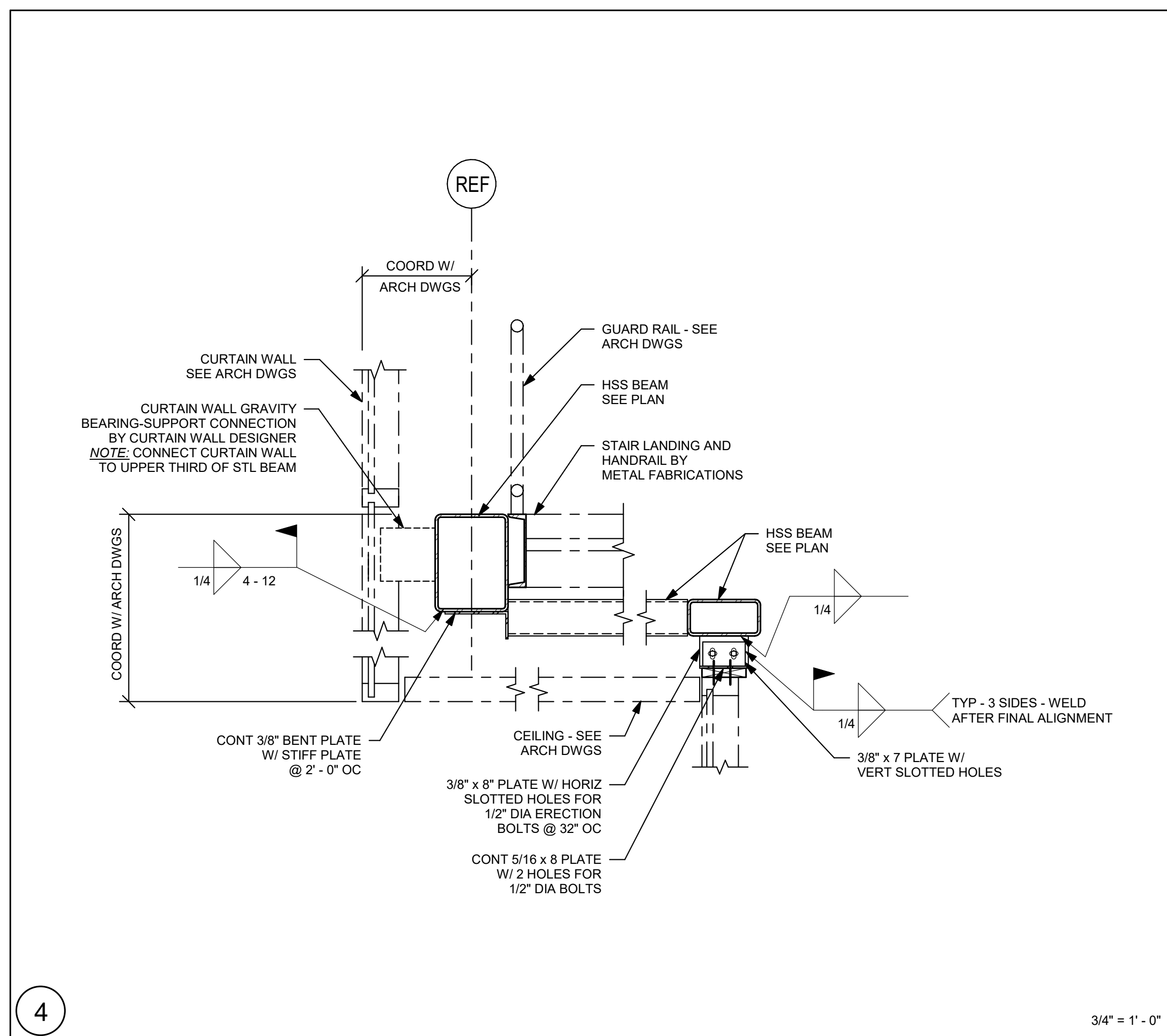
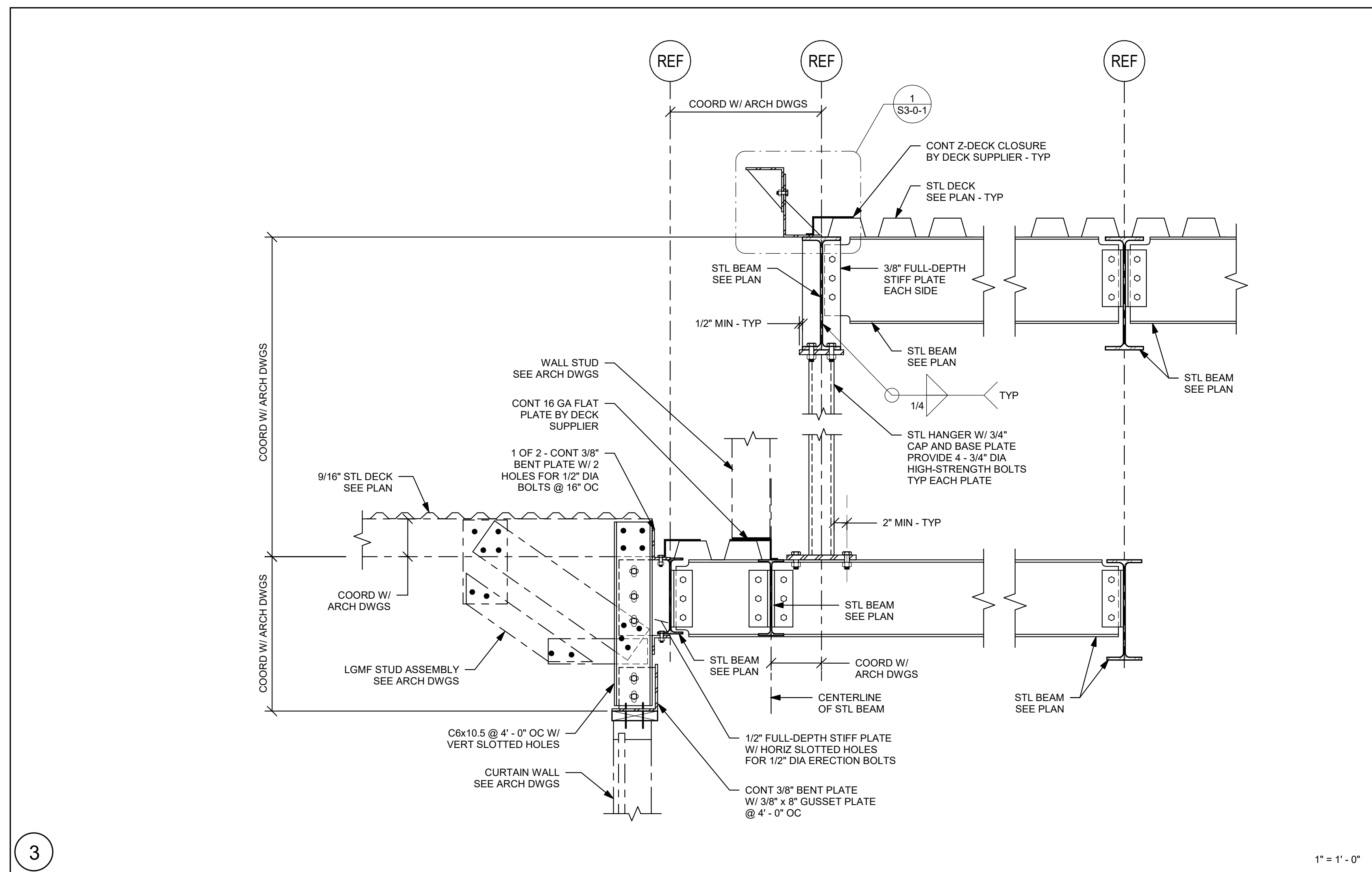
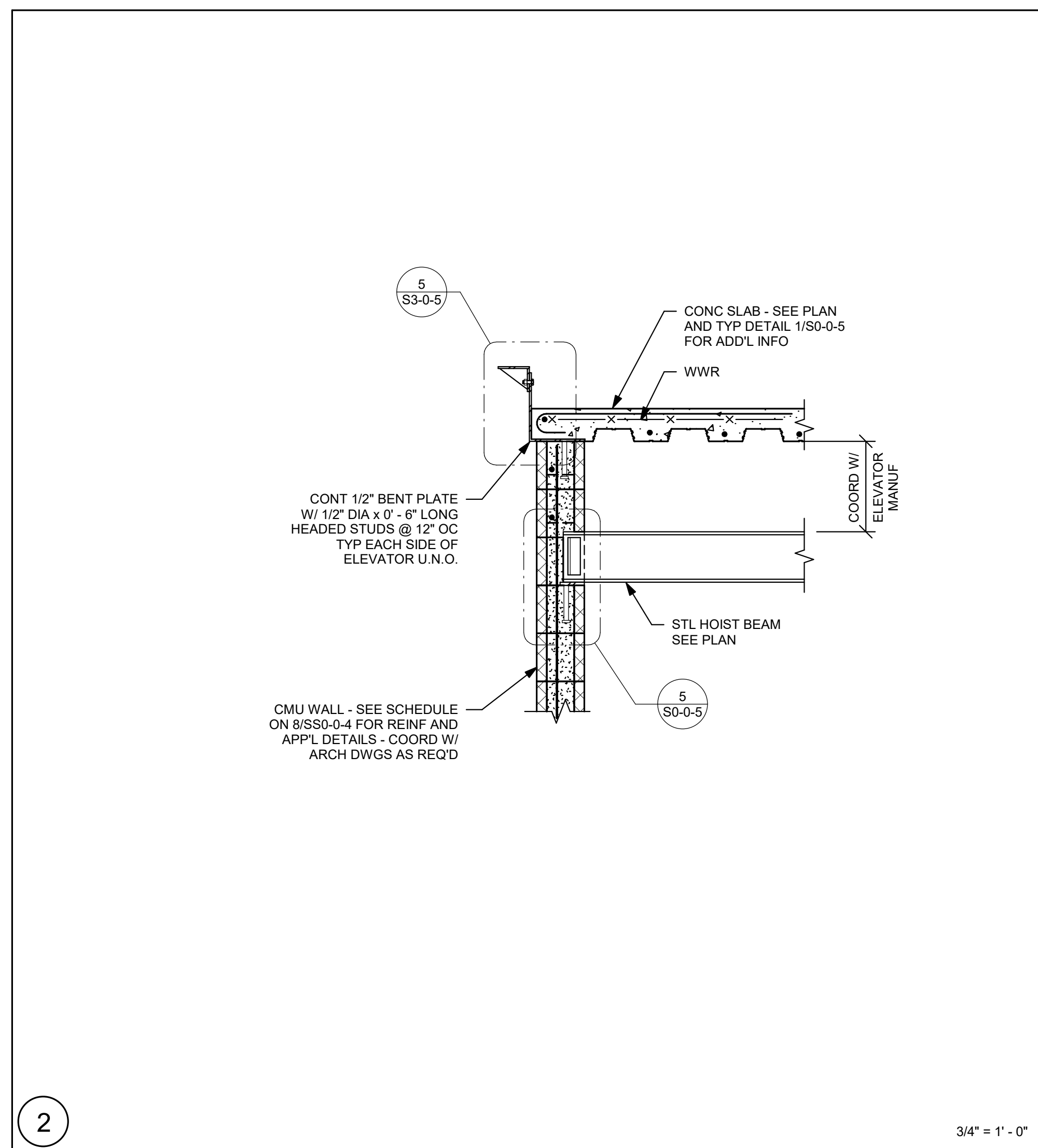
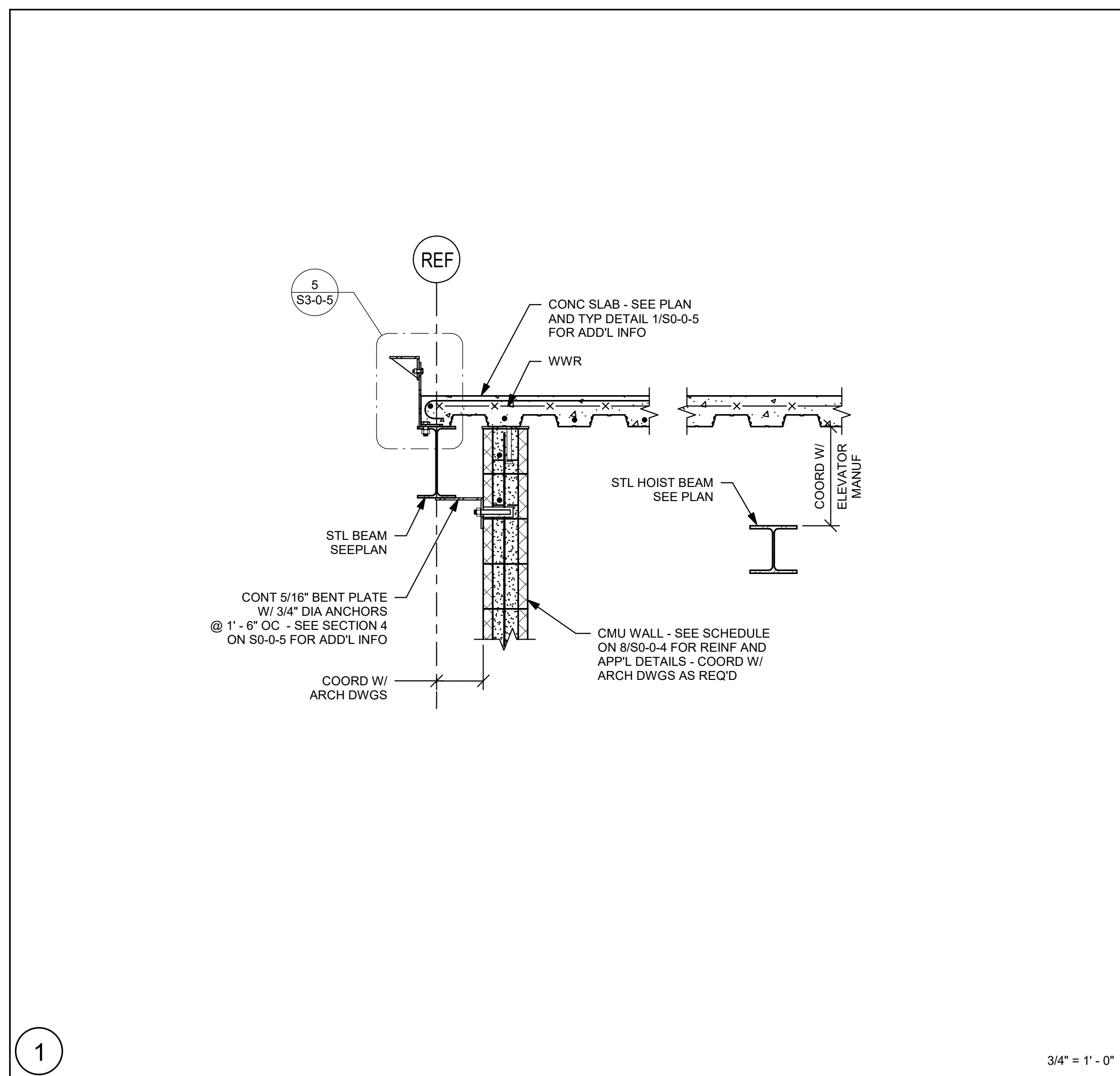
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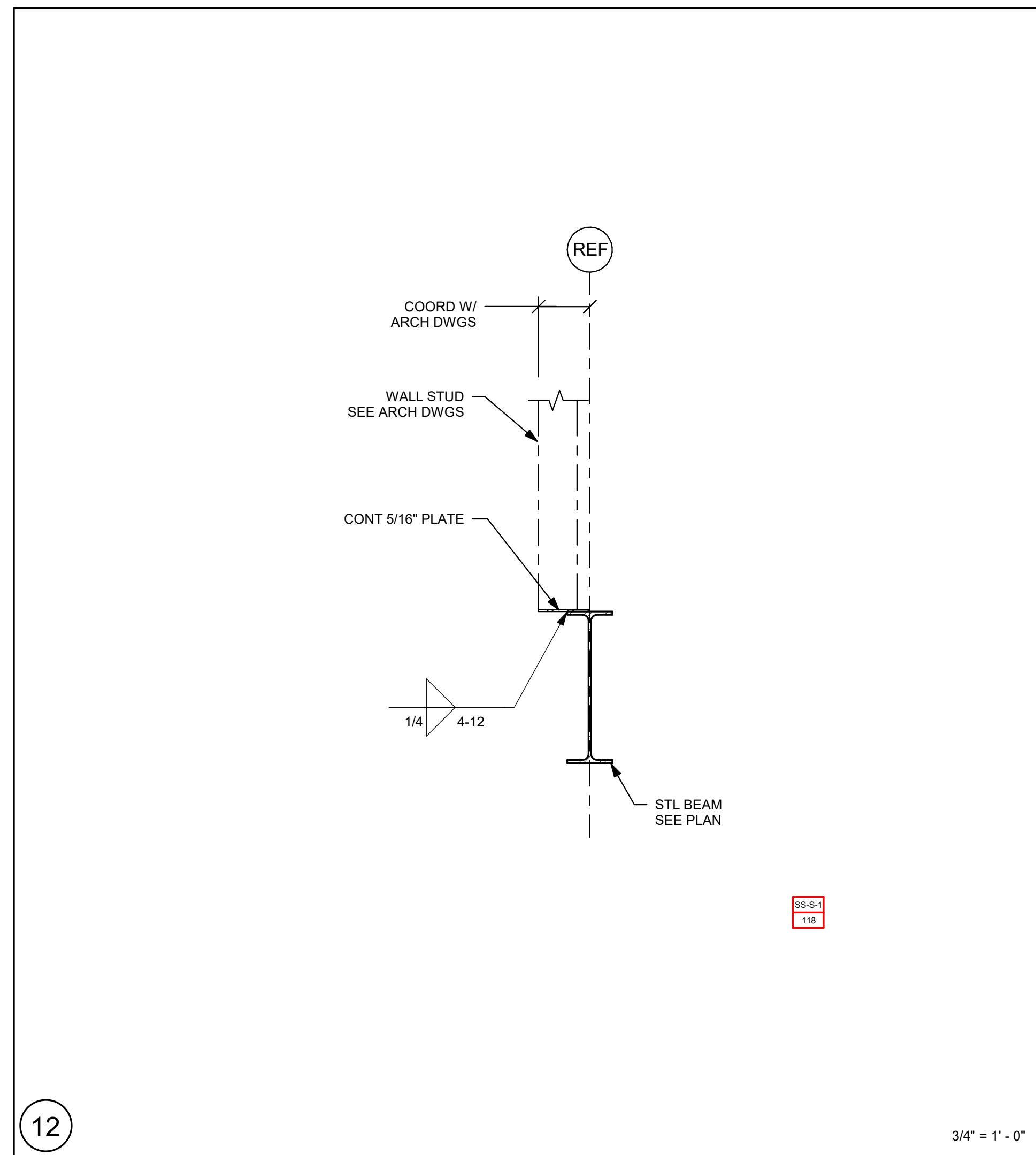
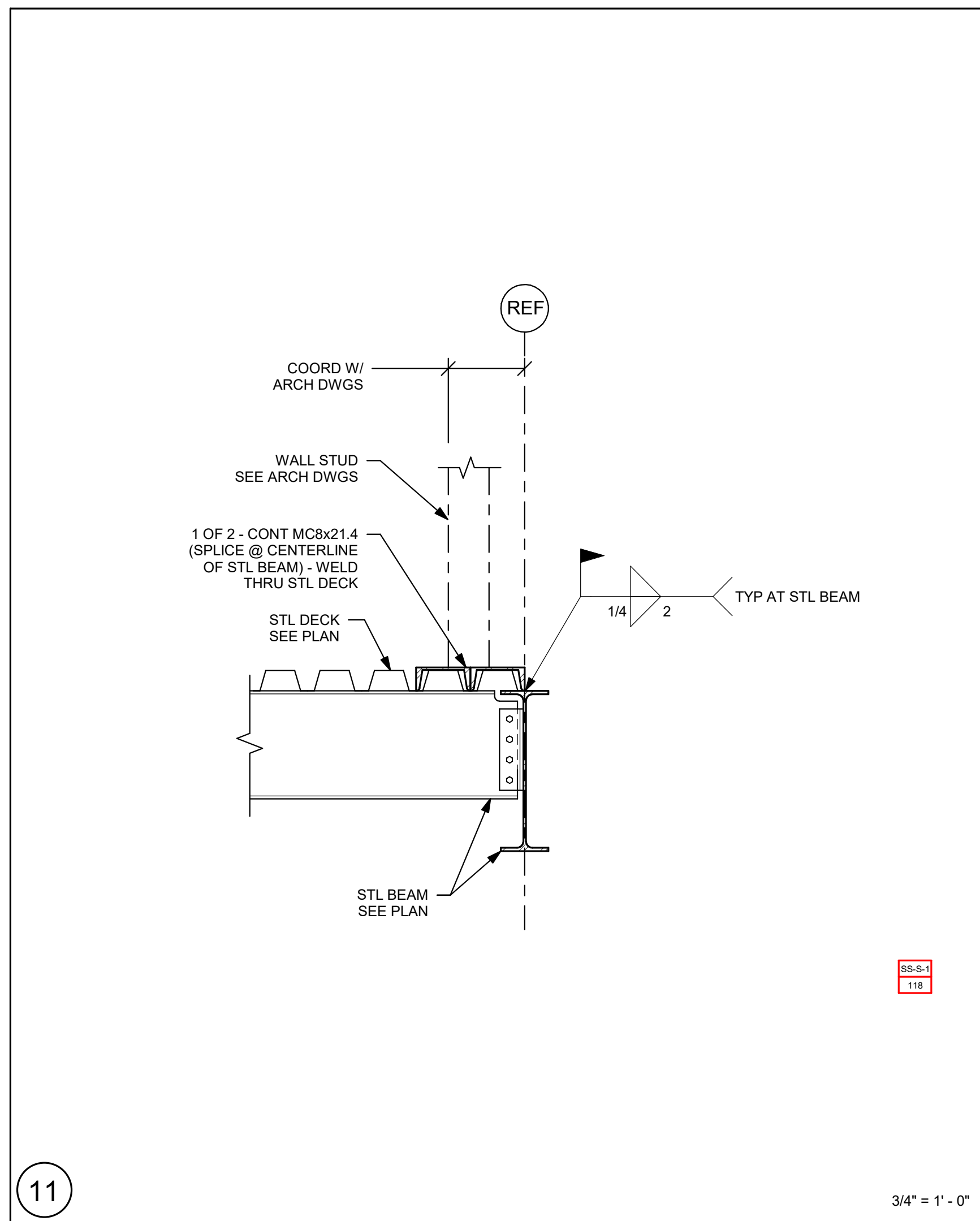
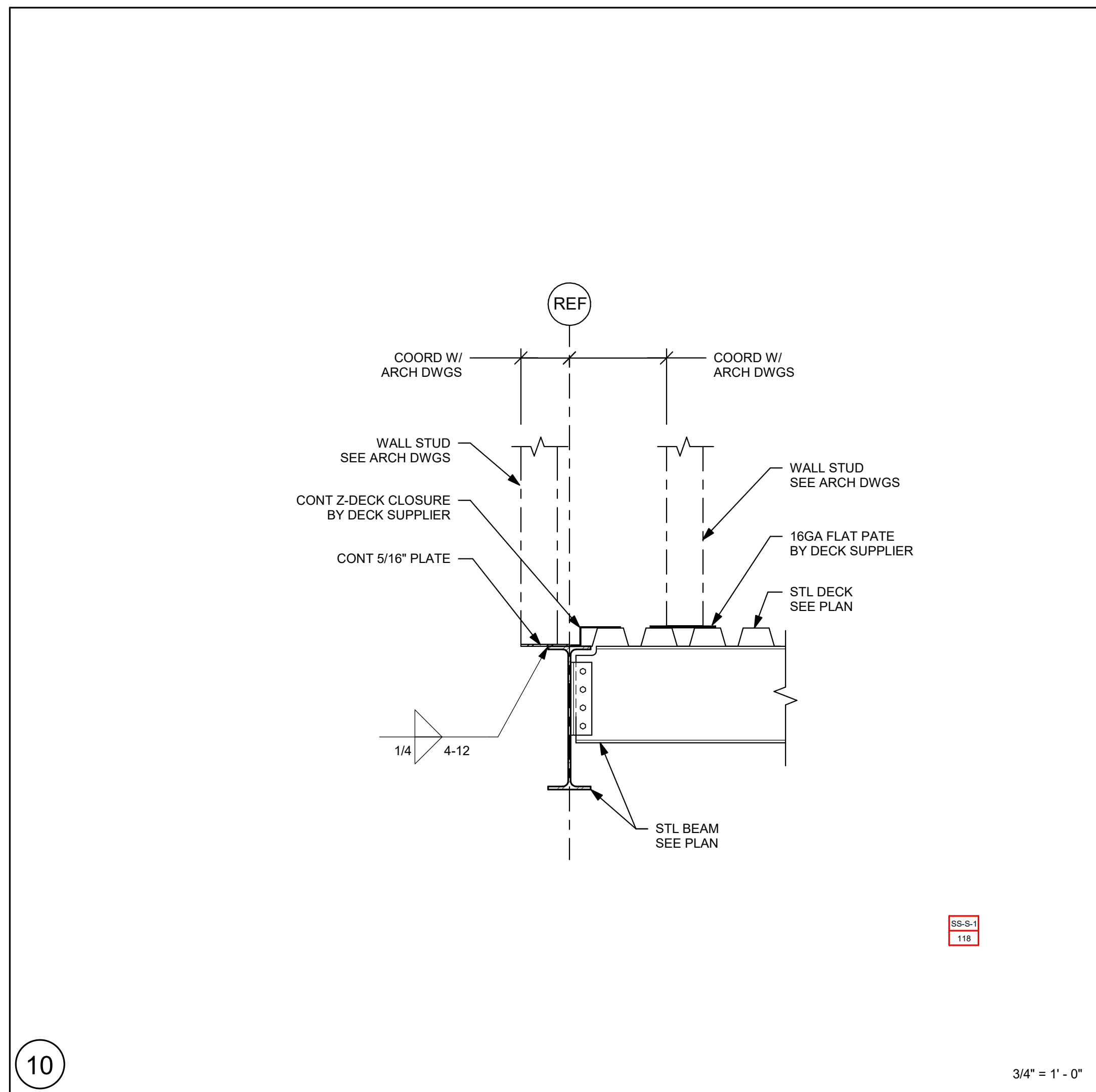
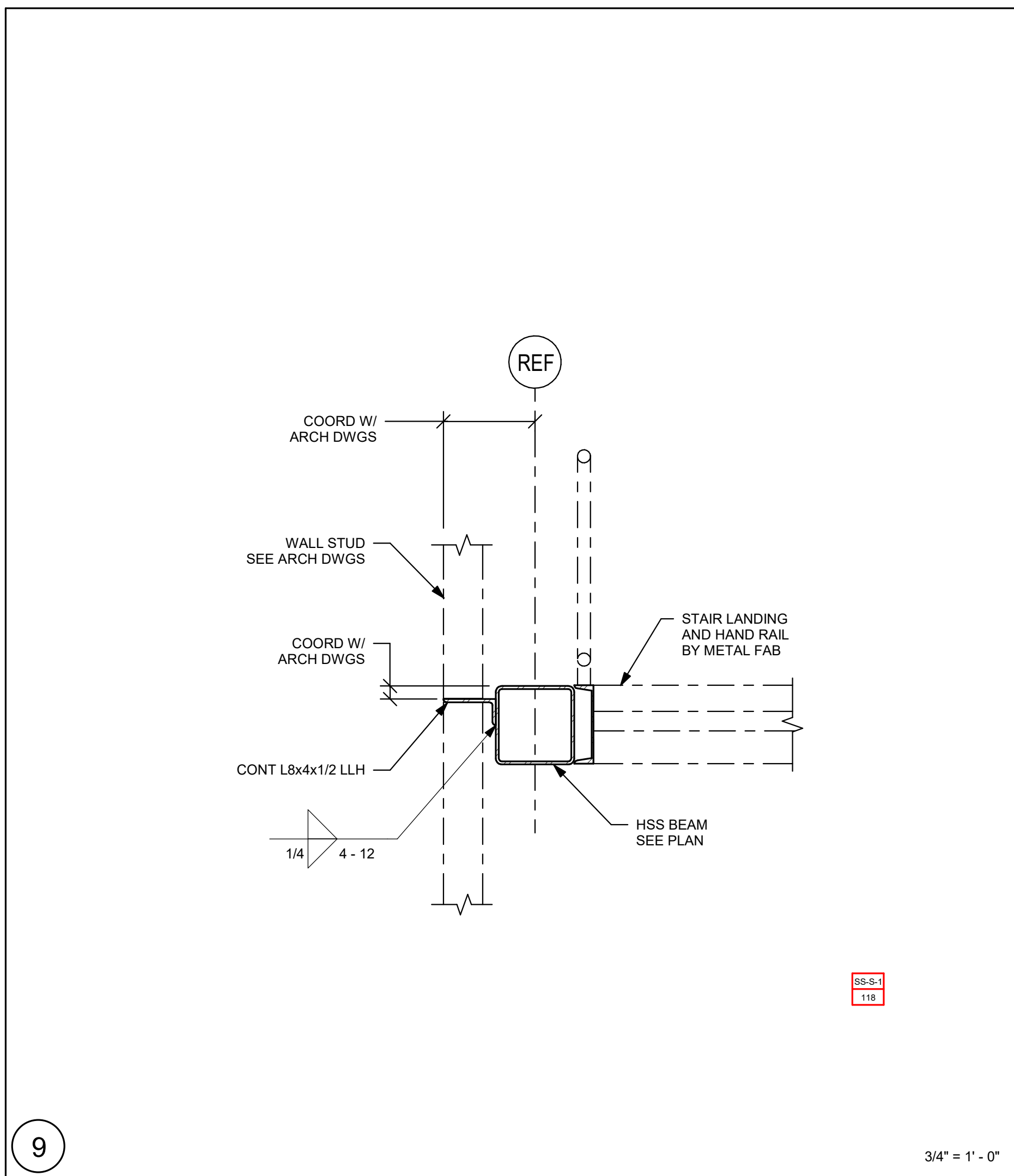
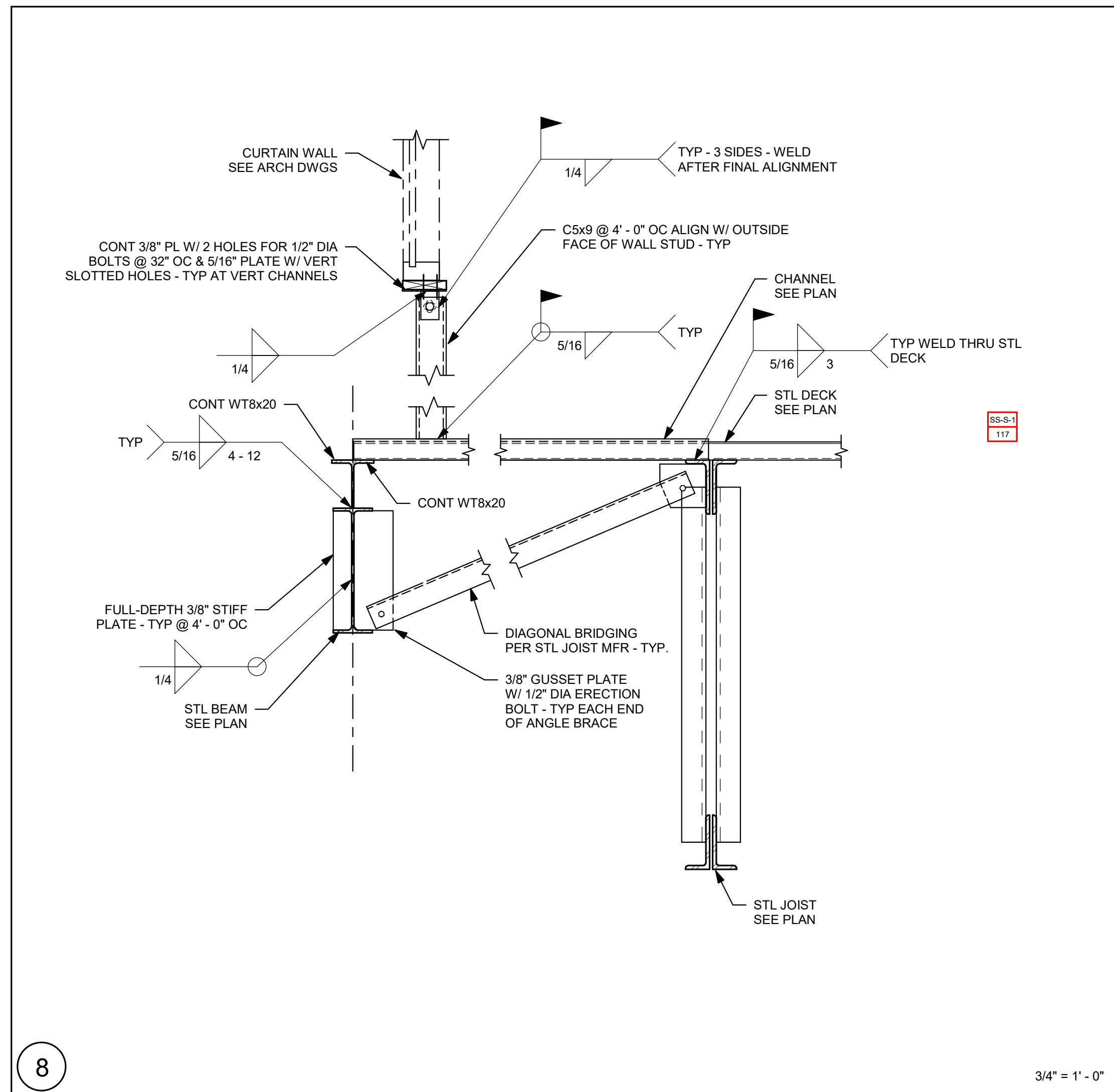
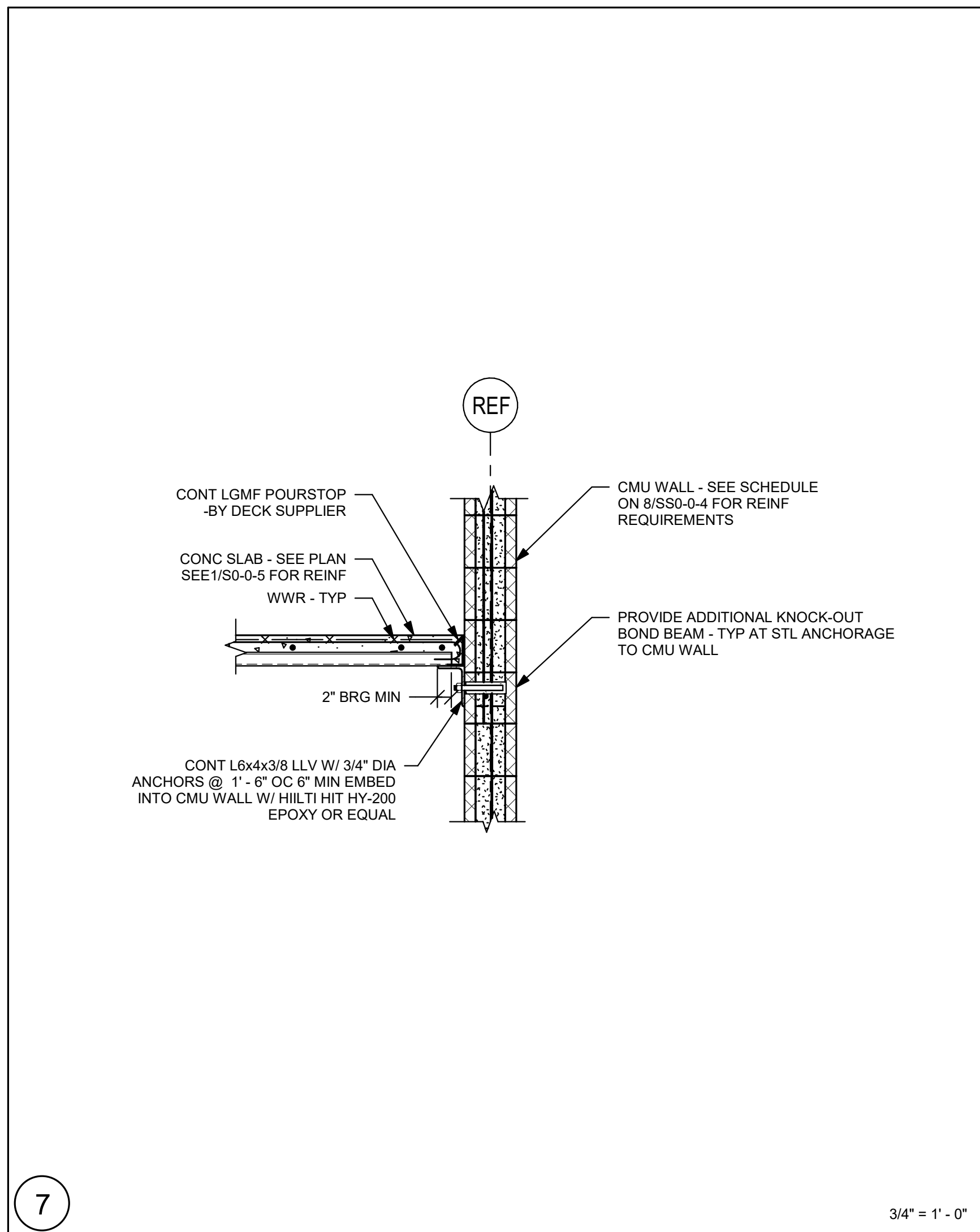
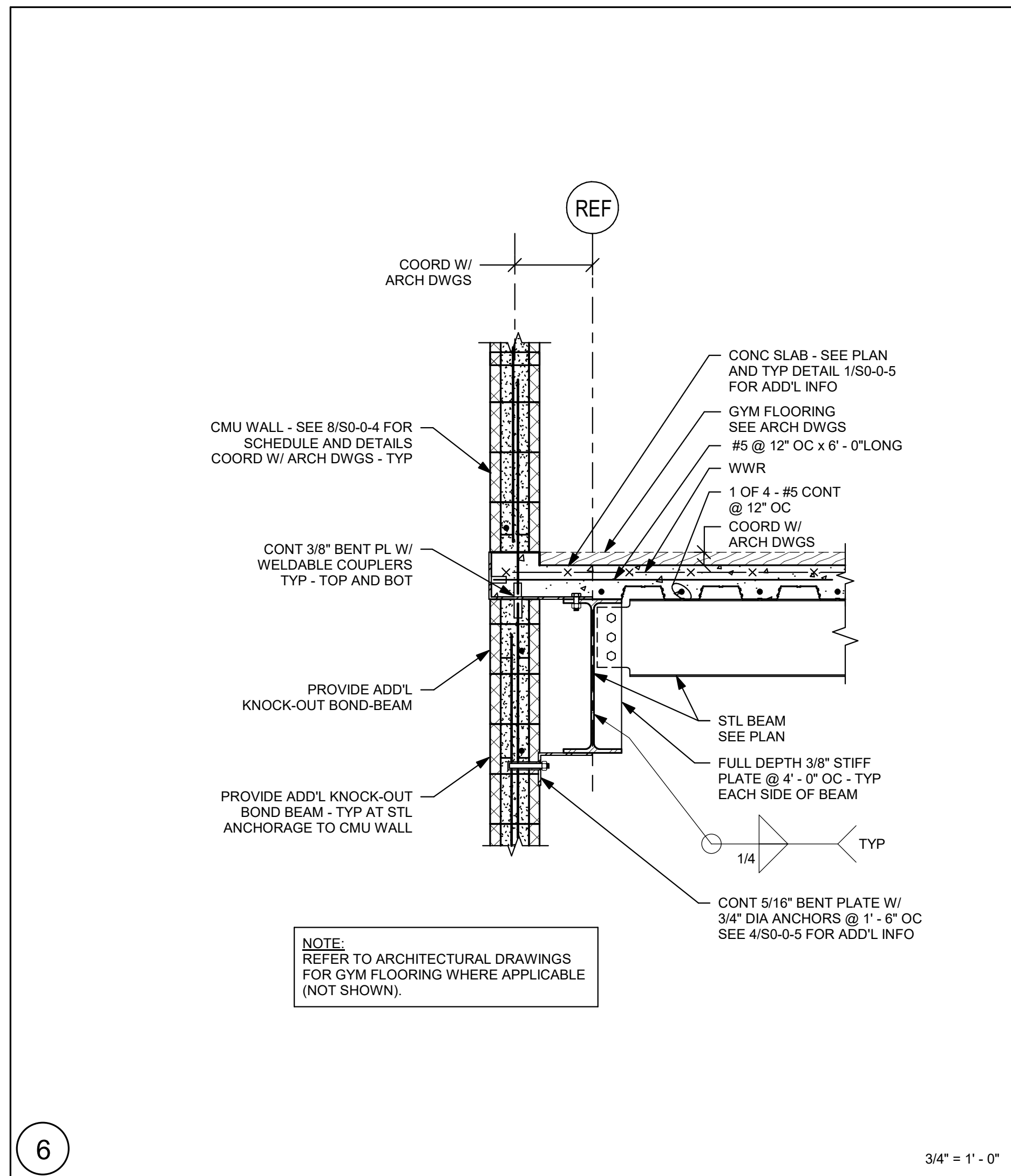
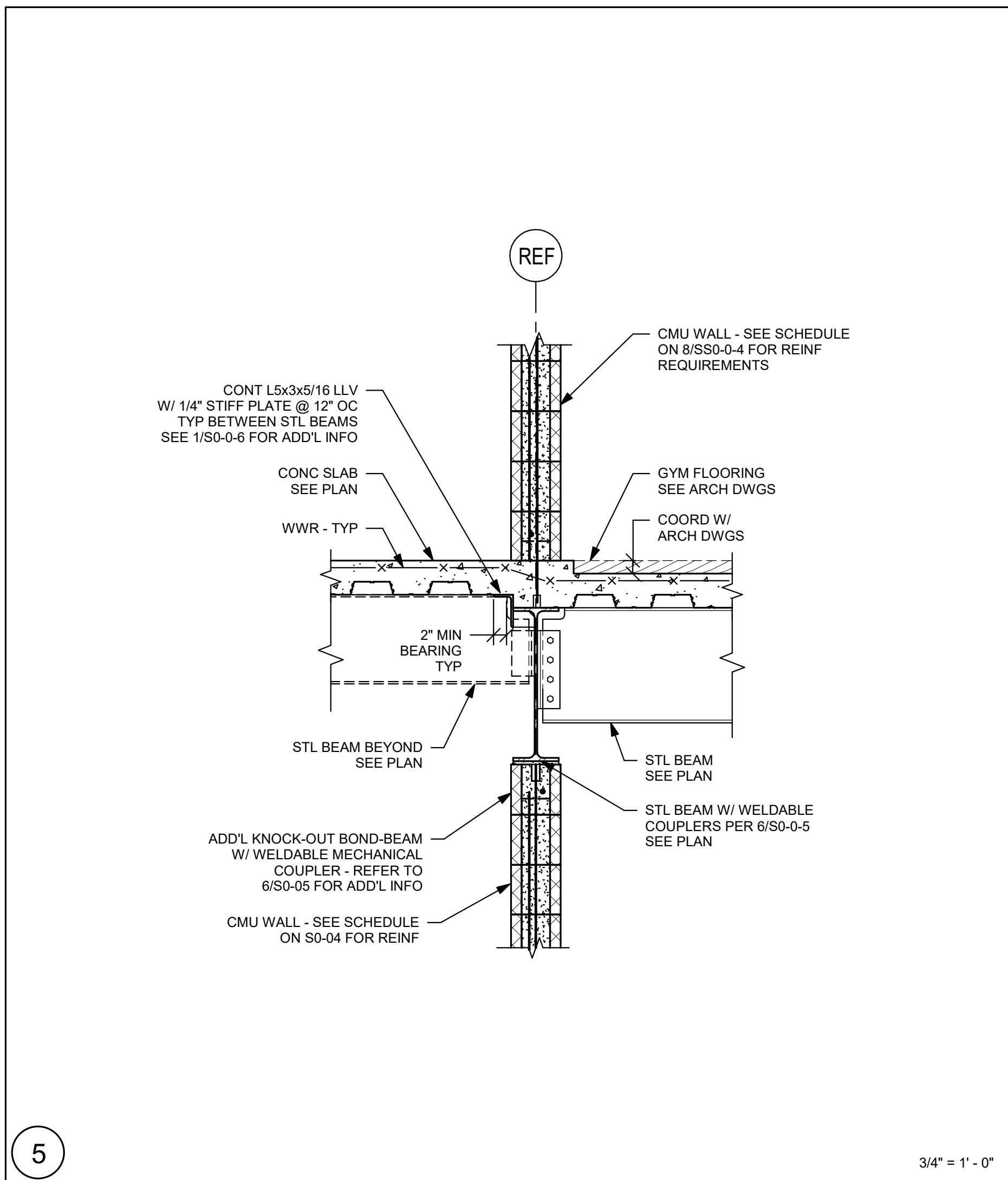
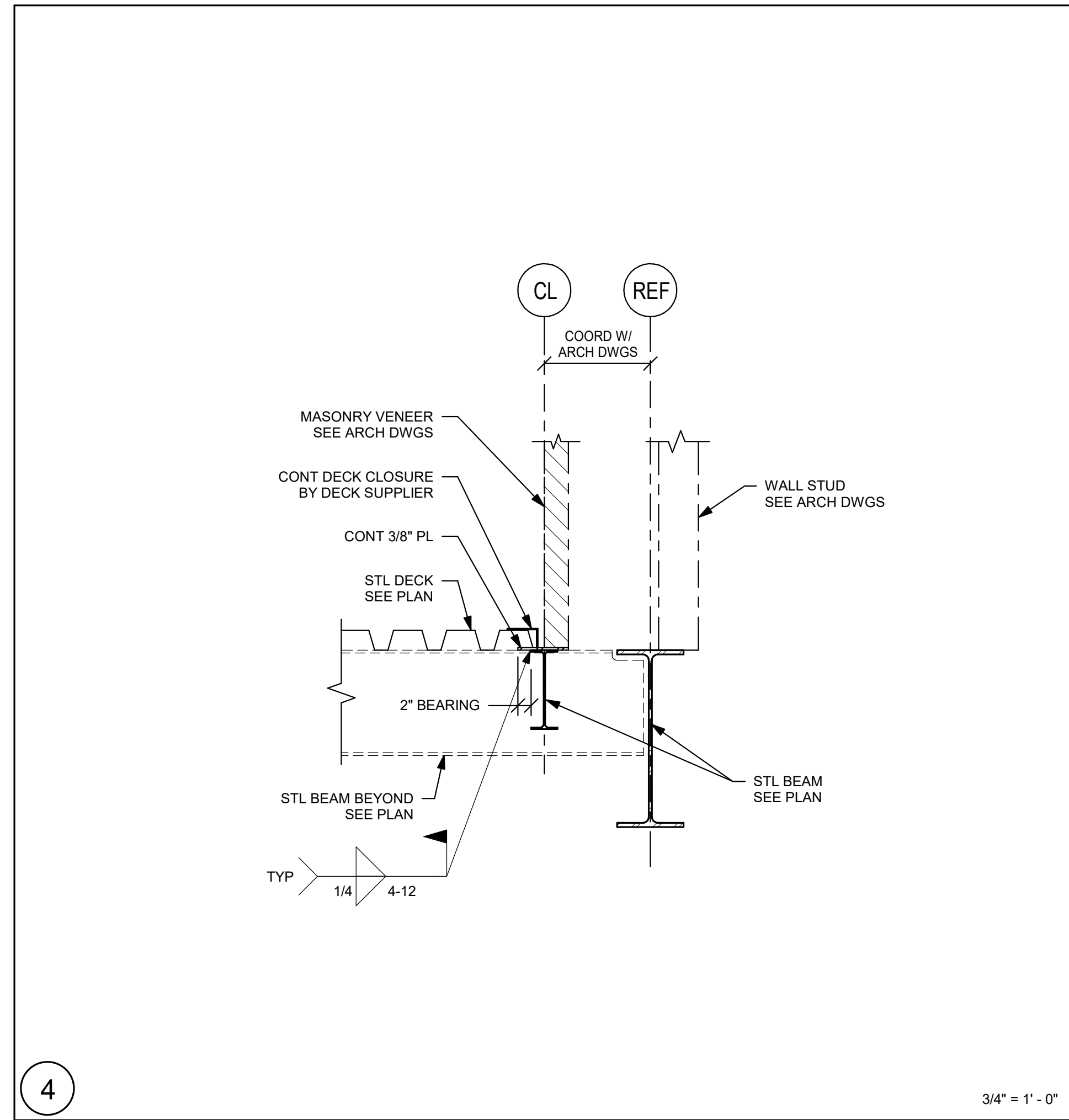
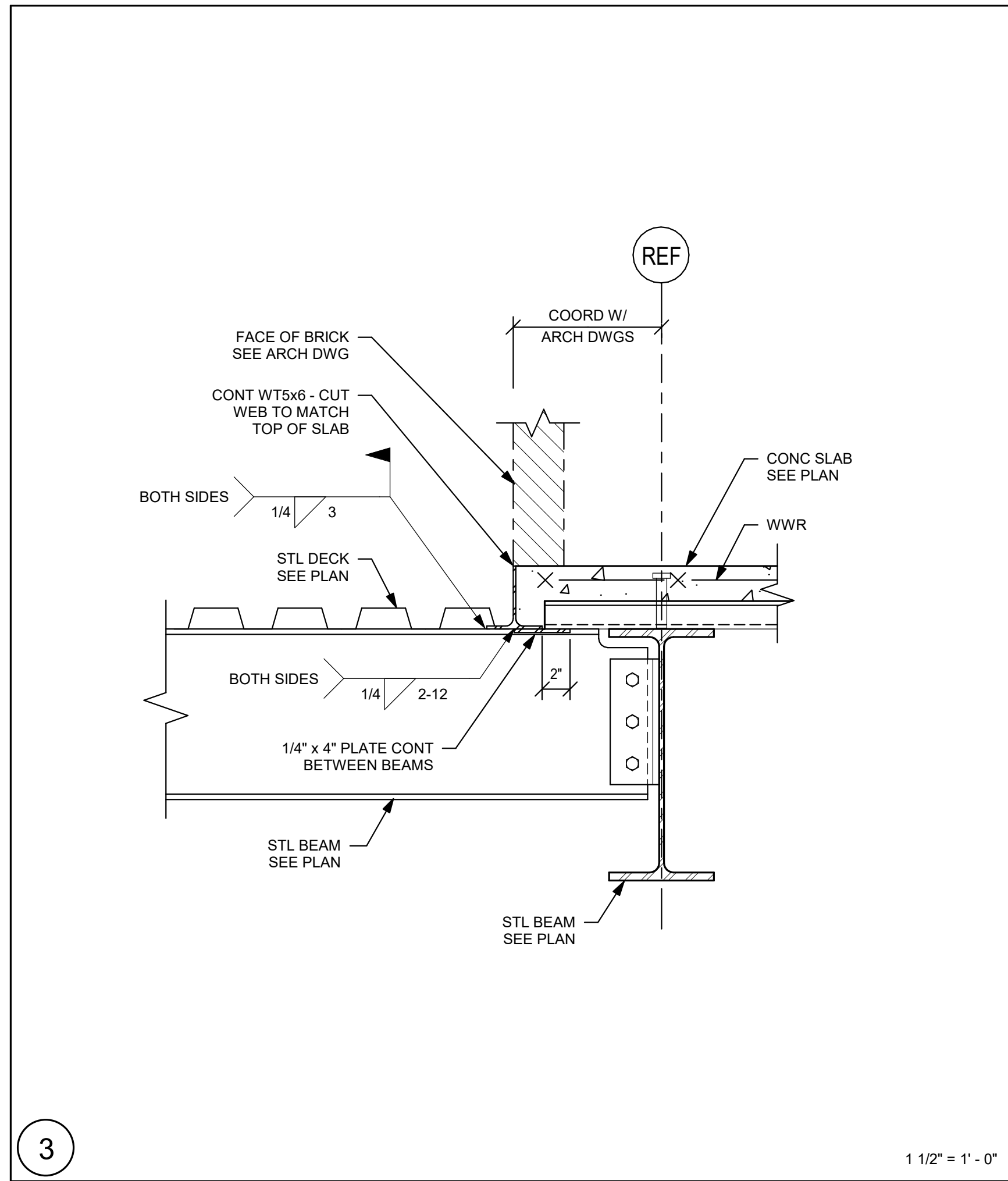
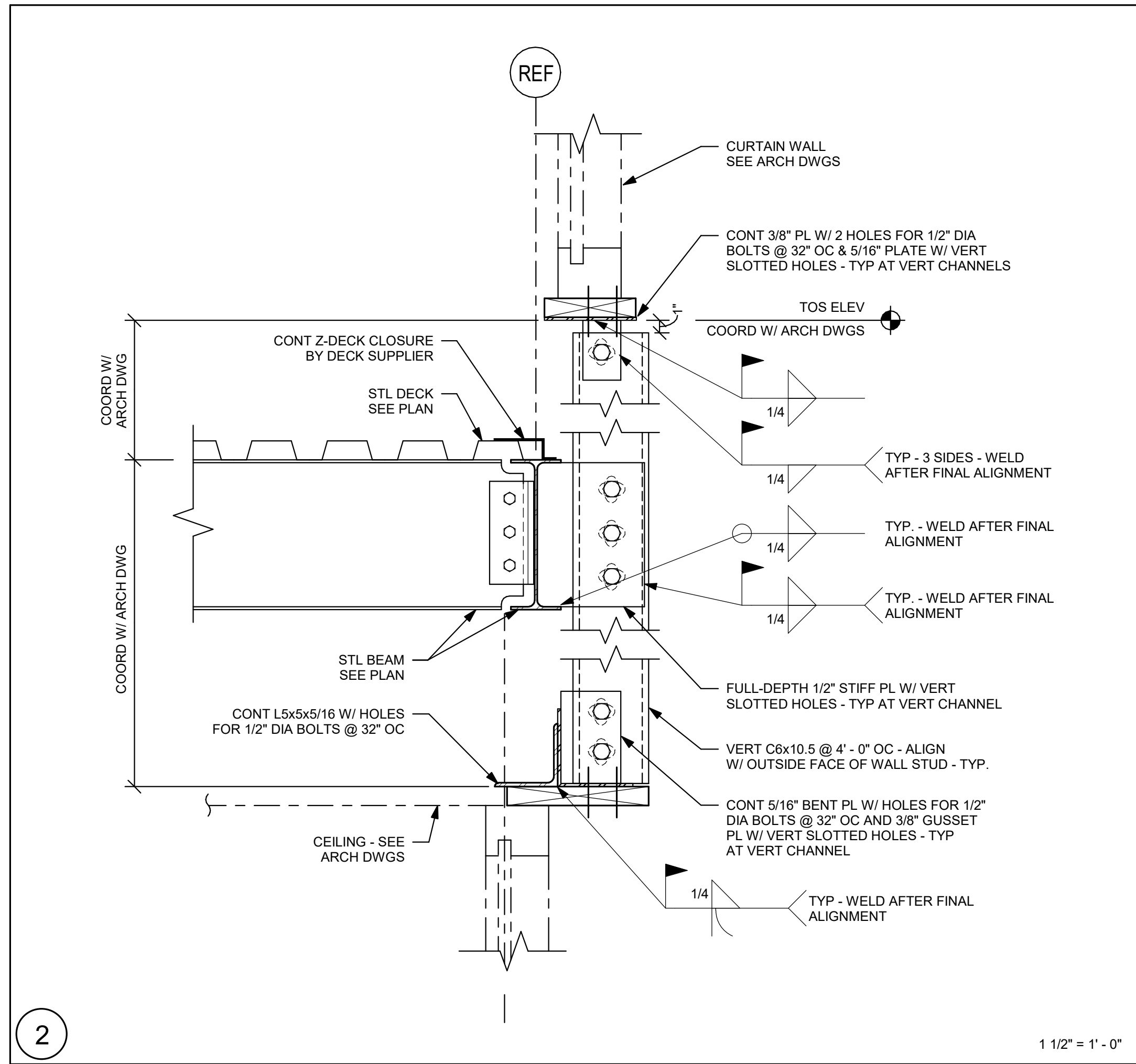
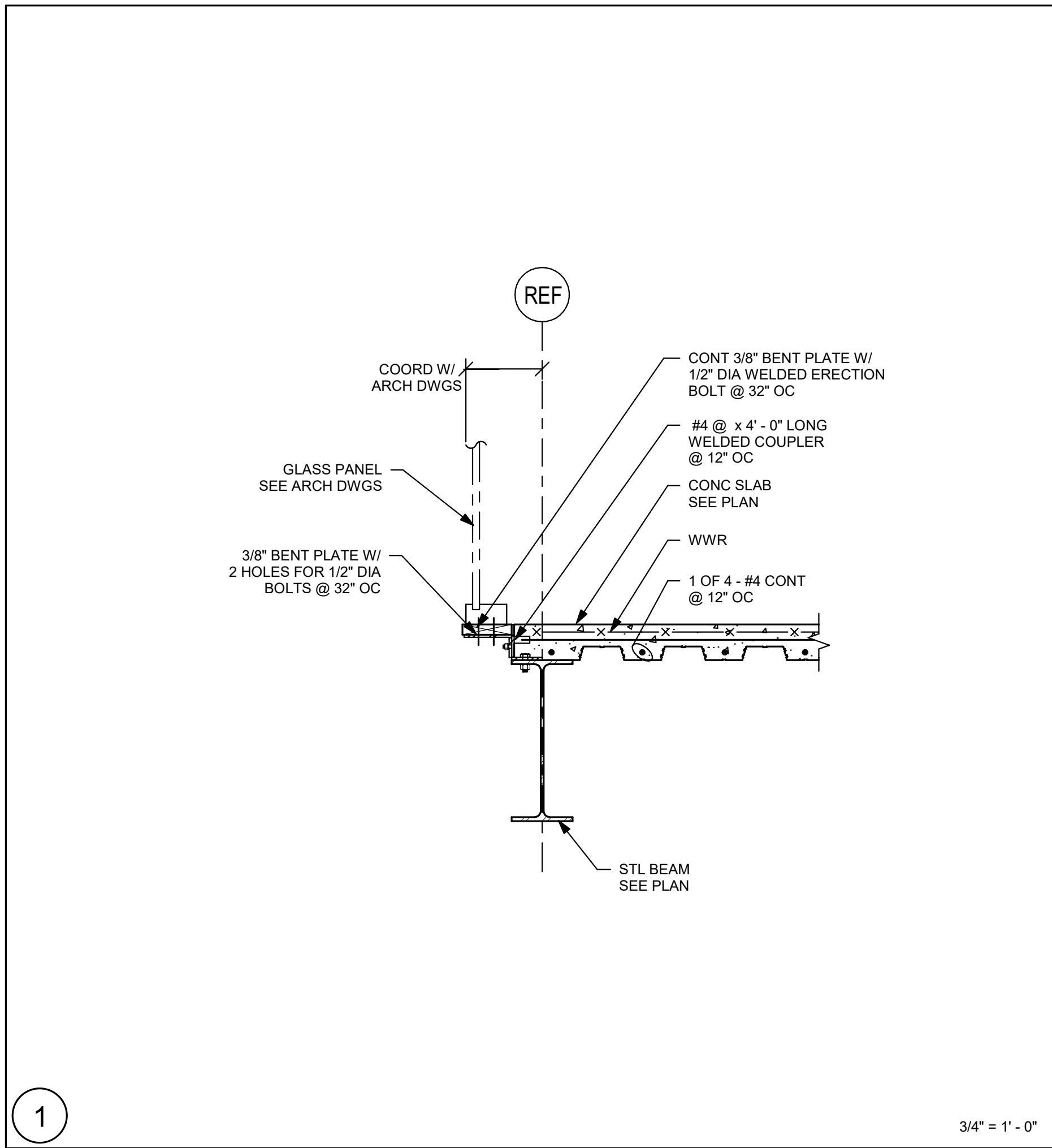
Job No.: 20202

Drawn By: EDG

Date: August 28th, 2023

S3-0-8





**NORTHEAST
METRO TECH**

100 Hemlock Rd.
Wakefield, MA 01880

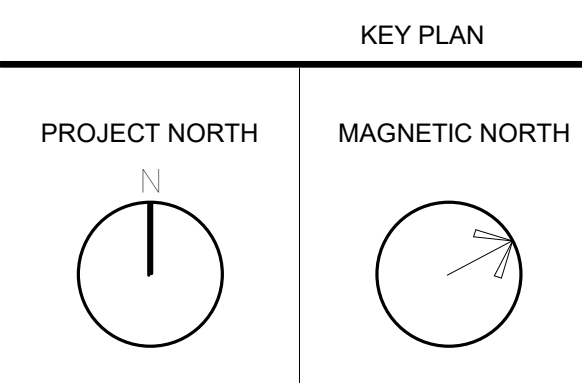
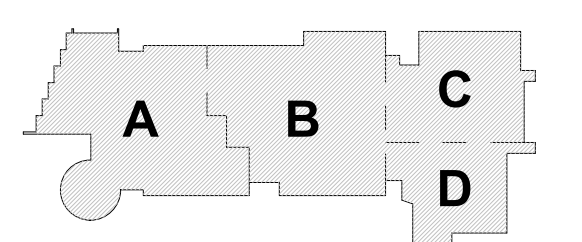
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03/31/2023	EARLY STRUCTURAL BID PACKAGE
REVISION LIST	
SS-S-1	4/14/2023 STRUCTURAL STEEL ADDENDUM 1

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BID SET

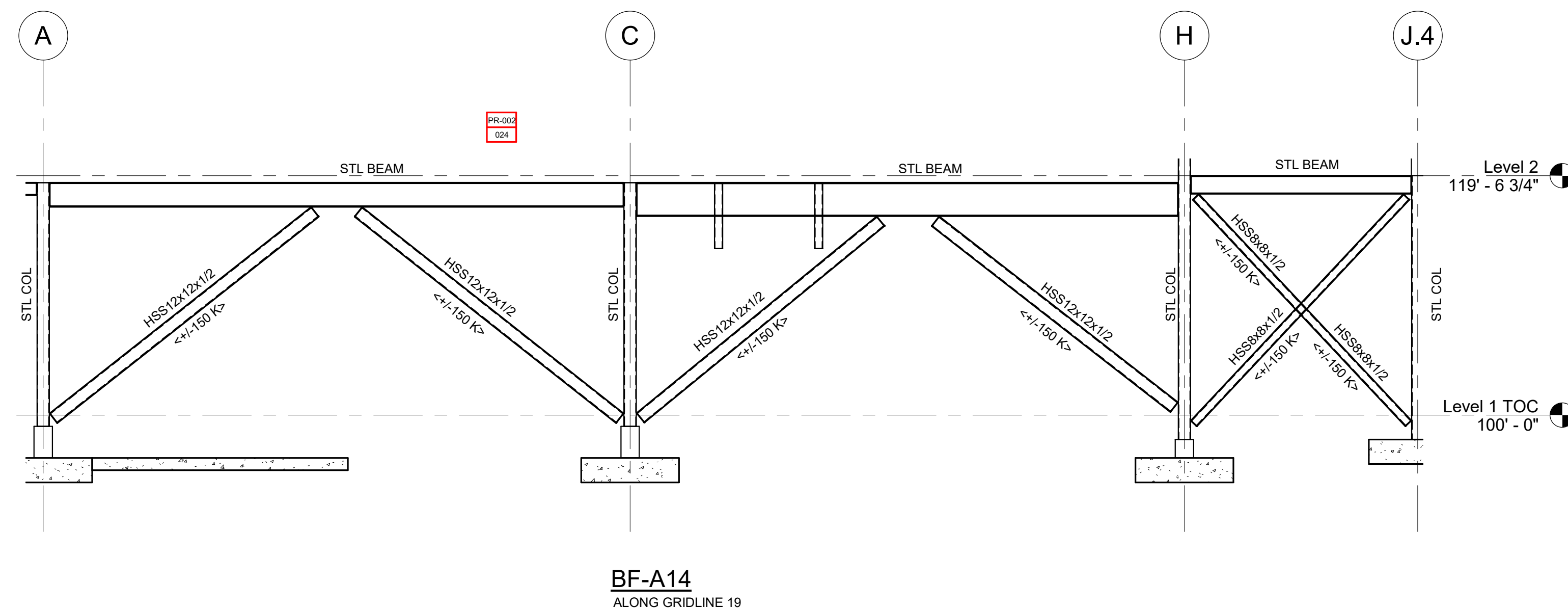
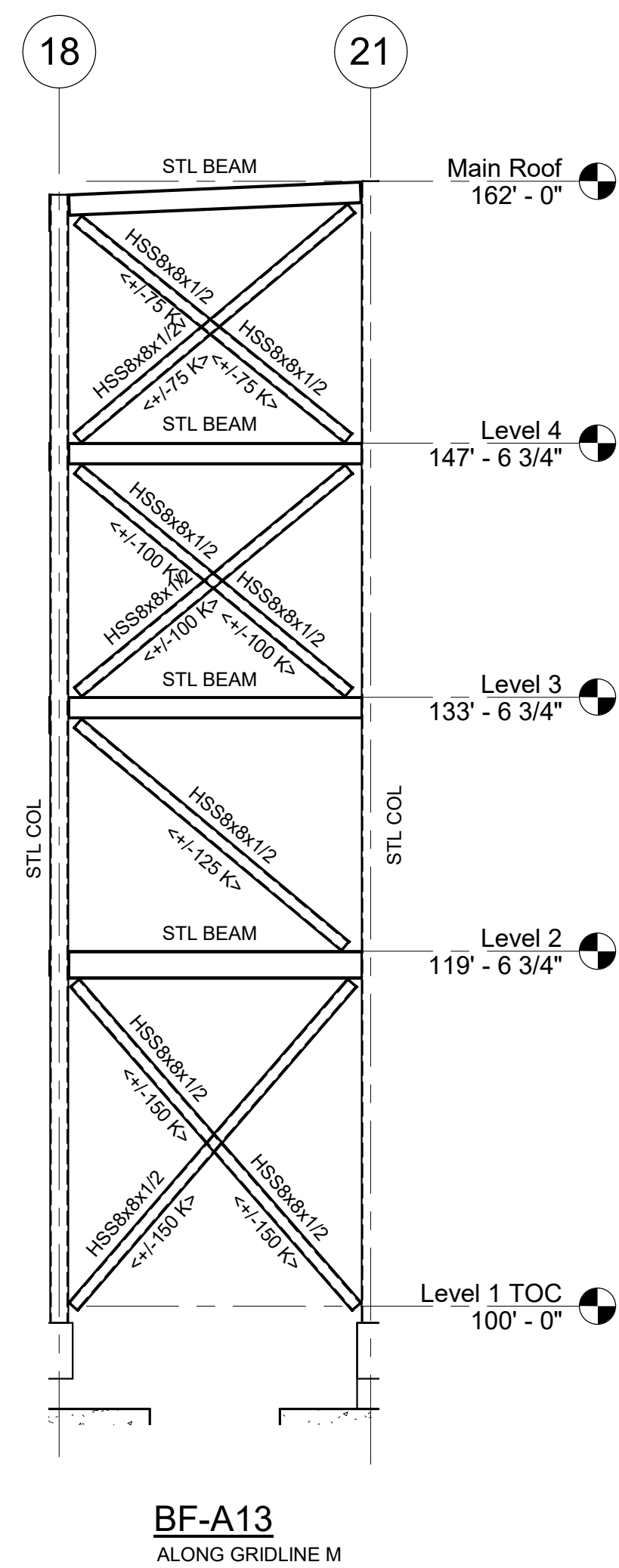
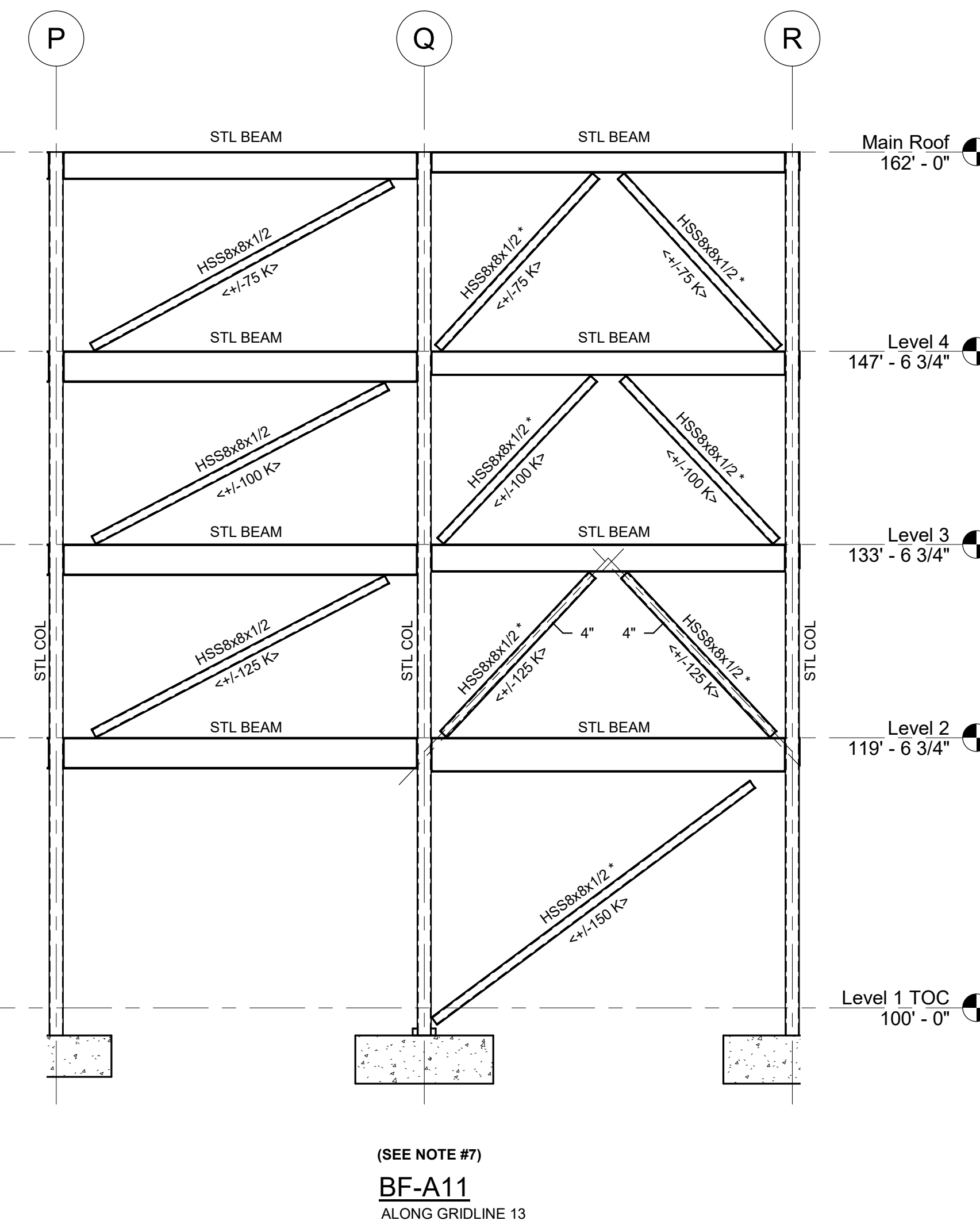
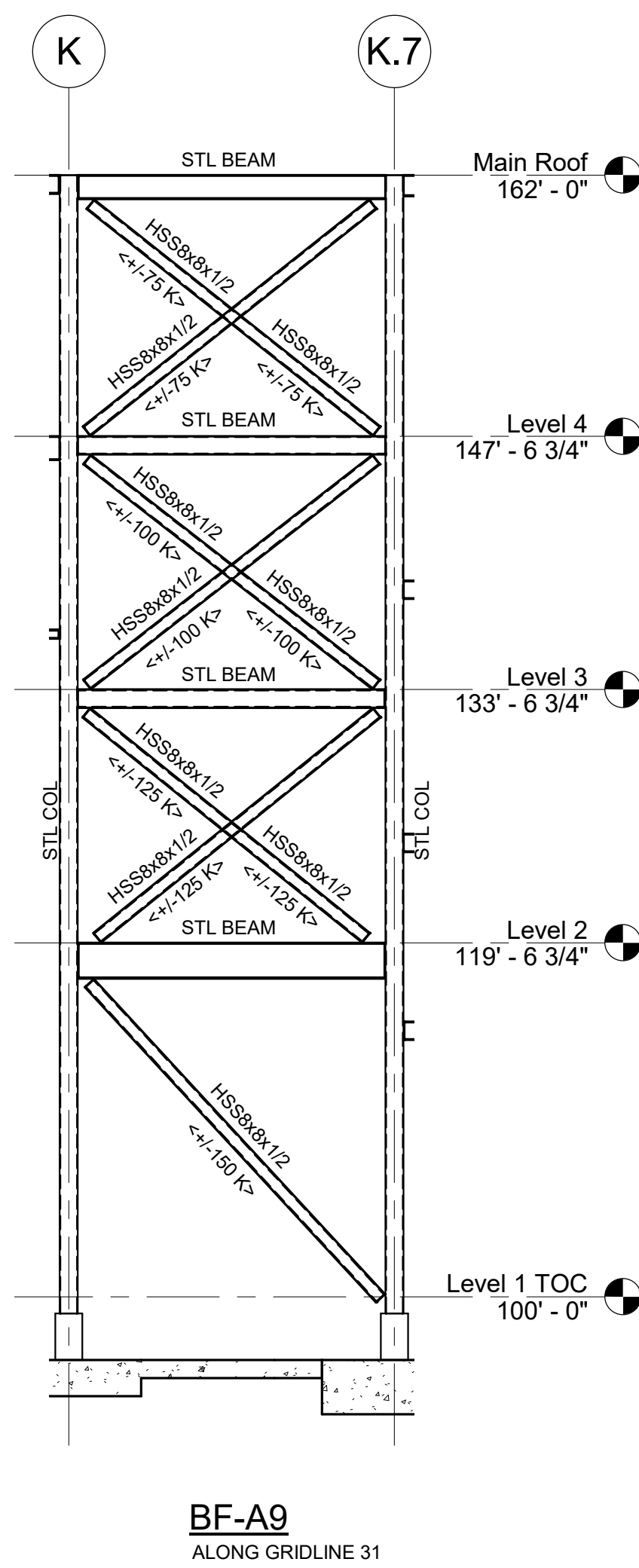
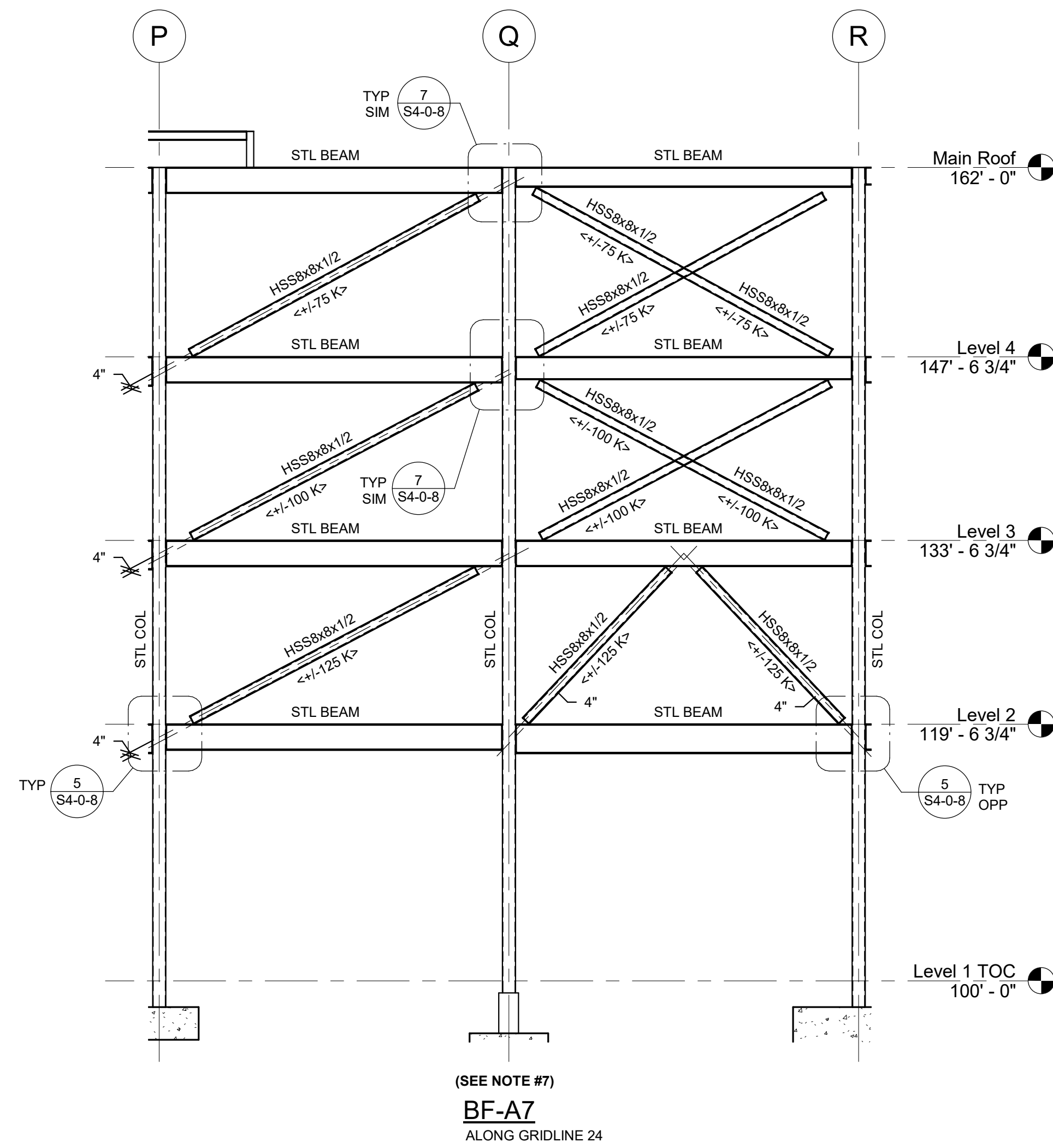
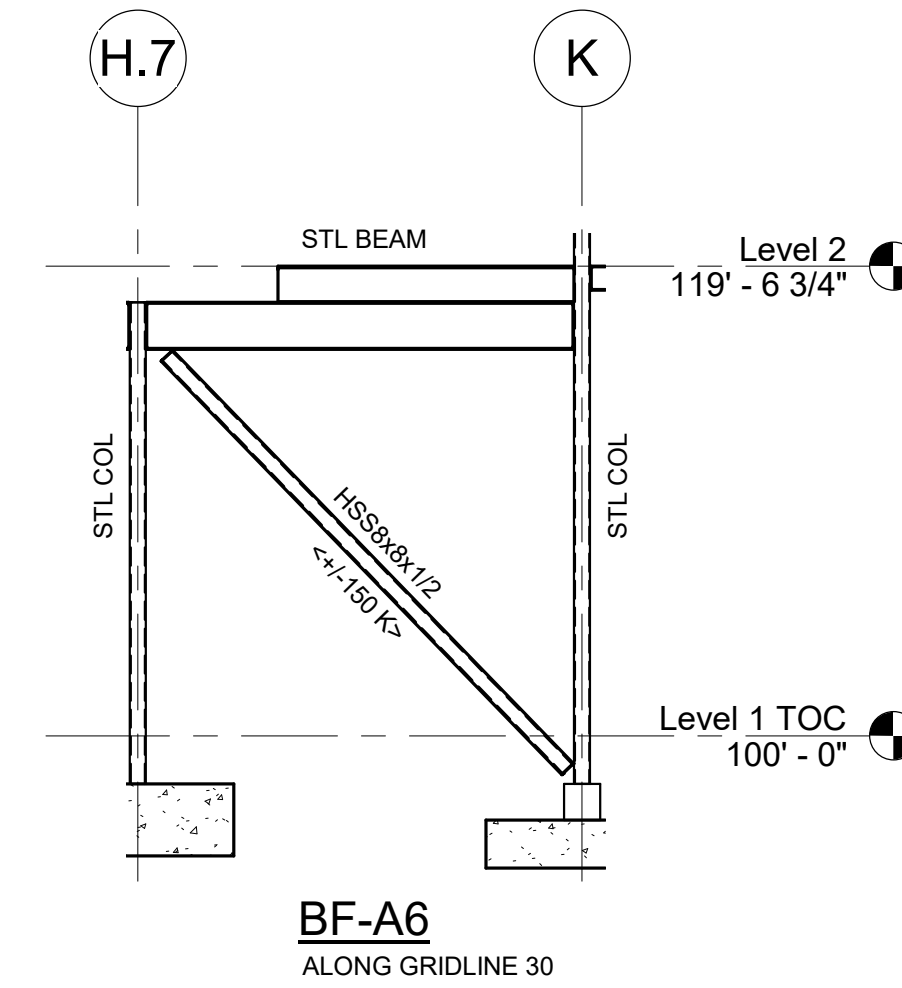
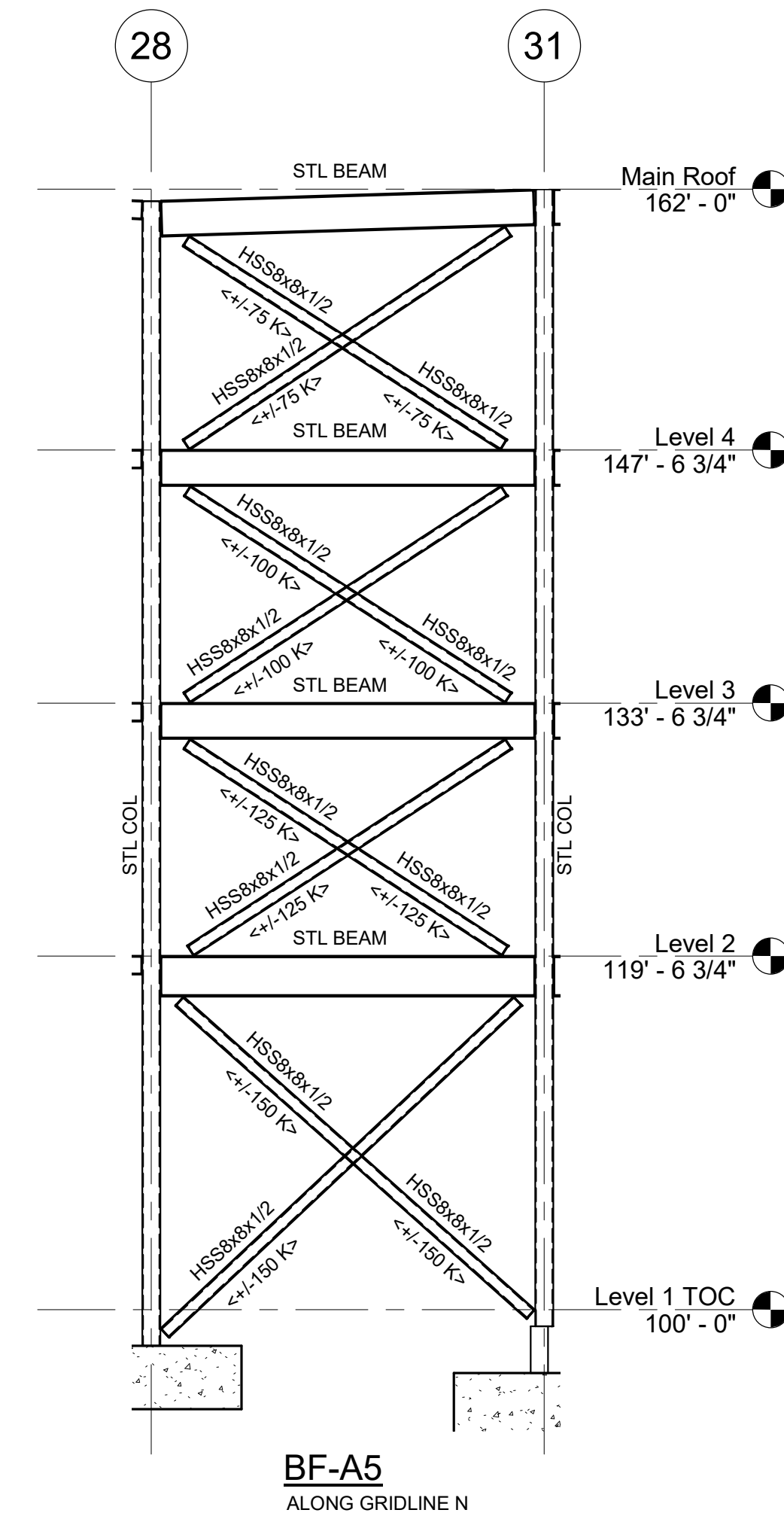
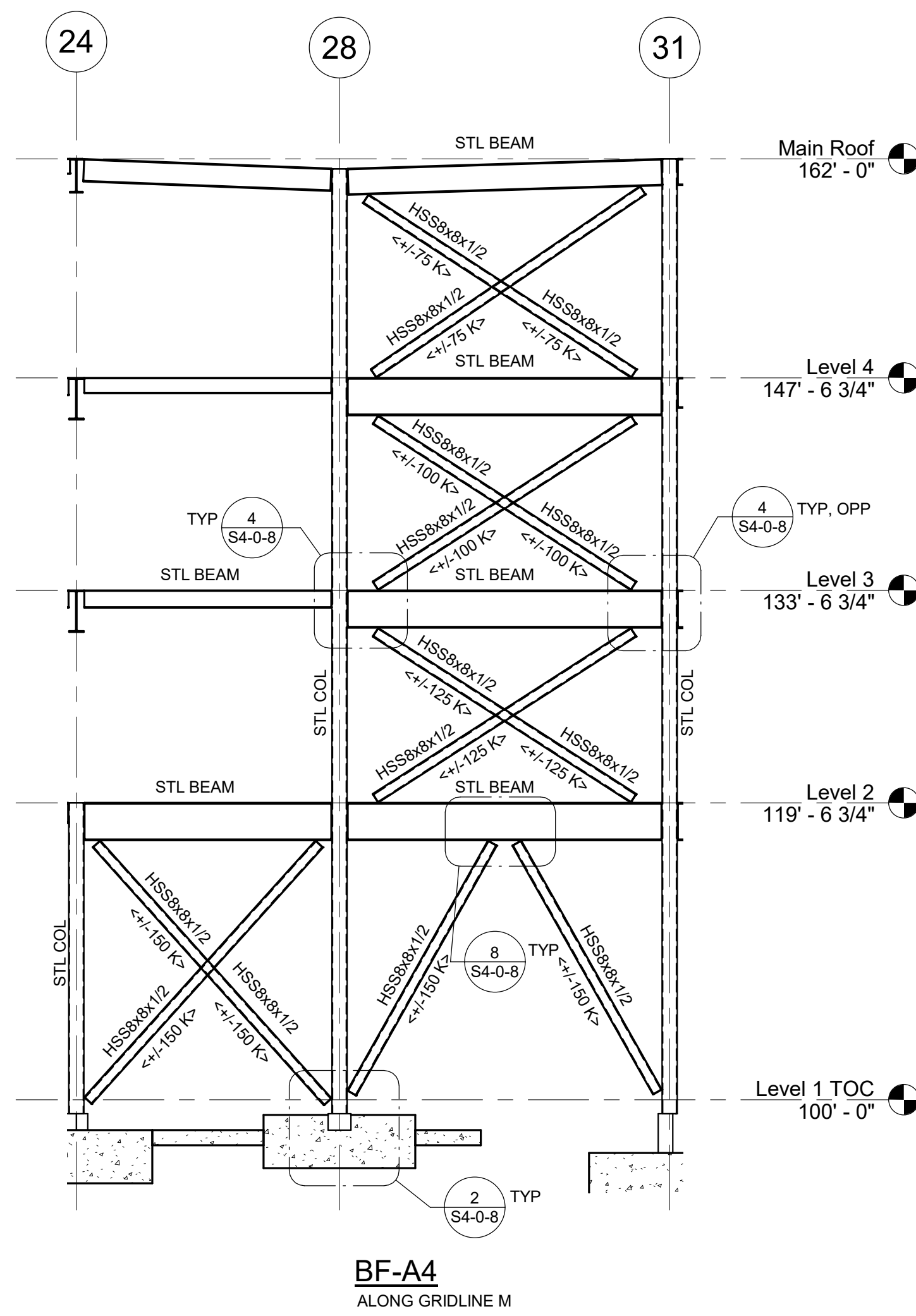
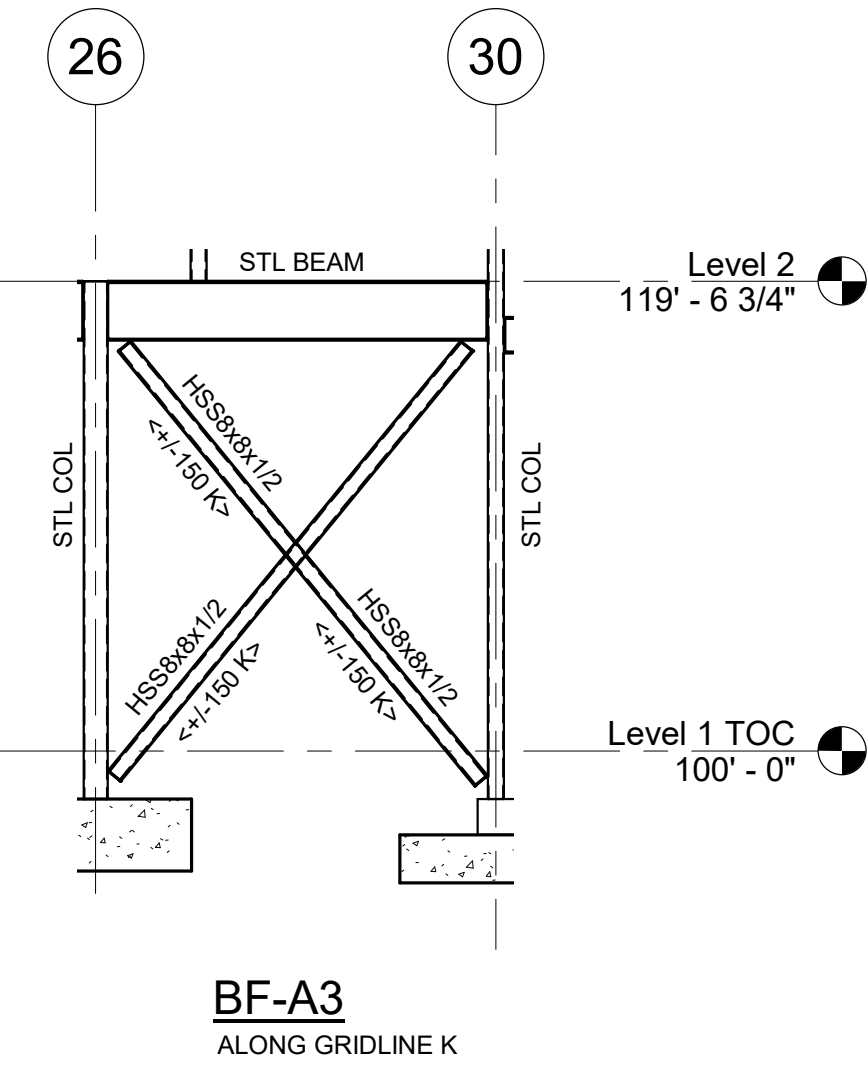
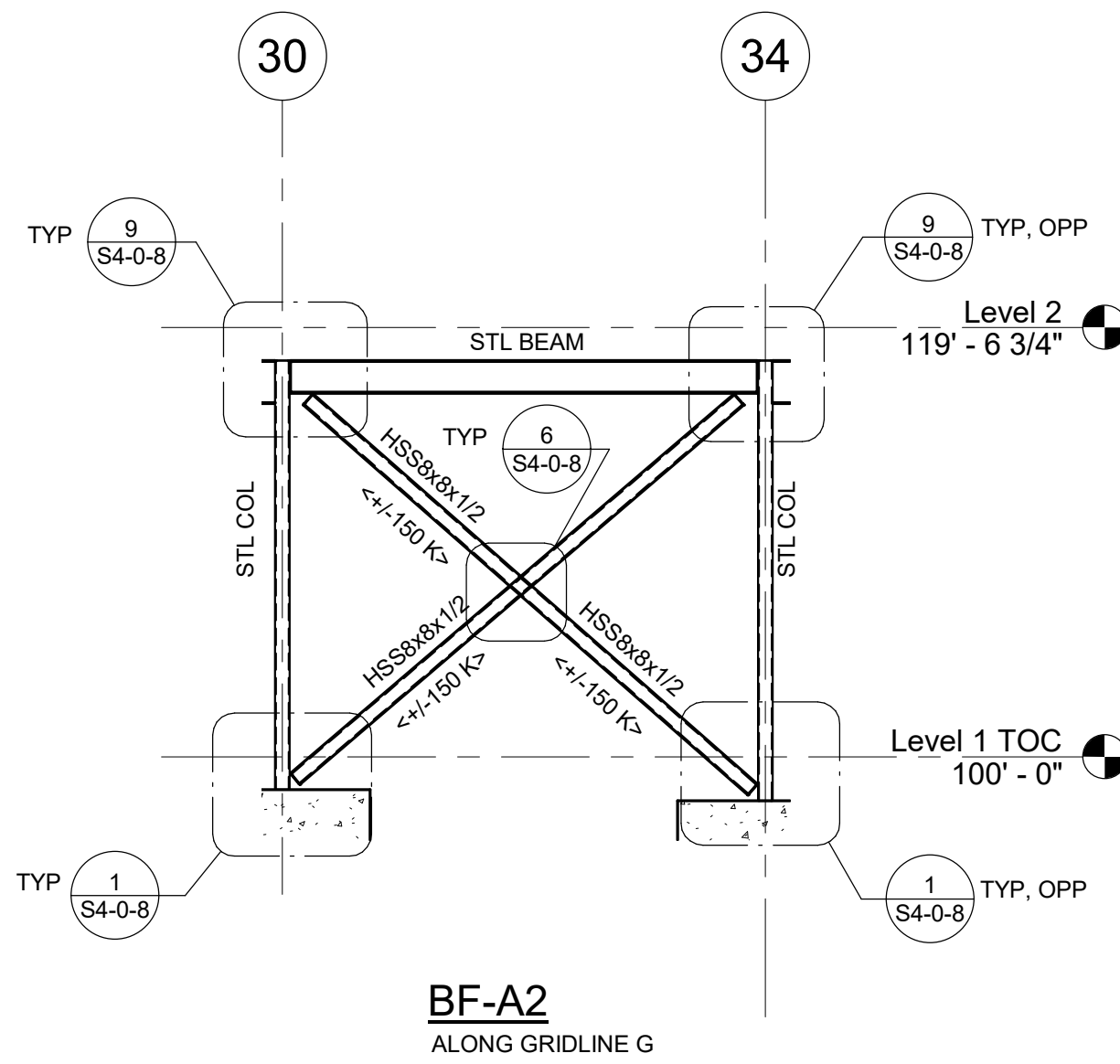
August 28th, 2023



SECTIONS

Scale: As indicated
Job No.: 20202
Drawn By: EDG
Date: August 28th, 2023

S3-0-9



BRACE FRAME NOTES:

- FABRICATOR IS RESPONSIBLE FOR BRACE CONNECTION DESIGN.
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- DESIGN DIAGONAL MEMBER CONNECTIONS FOR TWICE THE AXIAL DESIGN FORCE SHOWN BELOW EACH MEMBER <30k> (TENSION OR COMPRESSION). AXIAL DESIGN FORCE IS BASED ON LRFD. USE GENERAL UNIFORM FORCE METHOD FOR CONNECTION DESIGN.
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- SEE PLANS FOR COLUMN AND BEAM SIZES.
- CENTERLINE OF BRACE MEMBER SHALL BE OFFSET FROM WORKPOINT LINE AS NOTED AS SHOWN ON ELEVATION.
- * SYMBOL AT THE END OF A STEEL BEAM SIZE (HSS*, WP*, ETC.) DENOTES EXPOSED BRACE FRAME MEMBERS, INCLUDING STEEL BEAMS AND COLUMNS TO RECEIVE INTUMESCENT MASTIC FIREPROOFING.

FIREPROOFING NOTES:

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- COORDINATE FIREPROOFING REQUIREMENTS WITH THE SPECIFICATIONS AND ARCHITECTURAL DRAWINGS.

DRA

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NORTHEAST
METRO TECH

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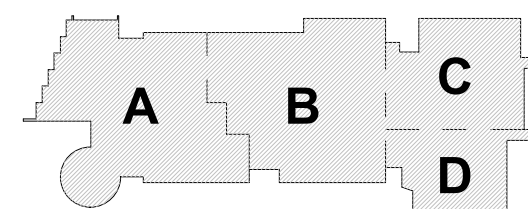
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03/12/2023	EARLY STRUCTURAL BID PACKAGE
REVISION LIST	
PR-002	6/29/2023 MISCELLANEOUS STRUCTURAL REVISIONS

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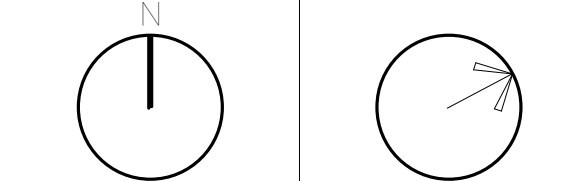
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August 28th, 2023



KEY PLAN

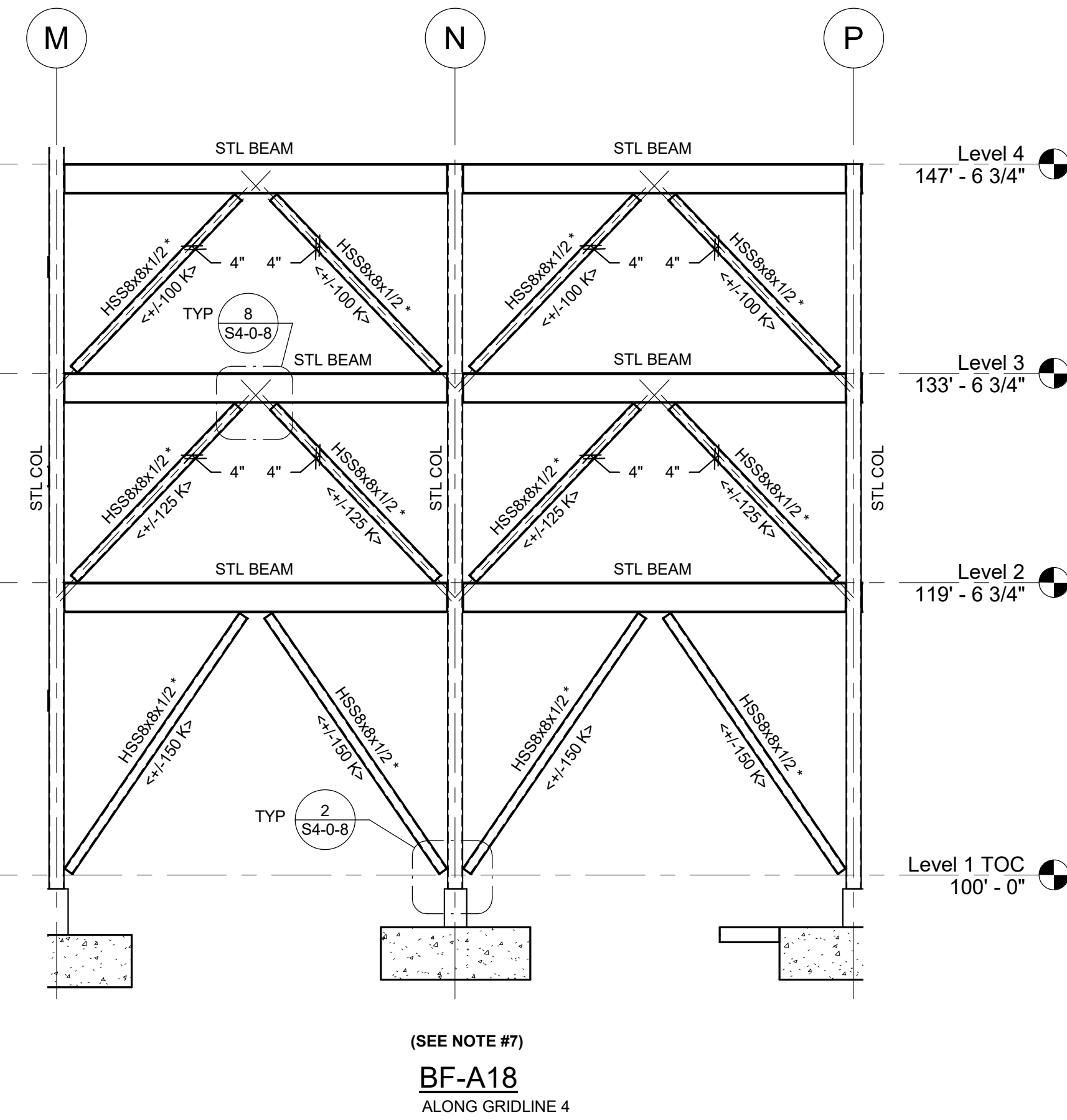
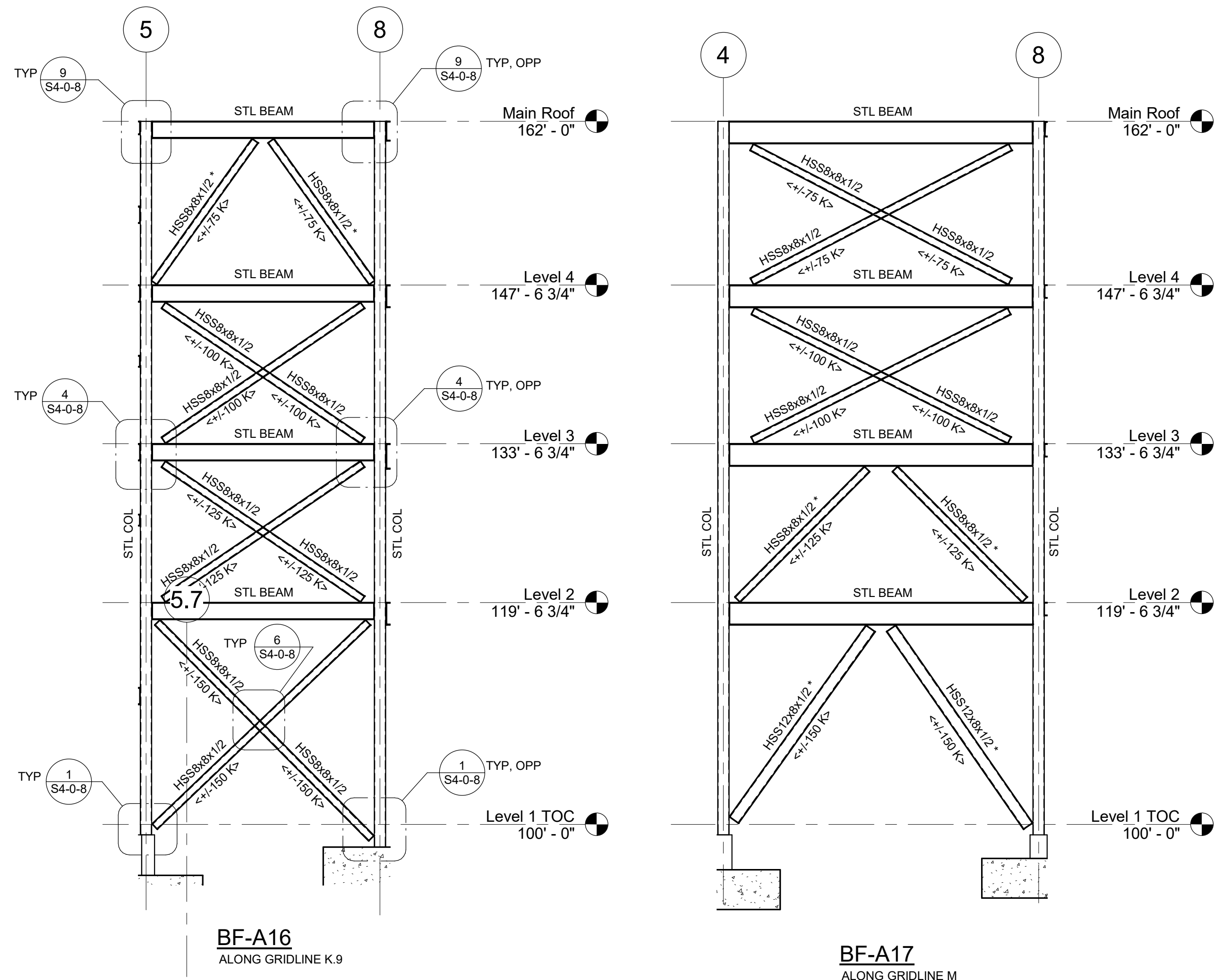
PROJECT NORTH MAGNETIC NORTH



BRACE FRAME
ELEVATIONS -
AREA A

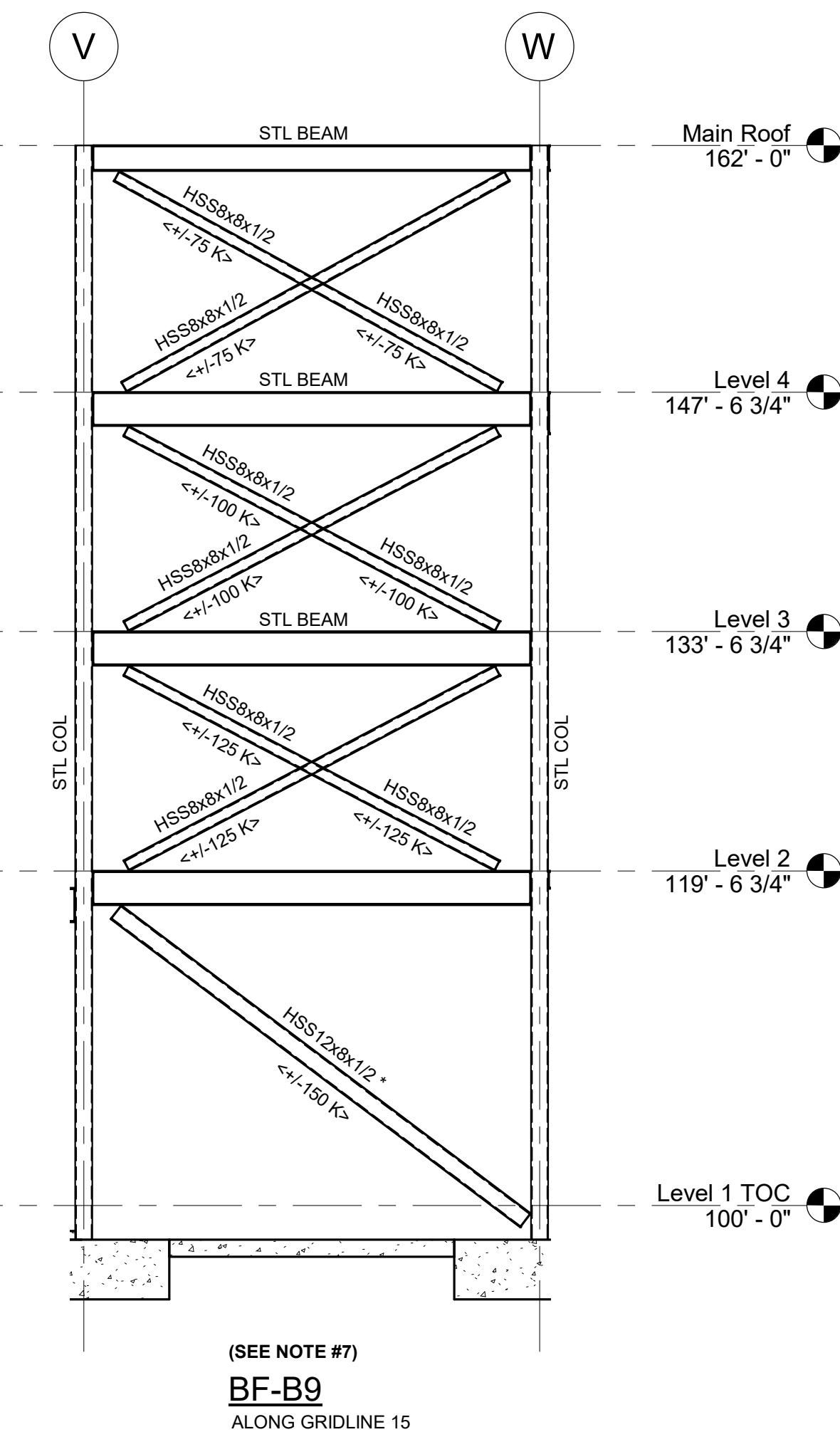
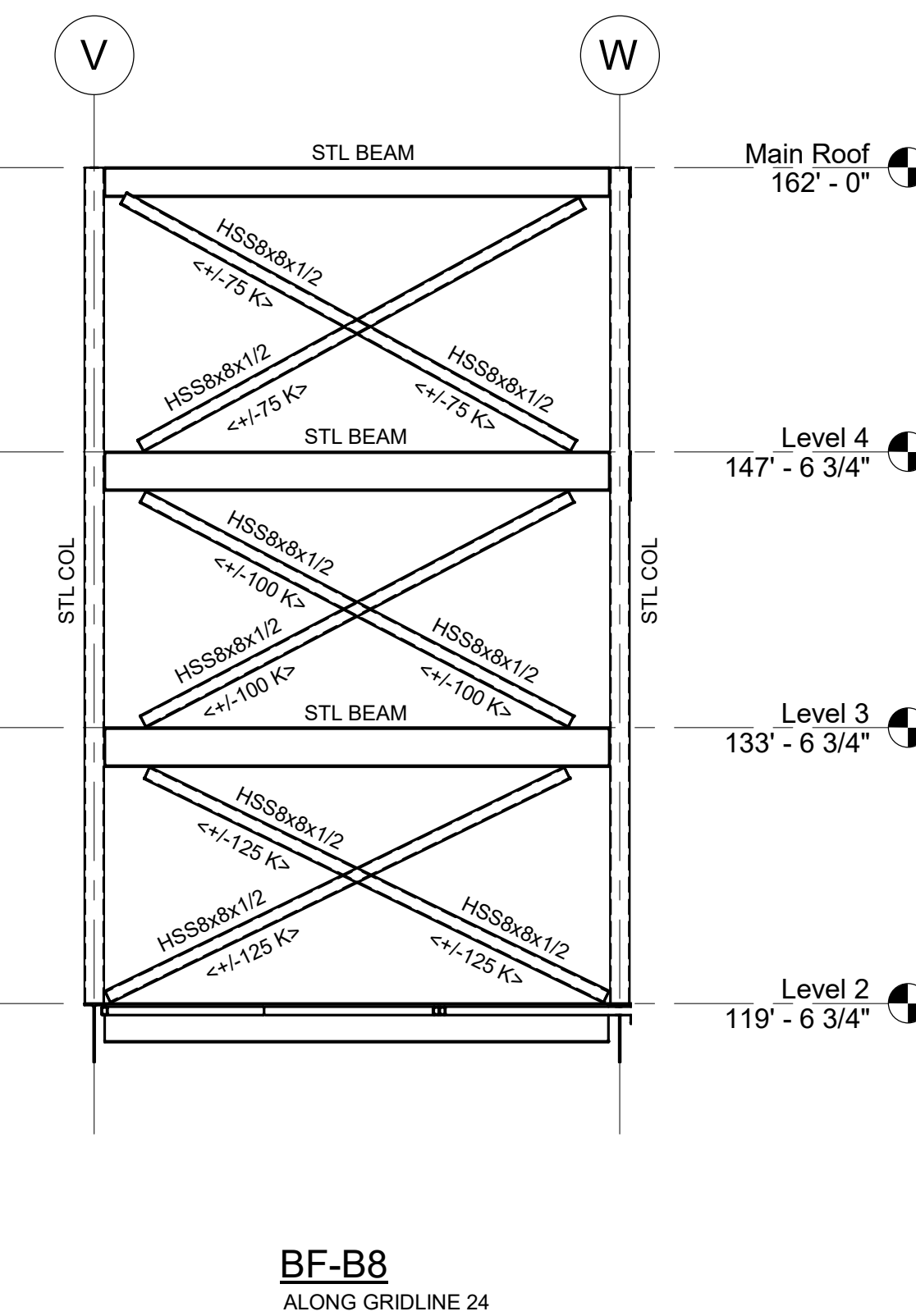
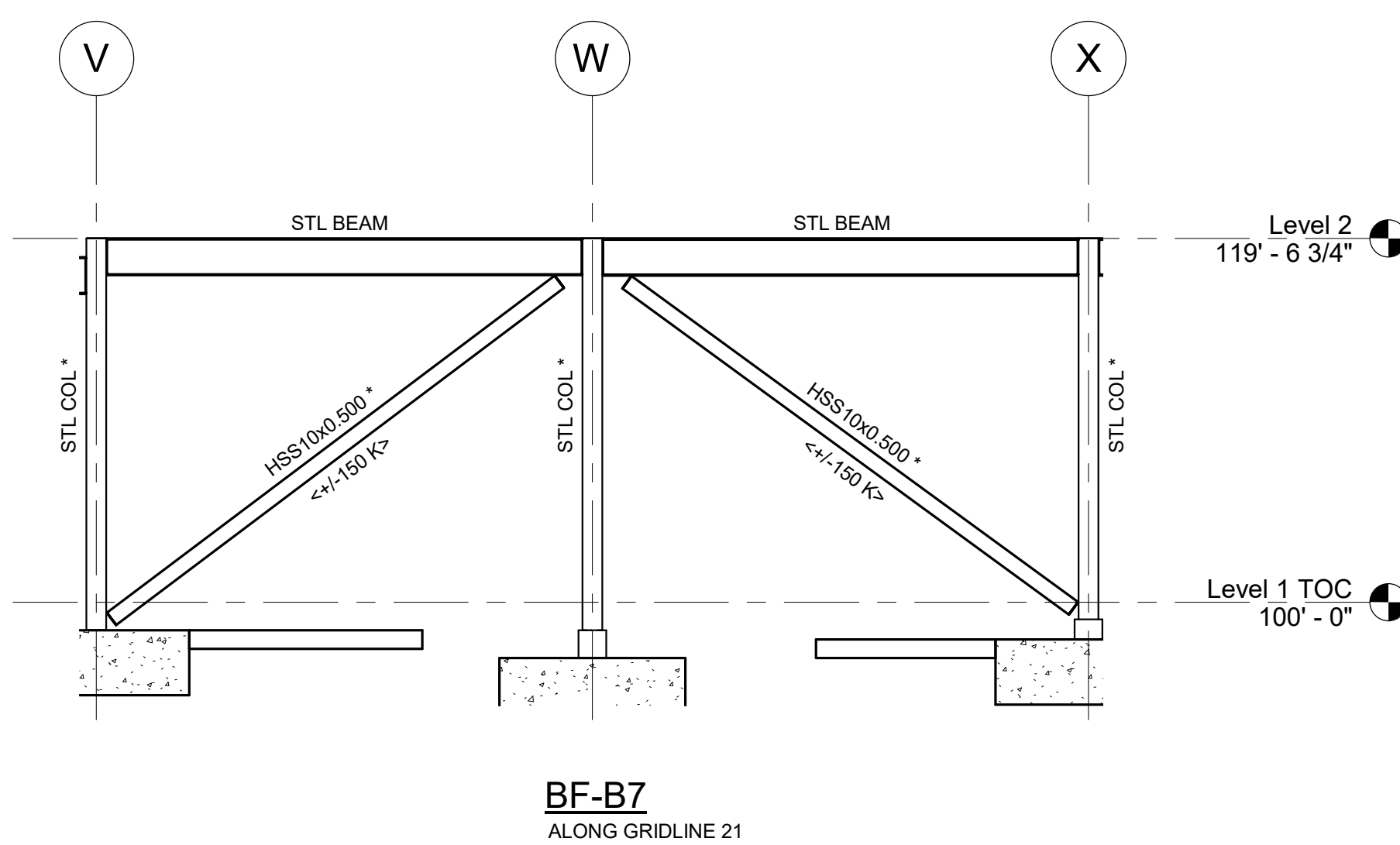
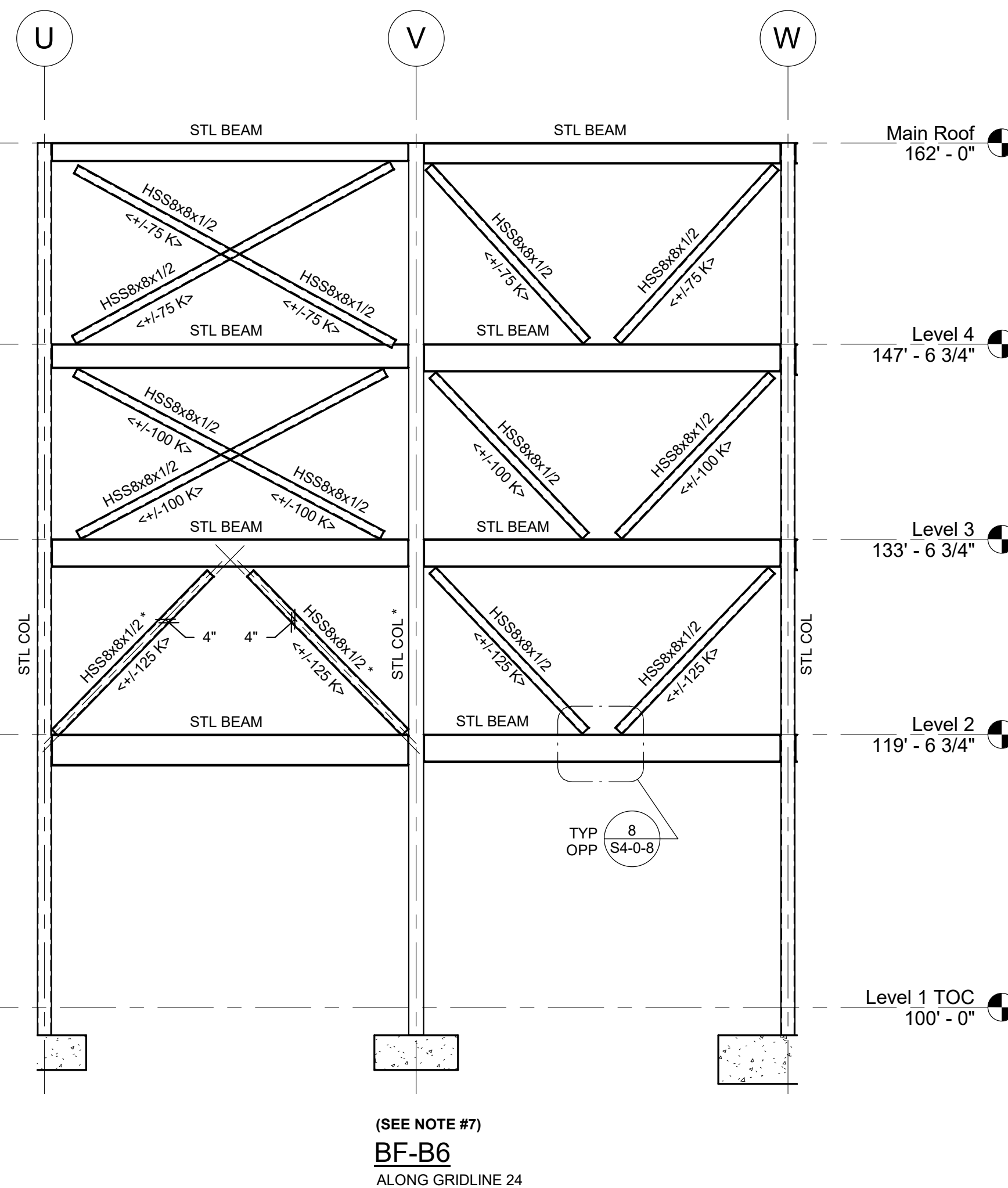
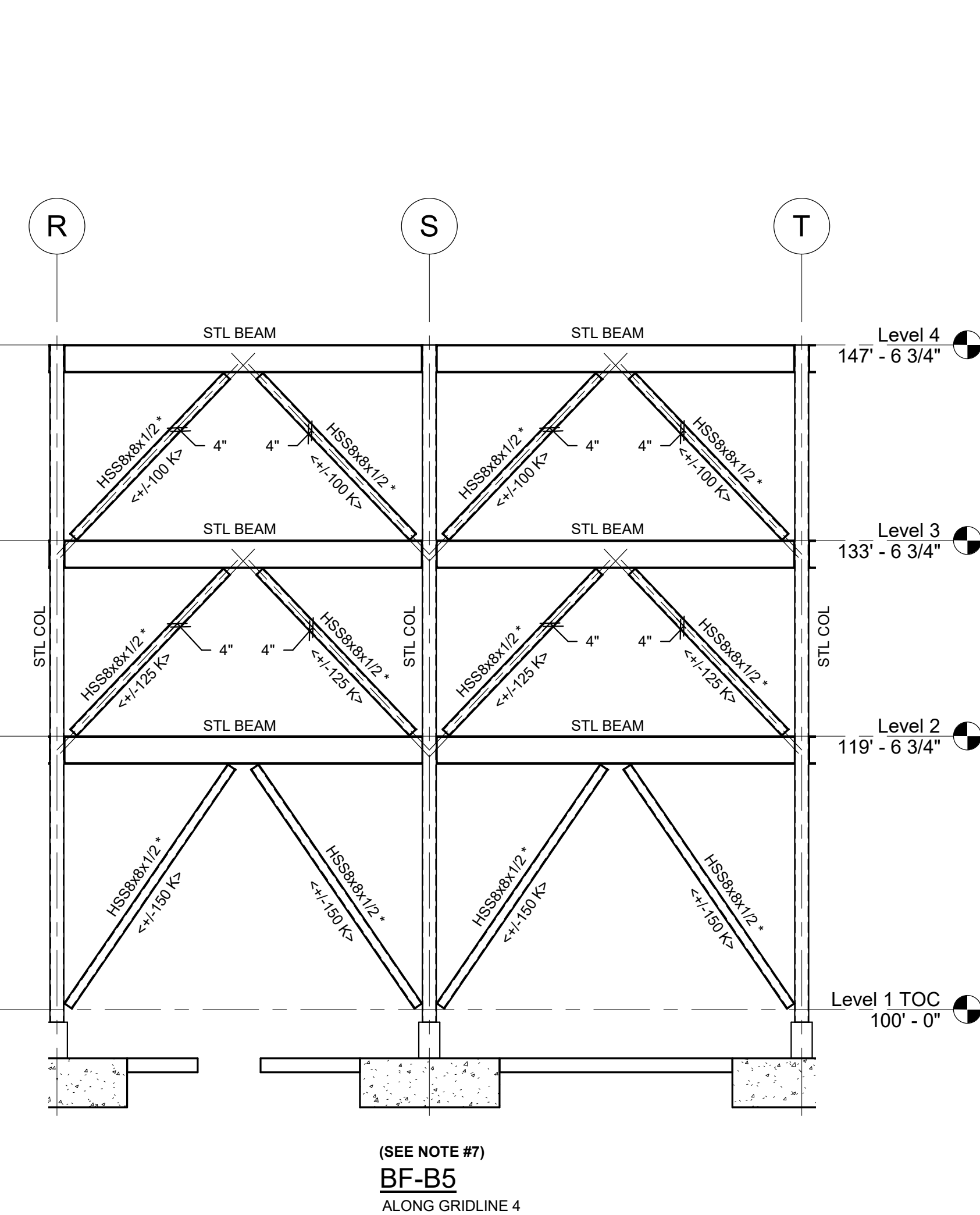
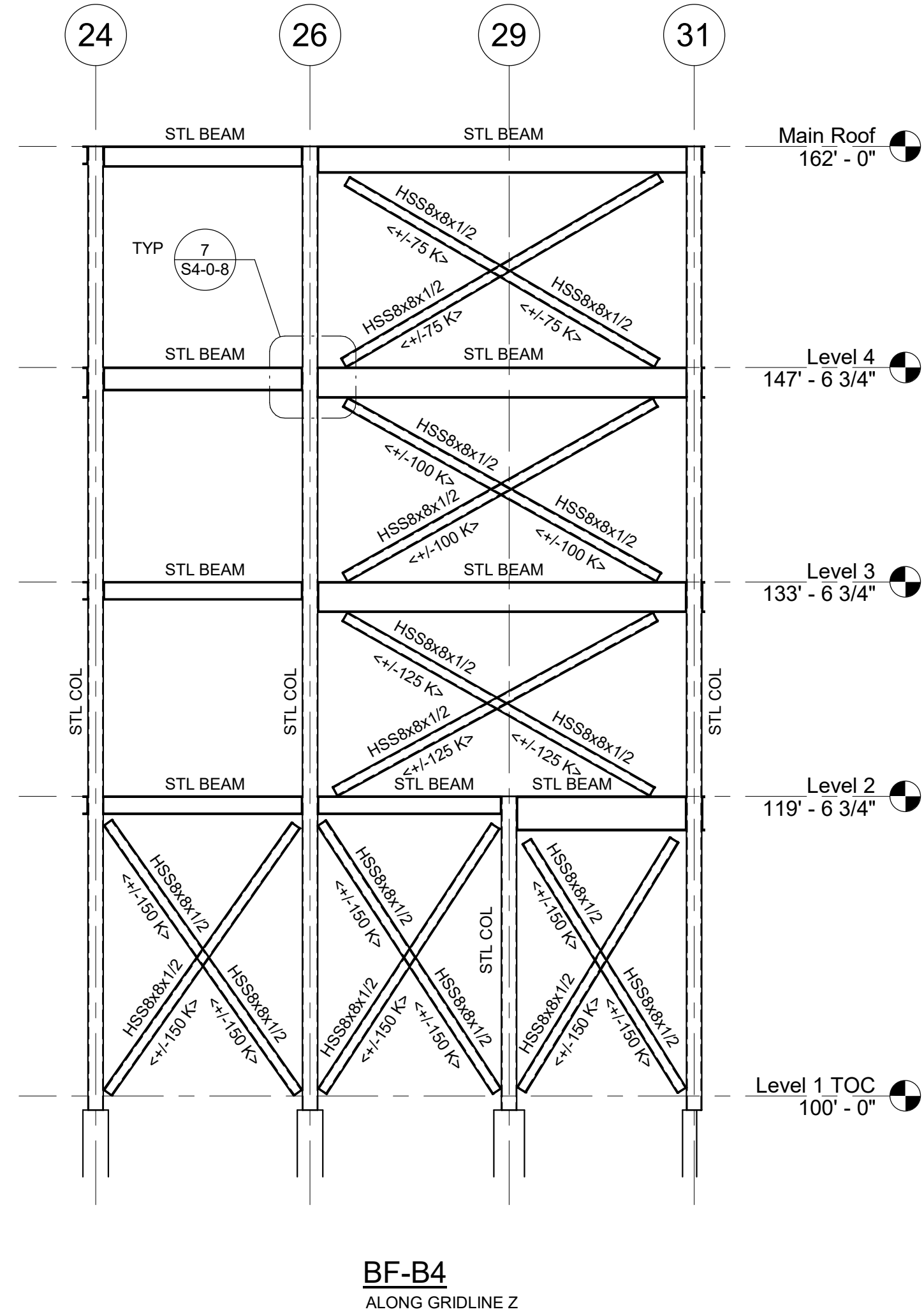
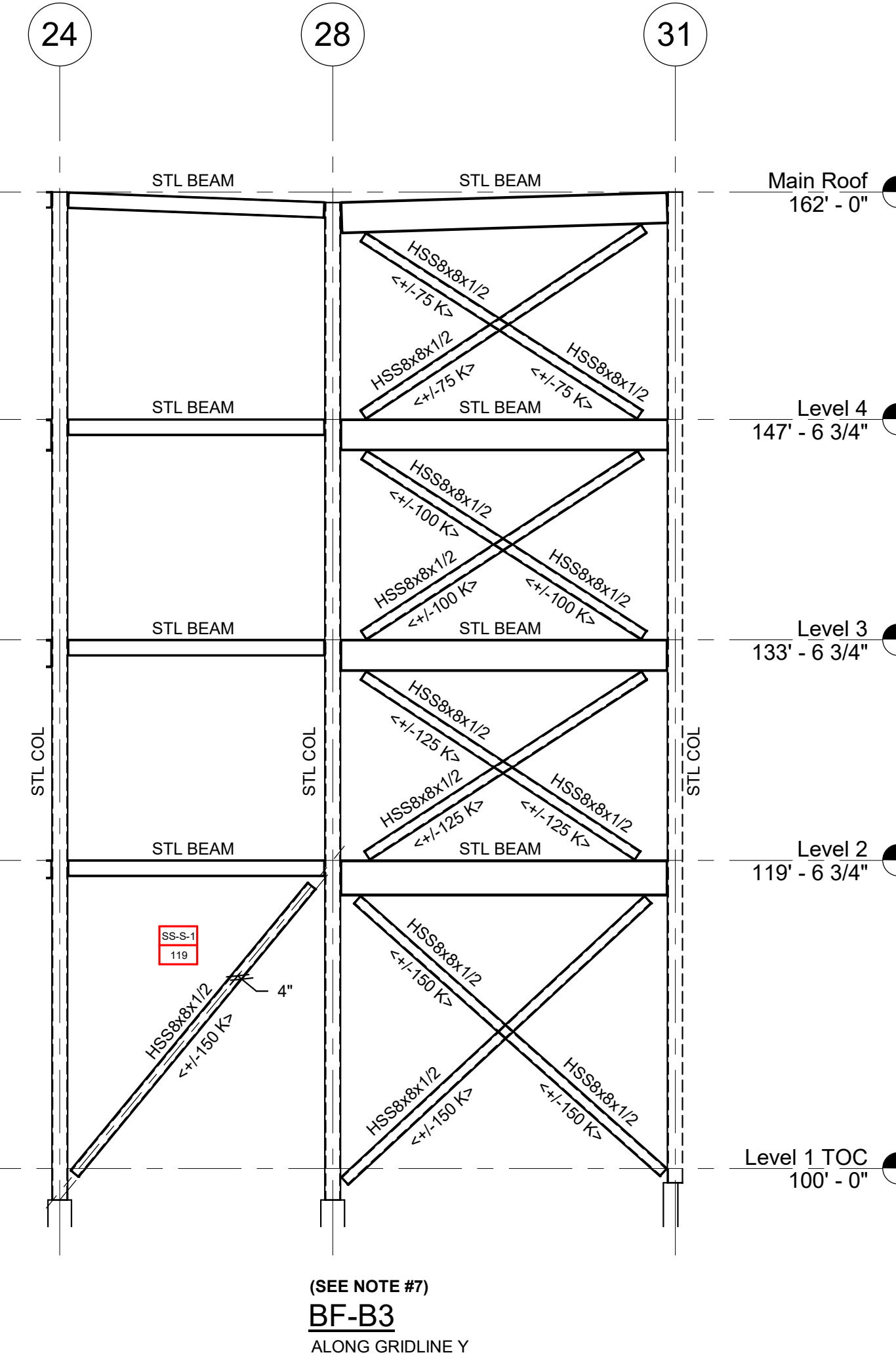
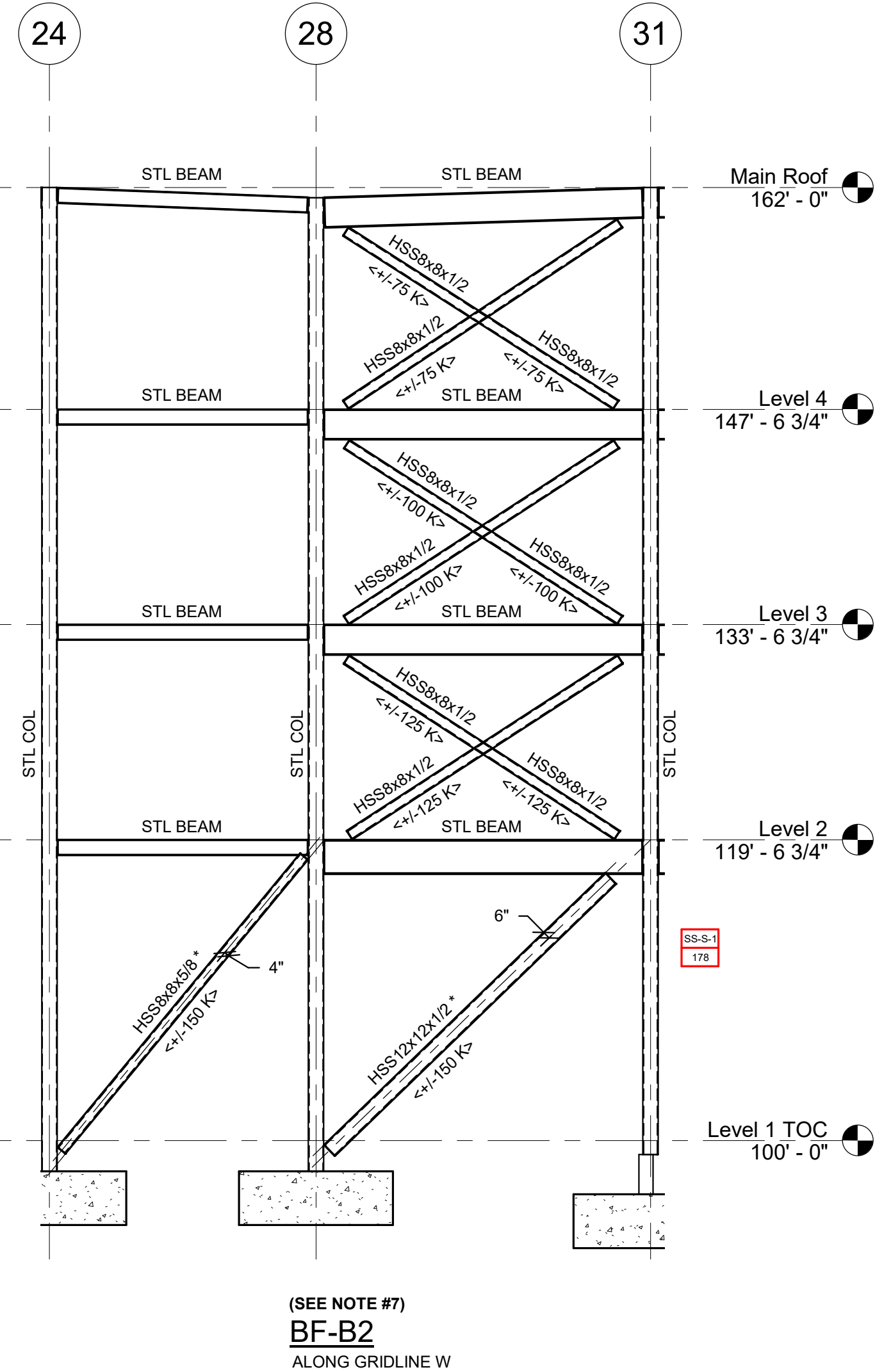
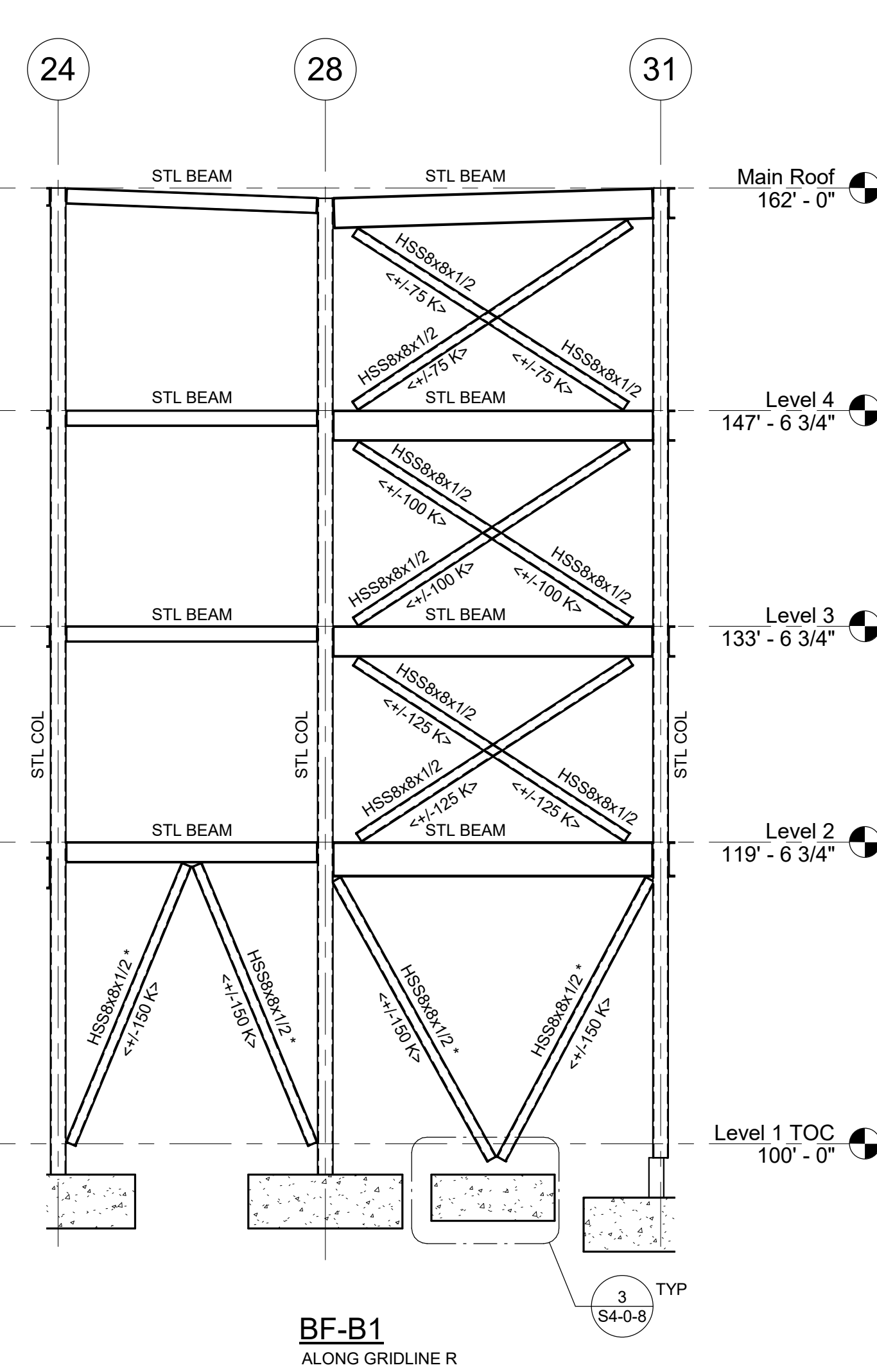
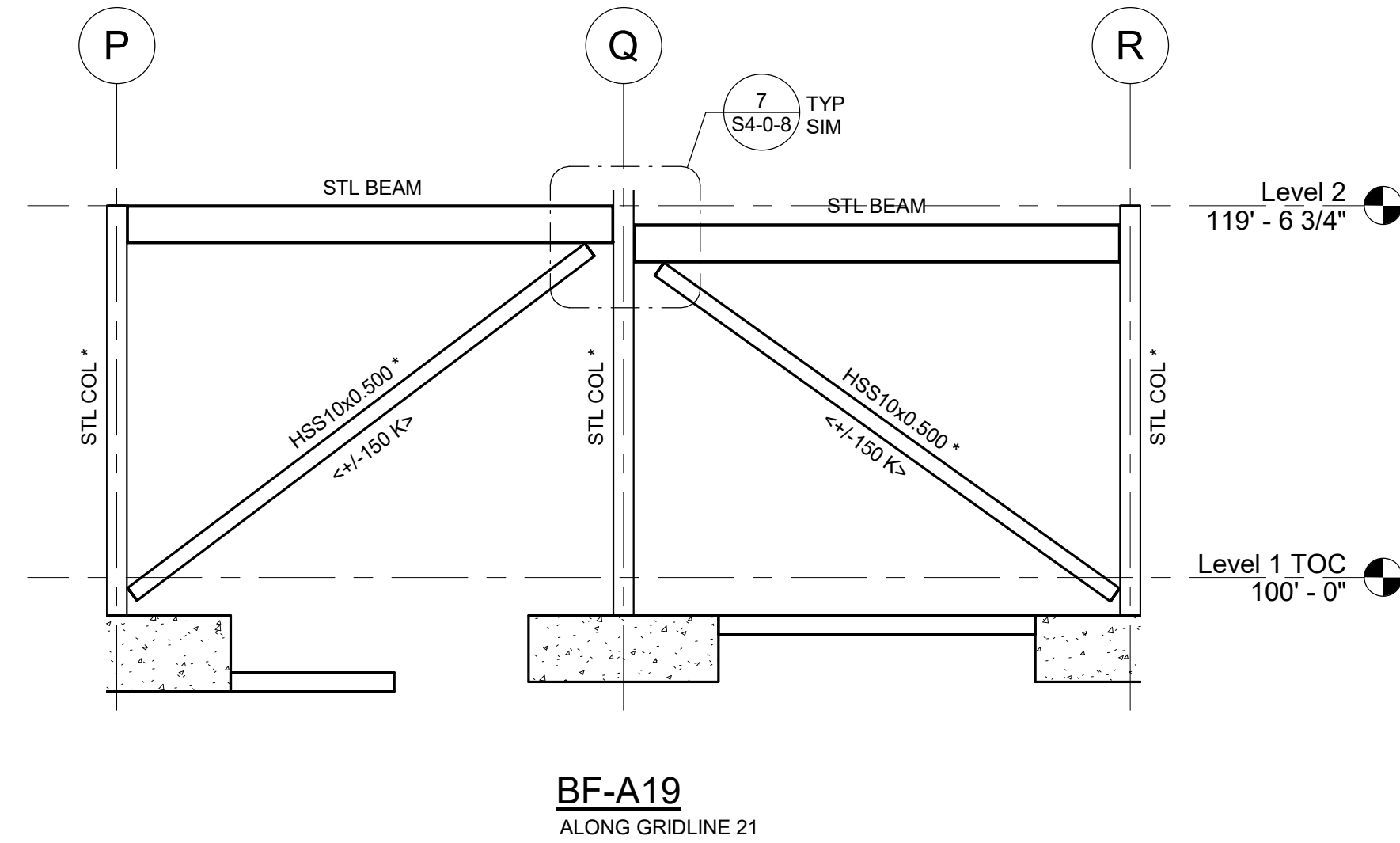
Scale: 1/8" = 1'-0"
Job No.: 20202
Drawn By: EDG
Date: August 28th, 2023

S4-0-1



- FIREPROOFING NOTES:**
- STEEL COLUMNS SHOWN ON THIS DRAWING SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING UNLESS INDICATED AS A ROUND HSS COLUMN. EXPOSED TO VIEW PORTION OF ROUND HSS COLUMNS SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING. CONCEALED FROM VIEW PORTION OF ROUND HSS COLUMN SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
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**NORTHEAST
METRO TECH**

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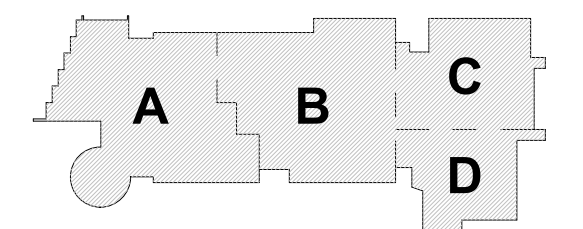
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Engineers Design Group Inc.
Structural Engineers
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(781)396-9007
EDG@EDGINC.COM

03/31/2023	EARLY STRUCTURAL BID PACKAGE
REVISION LIST	
SS-S-1	4/14/2023 STRUCTURAL STEEL ADDENDUM 1

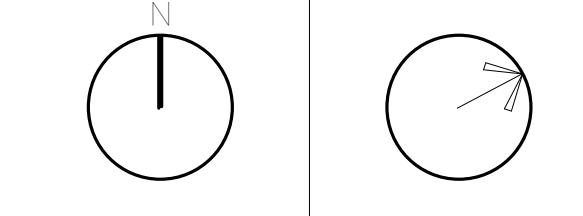
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BID SET

August 28th, 2023



PROJECT NORTH
MAGNETIC NORTH



**BRACE FRAME
ELEVATION -
AREAS A + B**

Scale: 1/8" = 1'-0"
Job No.: 20202
Drawn By: EDG
Date: August 28th, 2023

S4-0-2

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Studio 300
Waltham, MA 02453
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Wakefield, MA 01880



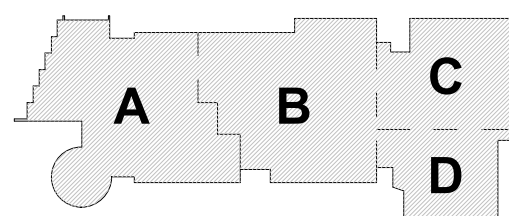
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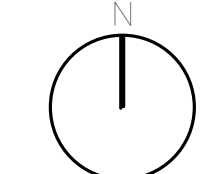
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August 28th, 2023

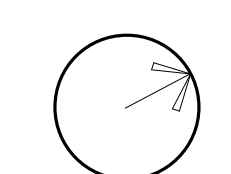


KEY PLAN

PROJECT NORTH



MAGNETIC NORTH



BRACE FRAME
ELEVATION -
AREA B

Scale: 1/8" = 1'-0"

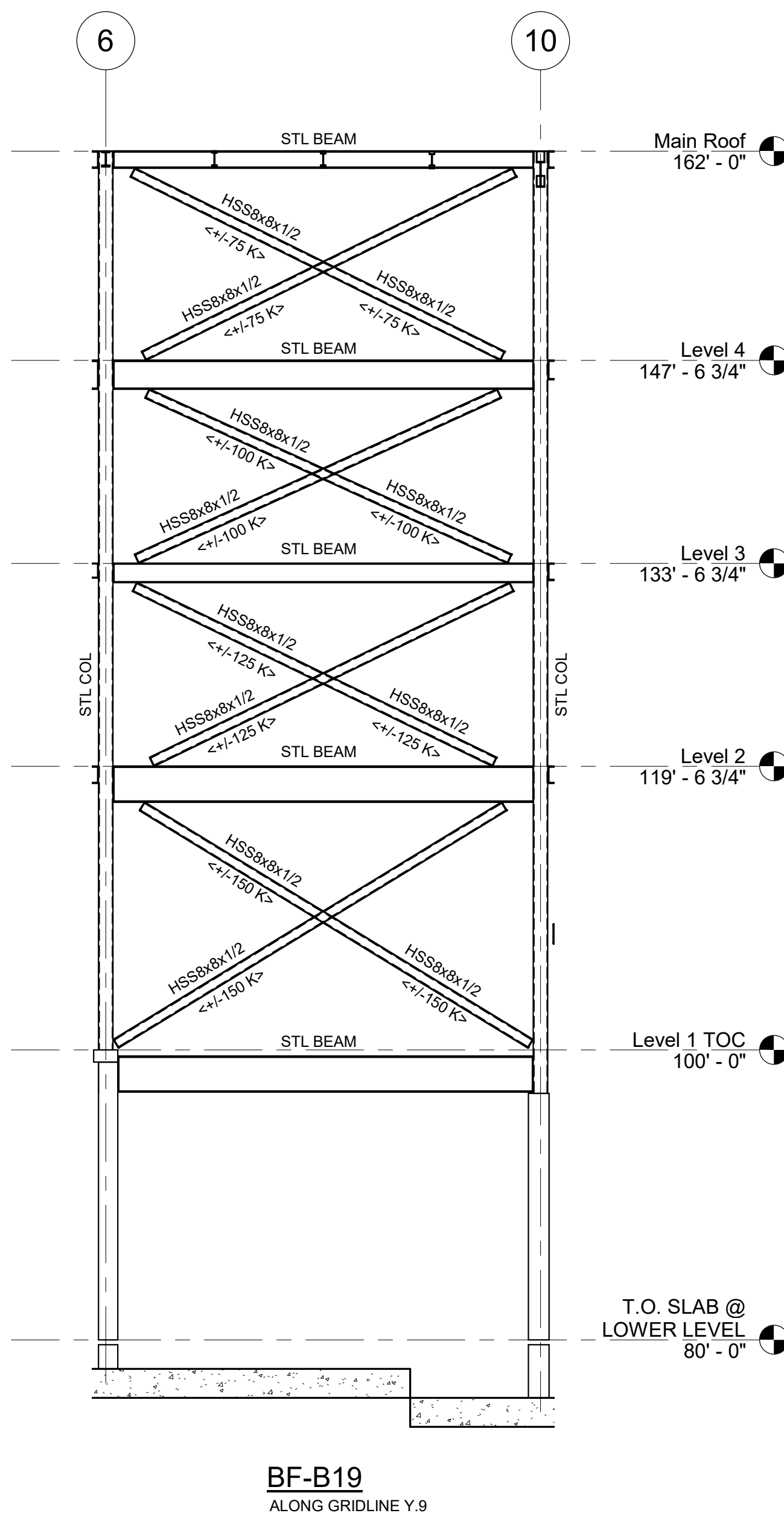
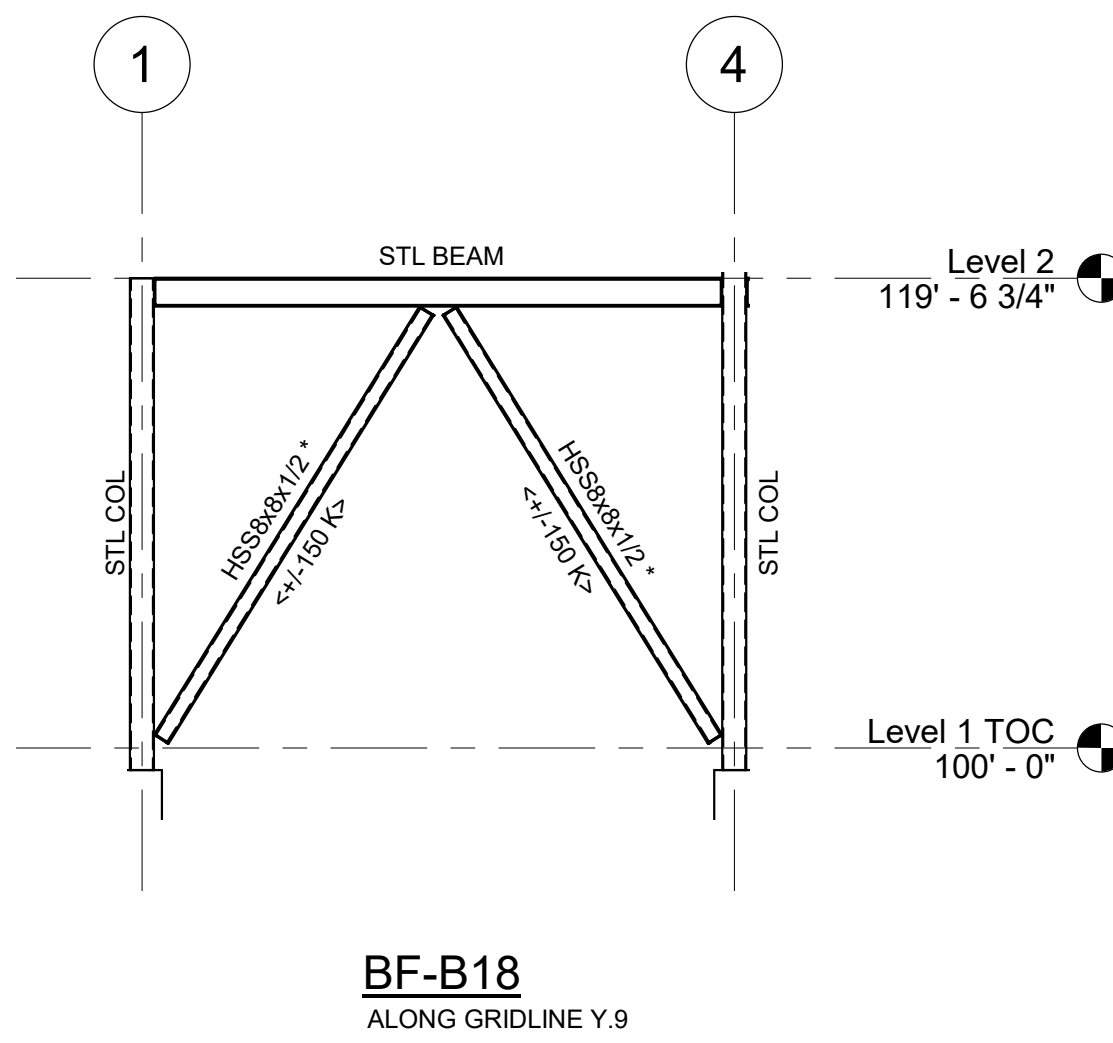
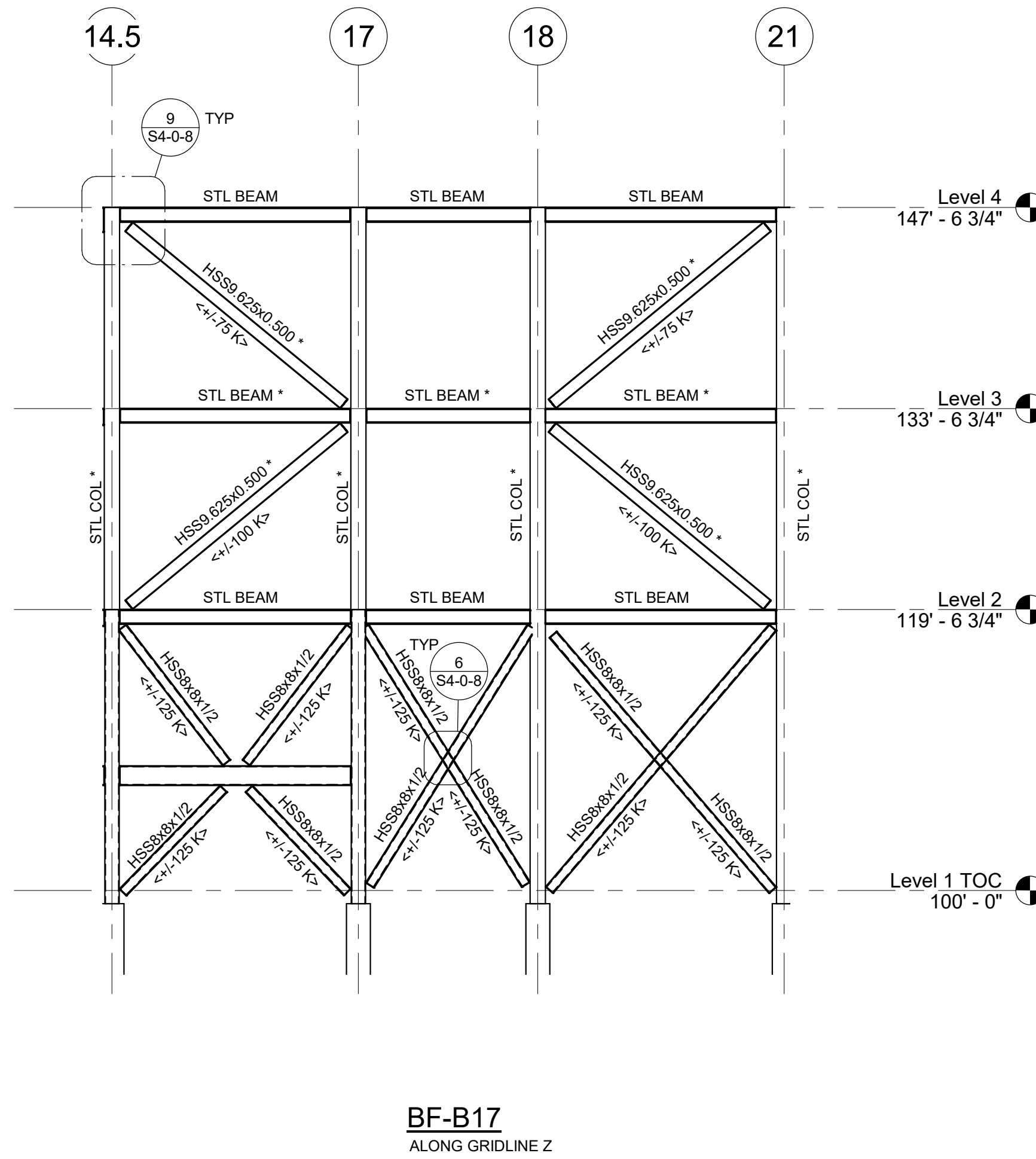
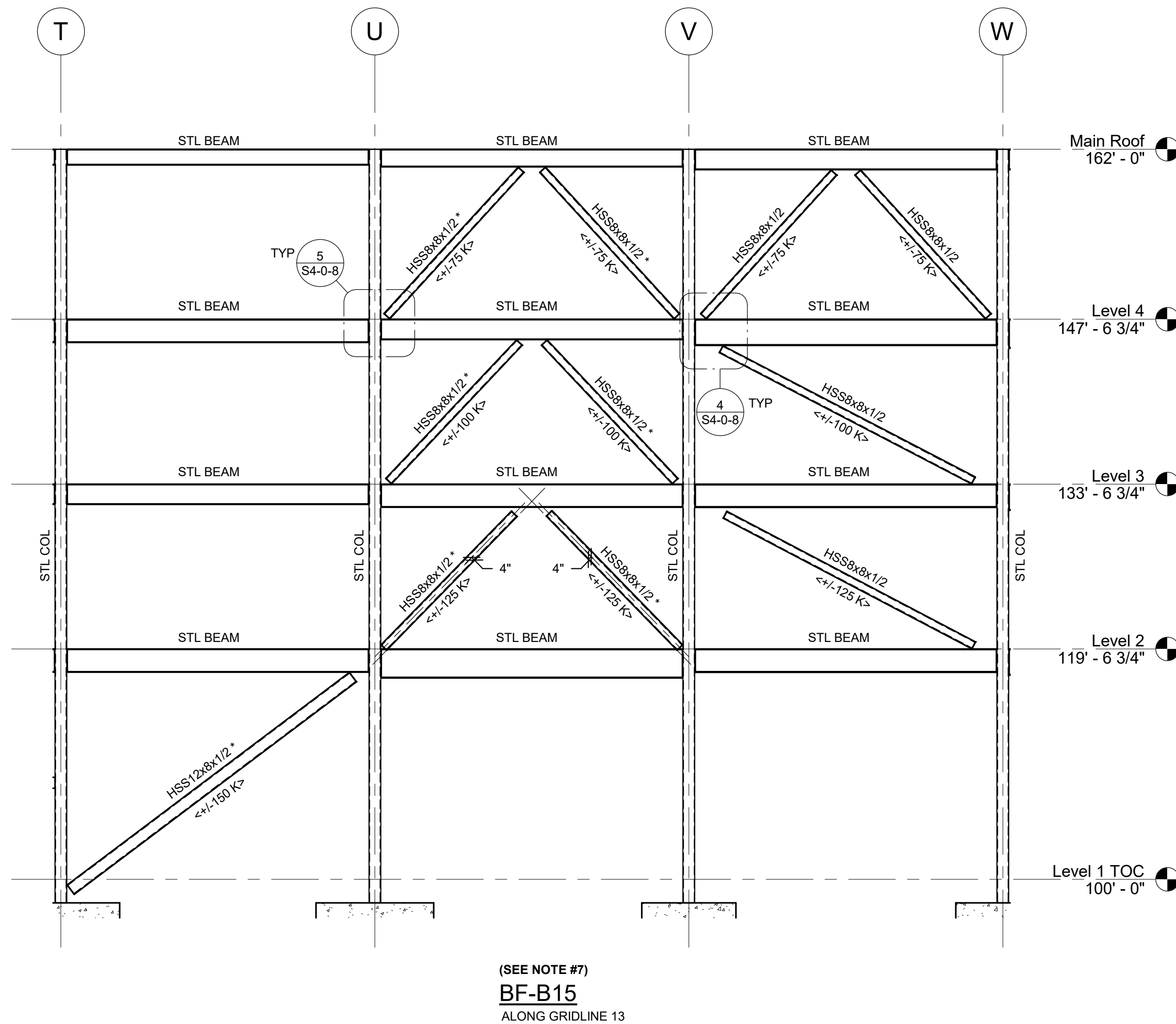
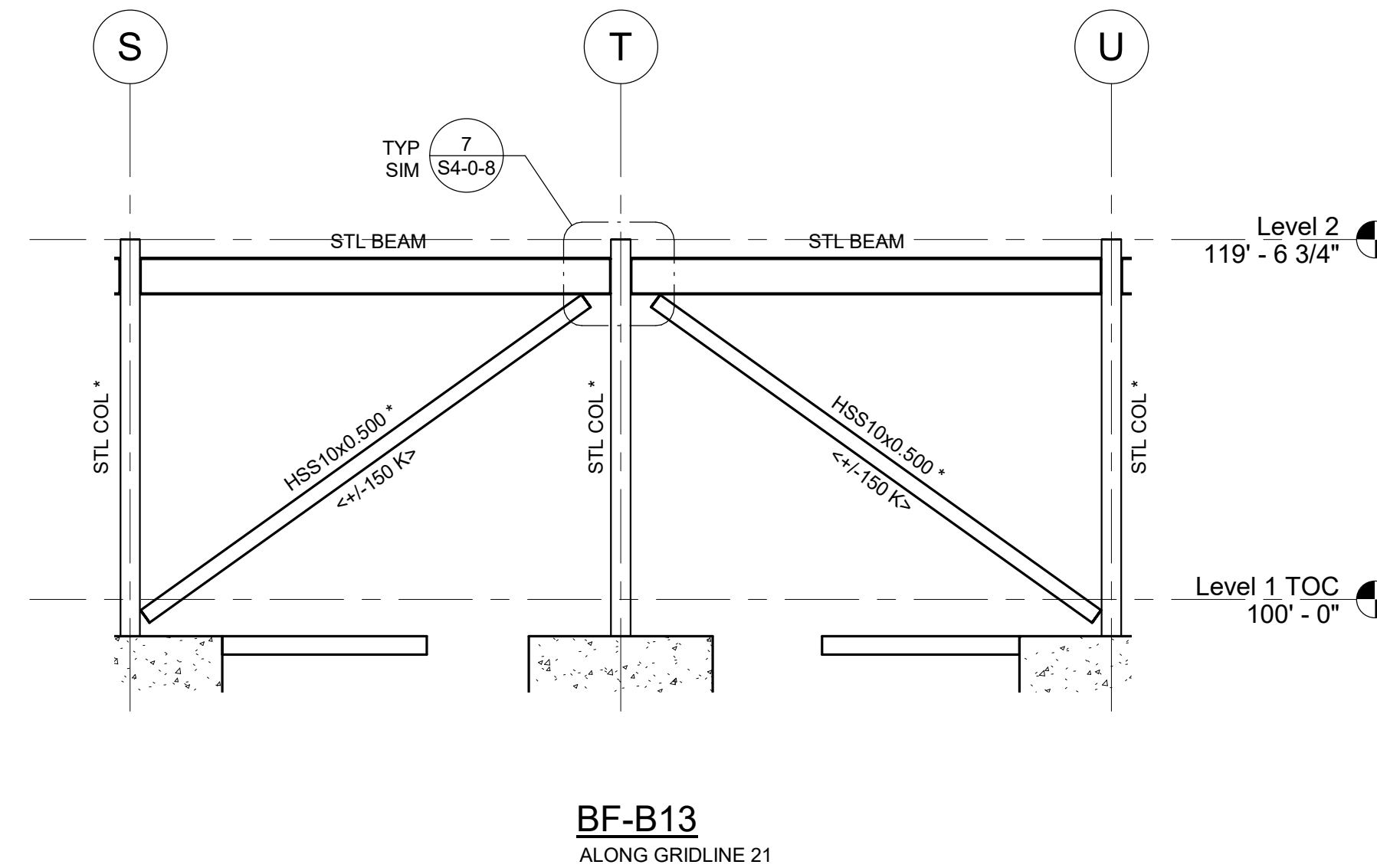
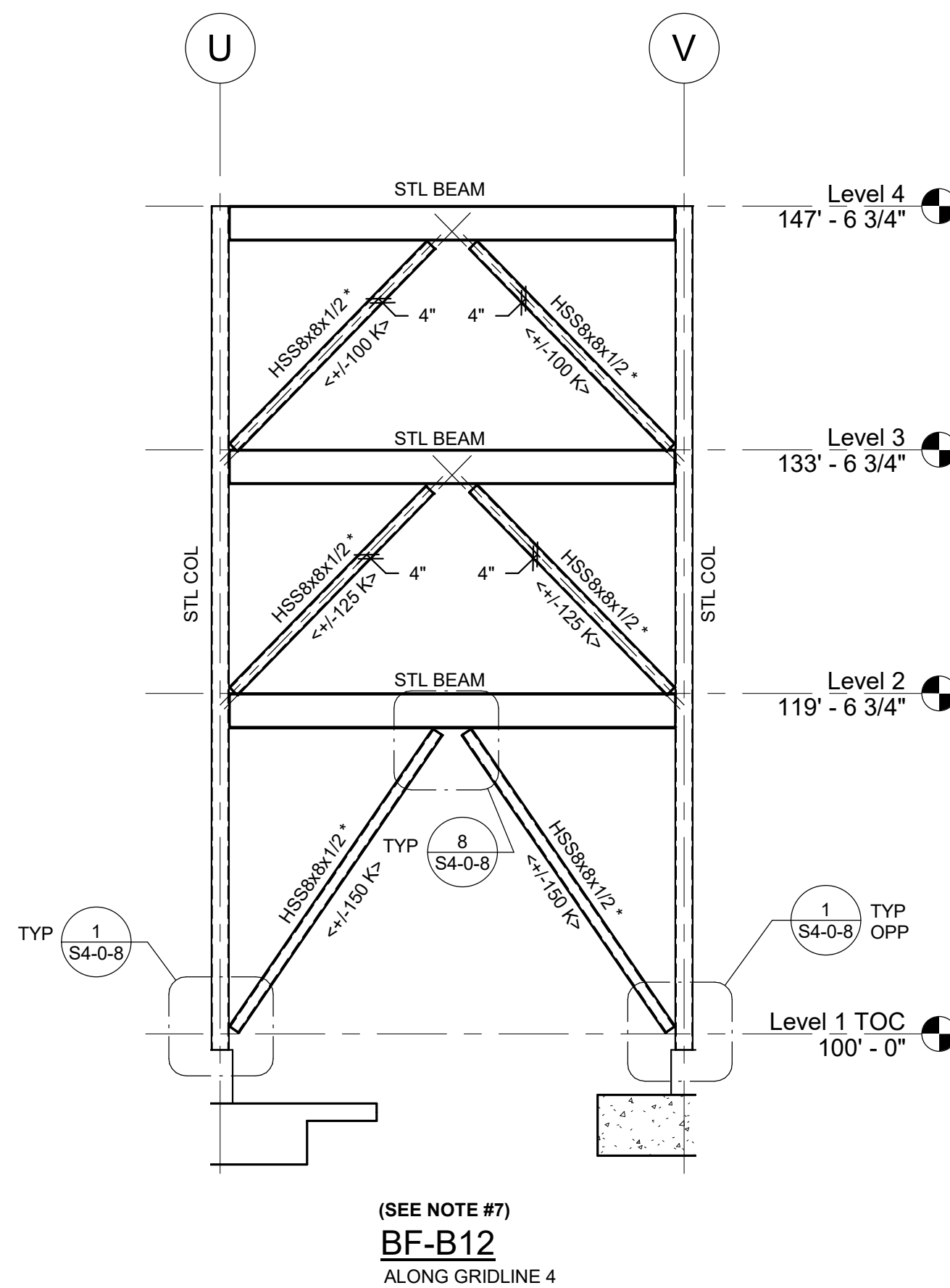
Job No.: 20202

Drawn By: EDG

Date: August 28th, 2023

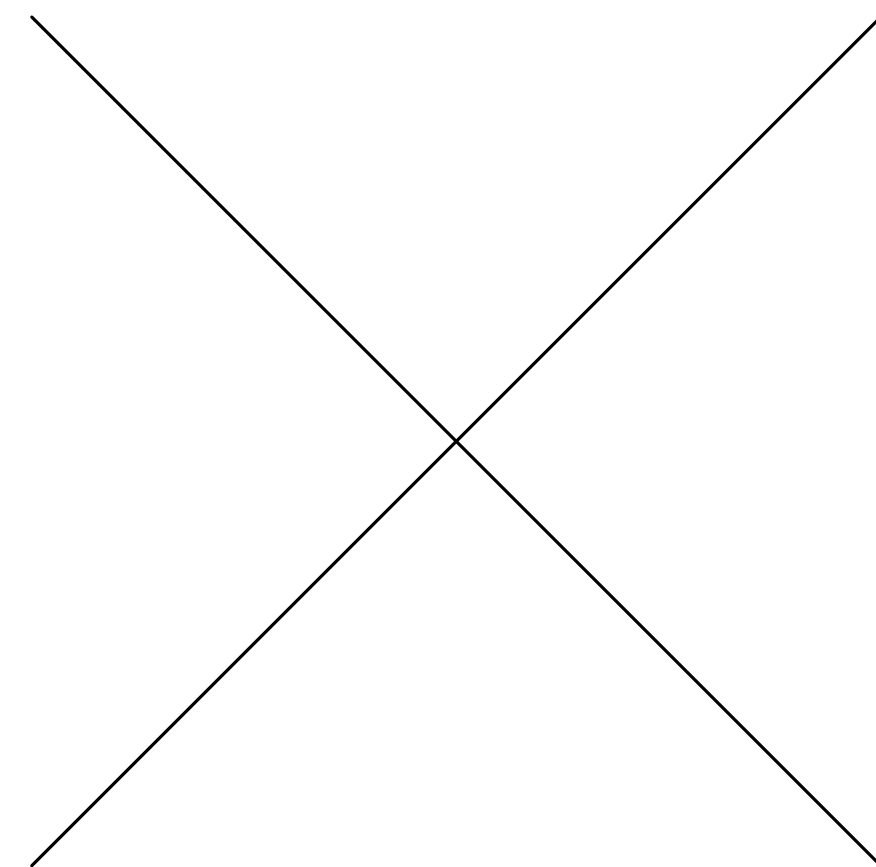
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NOT USED
BF-10B



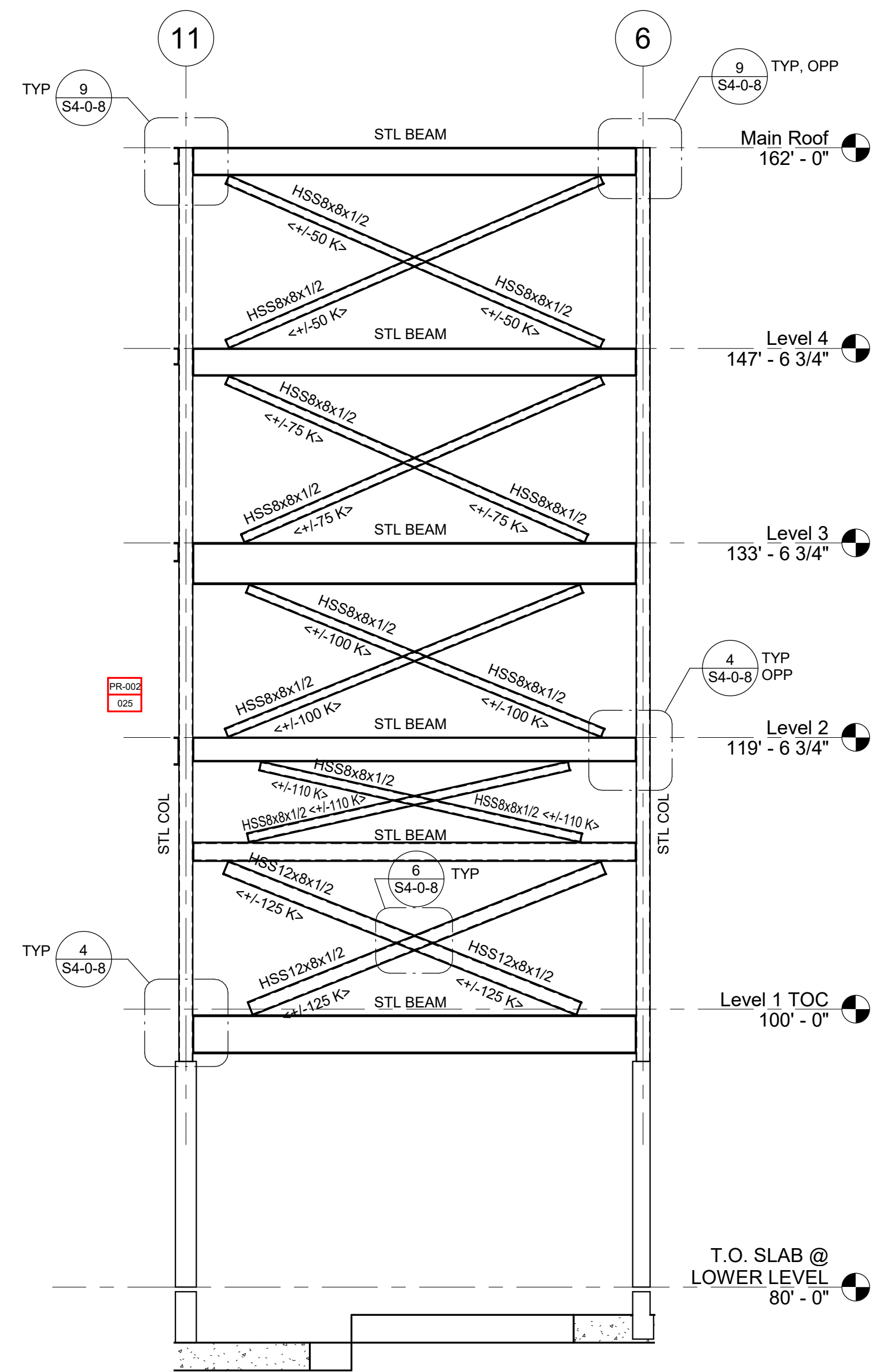
- FIREPROOFING NOTES:**
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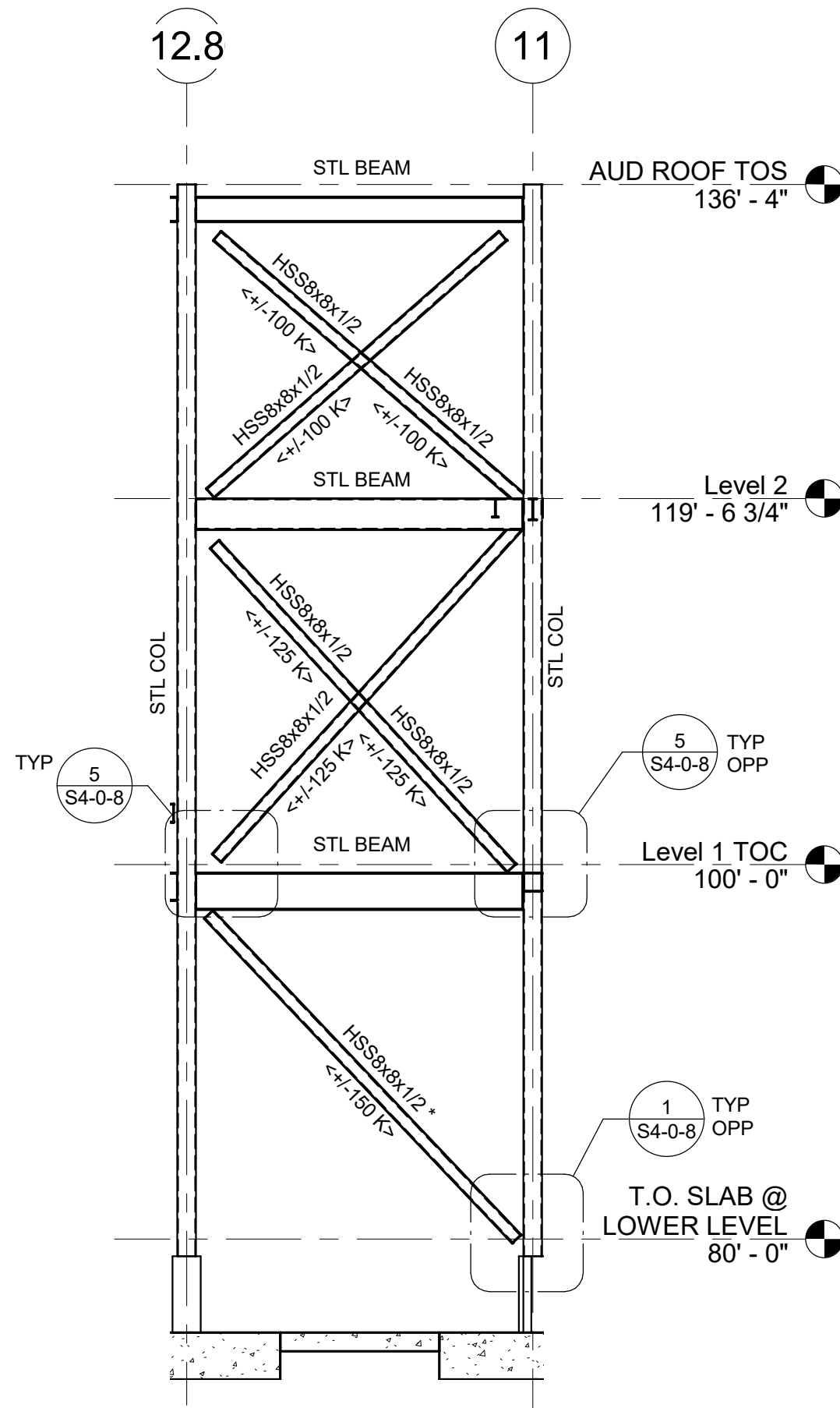


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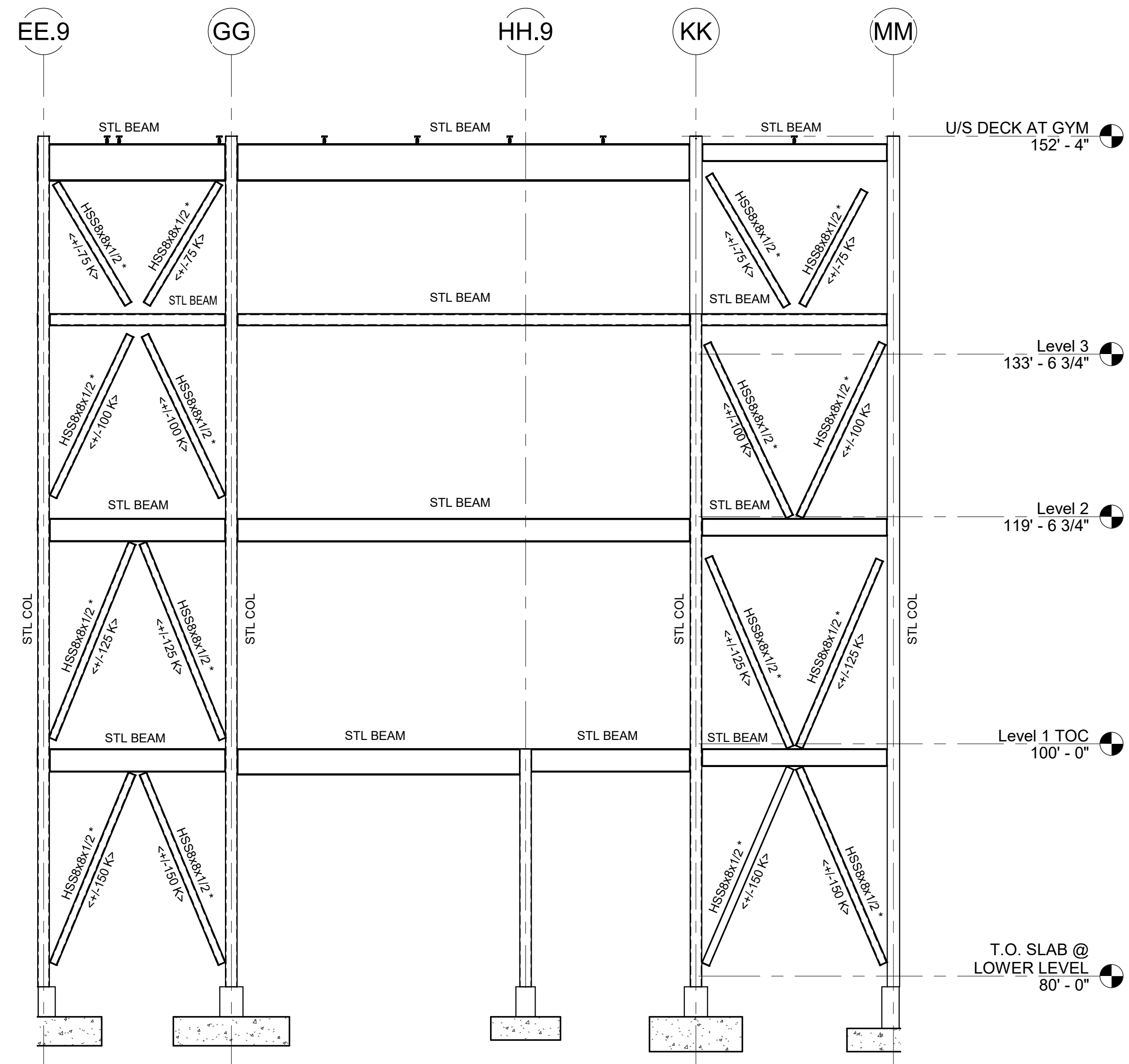
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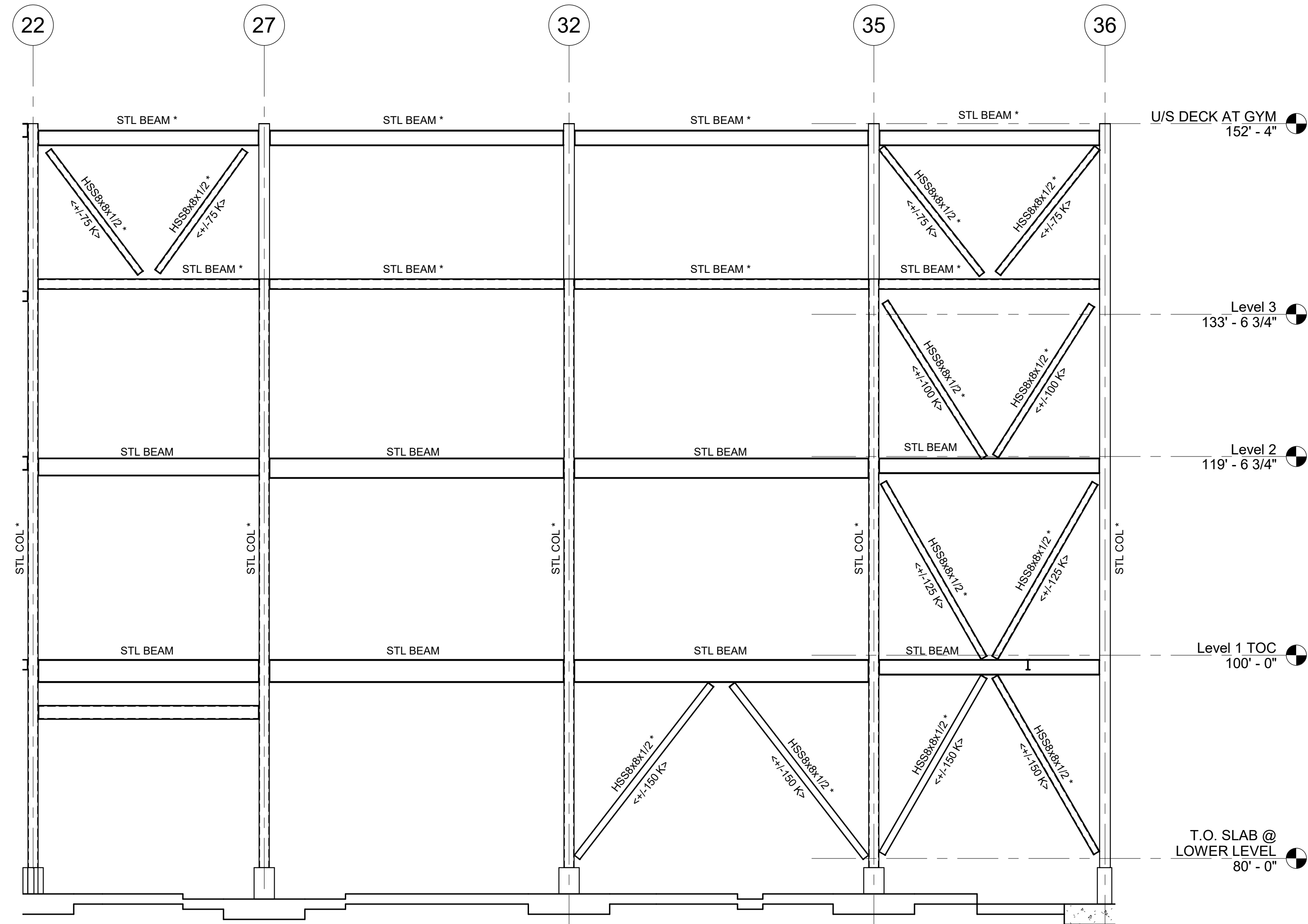
BF-C2
ALONG GRIDLINE AA



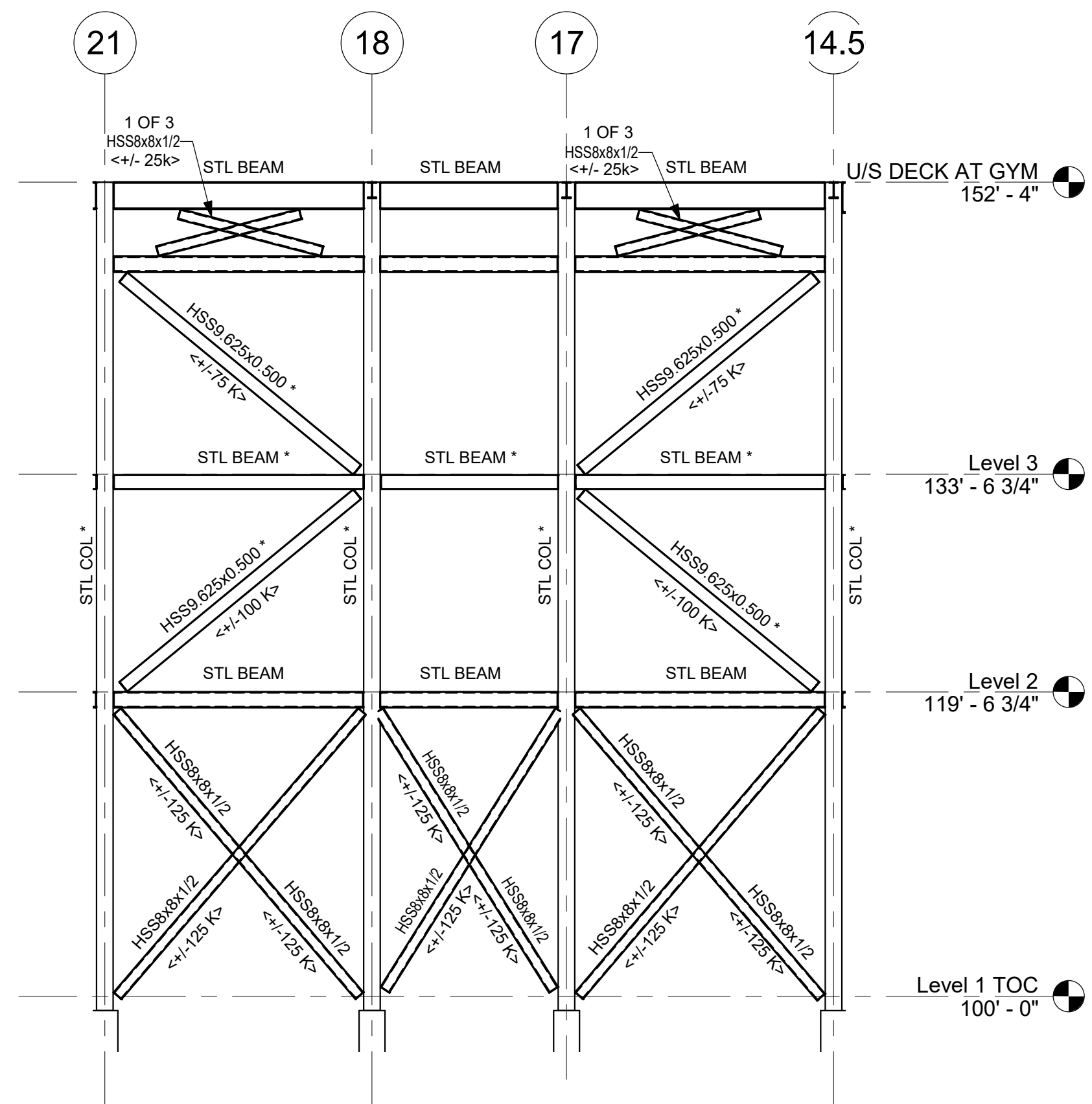
BF-C3
ALONG GRIDLINE PP



BF-D1 AND SW-D3
ALONG GRIDLINE 96



BF-D2 AND SW-D2
ALONG GRIDLINE MM



BF-D4
ALONG GRIDLINE AA

FIREPROOFING NOTES:

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Wakefield, MA 01880

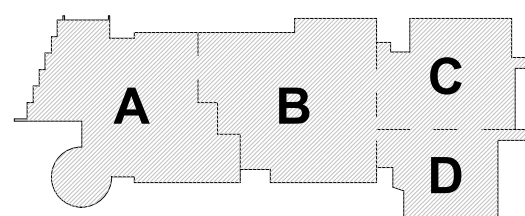
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03/31/2023	EARLY STRUCTURAL BID PACKAGE
REVISION LIST	
PR-002	6/29/2023 MISCELLANEOUS STRUCTURAL REVISIONS

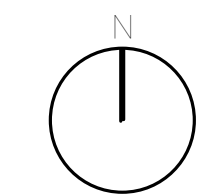
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August 28th, 2023

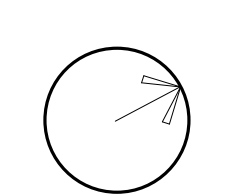


KEY PLAN

PROJECT NORTH



MAGNETIC NORTH



BRACED FRAME
ELEVATION C +
D

Scale: 1/8" = 1'-0"

Job No.: 20202

Drawn By: EDG

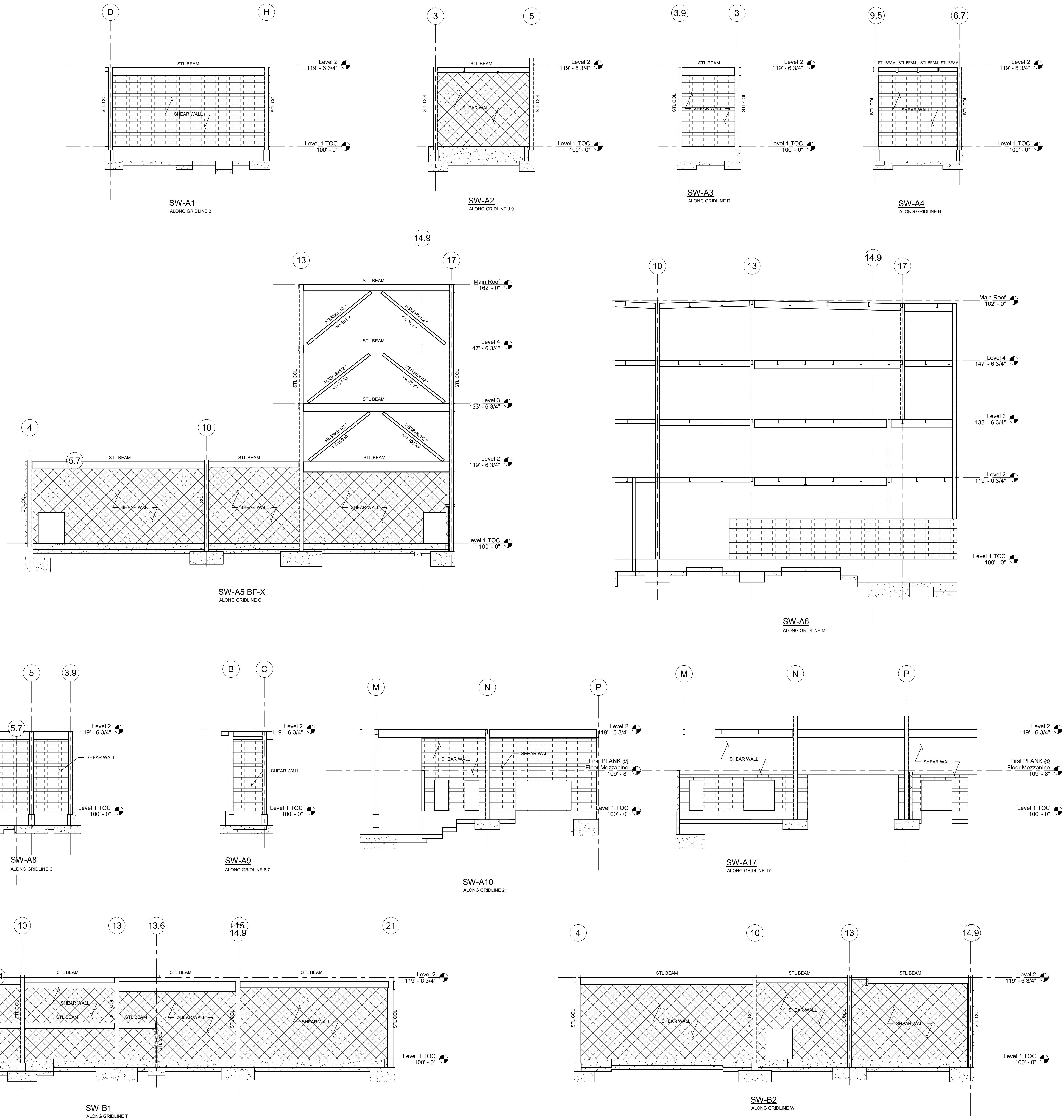
Date: August 28th, 2023

S4-0-4

- FIREPROOFING NOTES:**
1. STEEL COLUMNS SHOWN ON THIS DRAWING SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING UNLESS INDICATED AS A ROUND HSS COLUMN. EXPOSED TO VIEW PORTION OF ROUND HSS COLUMNS SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING. CONCEALED FROM VIEW PORTION OF ROUND HSS COLUMN SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
 2. CONCEALED FROM VIEW BRACED FRAME MEMBERS SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
 3. EXPOSED TO VIEW BRACED FRAME MEMBERS SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING.
 4. COORDINATE FIREPROOFING REQUIREMENTS WITH THE SPECIFICATIONS AND ARCHITECTURAL DRAWINGS.

- BRACE FRAME NOTES:**
- 1.) FABRICATOR IS RESPONSIBLE FOR BRACE CONNECTION DESIGN.
 - 2.) ELEVATIONS ARE SCHEMATIC ONLY AND INTENDED TO SHOW CONFIGURATION OF BRACED FRAMES AND BRACE FORCES.
 - 3.) DESIGN DIAGONAL MEMBER CONNECTIONS FOR TWICE THE AXIAL DESIGN FORCE SHOWN BELOW EACH MEMBER <30k> (TENSION OR COMPRESSION). AXIAL DESIGN FORCE IS BASED ON LRFD. USE GENERAL UNIFORM FORCE METHOD FOR CONNECTION DESIGN.
 - 4.) DO NOT WELD TOP END OF DIAGONAL BRACE MEMBERS IN PLACE UNTIL FLOOR SLABS AND ROOFING ARE IN PLACE. WELDS MUST BE FULLY INSPECTED AND APPROVED PRIOR TO PLACING ANY CONCRETE OR INSTALLING OTHER MATERIALS THAT WOULD COVER THE CONNECTIONS.
 - 5.) BOLTED CONNECTIONS IN BRACED FRAMES SHALL BE DESIGNED AS SLIP CRITICAL CONNECTIONS.
 - 6.) SEE PLANS FOR COLUMN AND BEAM SIZES.
 - 7.) CENTERLINE OF BRACE MEMBER SHALL BE OFFSET FROM WORKPOINT LINE AS NOTED AS SHOWN ON ELEVATION.
 - 8.) * SYMBOL AT THE END OF A STEEL BEAM SIZE (HSS*, WF*, ETC.) DENOTES EXPOSED BRACE FRAME MEMBERS, (INCLUDING STEEL BEAMS AND COLUMNS) TO RECEIVE INTUMESCENT MASTIC FIREPROOFING.

- SHEAR WALL NOTES:**
1. ELEVATIONS ARE SCHEMATIC ONLY, AND INTENDED TO SHOW CONFIGURATION OF SHEAR WALLS.
 2. SEE PLANS FOR COLUMN AND BEAM SIZES.
 3. COORDINATE LOCATIONS OF OPENINGS WITHIN WALLS WITH ARCHITECTURAL DRAWINGS.
 4. SEE SCHEDULE ON S0-0.4 FOR REINFORCING REQUIREMENTS AT SHEAR WALLS.
 5. SEE DETAILS ON S0-0.4 FOR ADDITIONAL REINFORCING REQUIRED AT OPENINGS IN WALLS, CONTROL JOINT ARRANGEMENTS, ETC.
 6. SEE MASONRY UNTEL DETAIL AND SCHEDULE ON S0-0.4 FOR REINFORCING REQUIREMENTS.
 7. SEE DETAIL 6 ON S0-0.5 FOR REQUIREMENTS OF WELDABLE COUPLERS ALONG STRUCTURAL STEEL COLUMNS AND BEAMS AT CMU SHEAR WALLS.
 8. COORDINATE ADDITIONAL BOND-BEAMS AND REINFORCING REQUIRED WITHIN SHEAR WALLS WITH VARIOUS SECTIONS ON THE STRUCTURAL CONTRACT DRAWINGS.



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03/31/2023

EARLY STRUCTURAL BID PACKAGE

REVISION LIST

BID SET

August 28th, 2023

A B C D

KEY PLAN

PROJECT NORTH

MAGNETIC NORTH

SHEAR WALLS

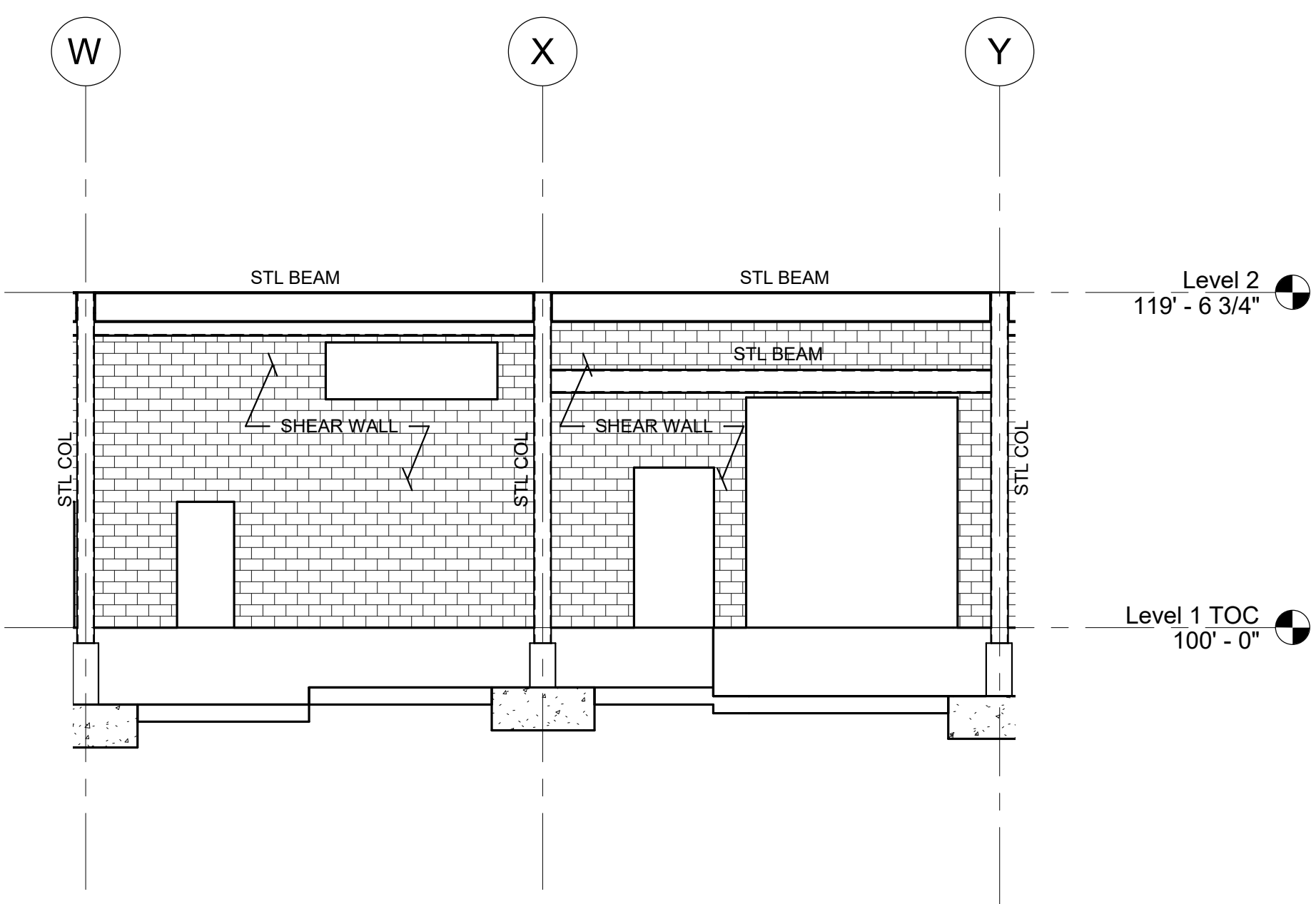
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Job No.: 20202

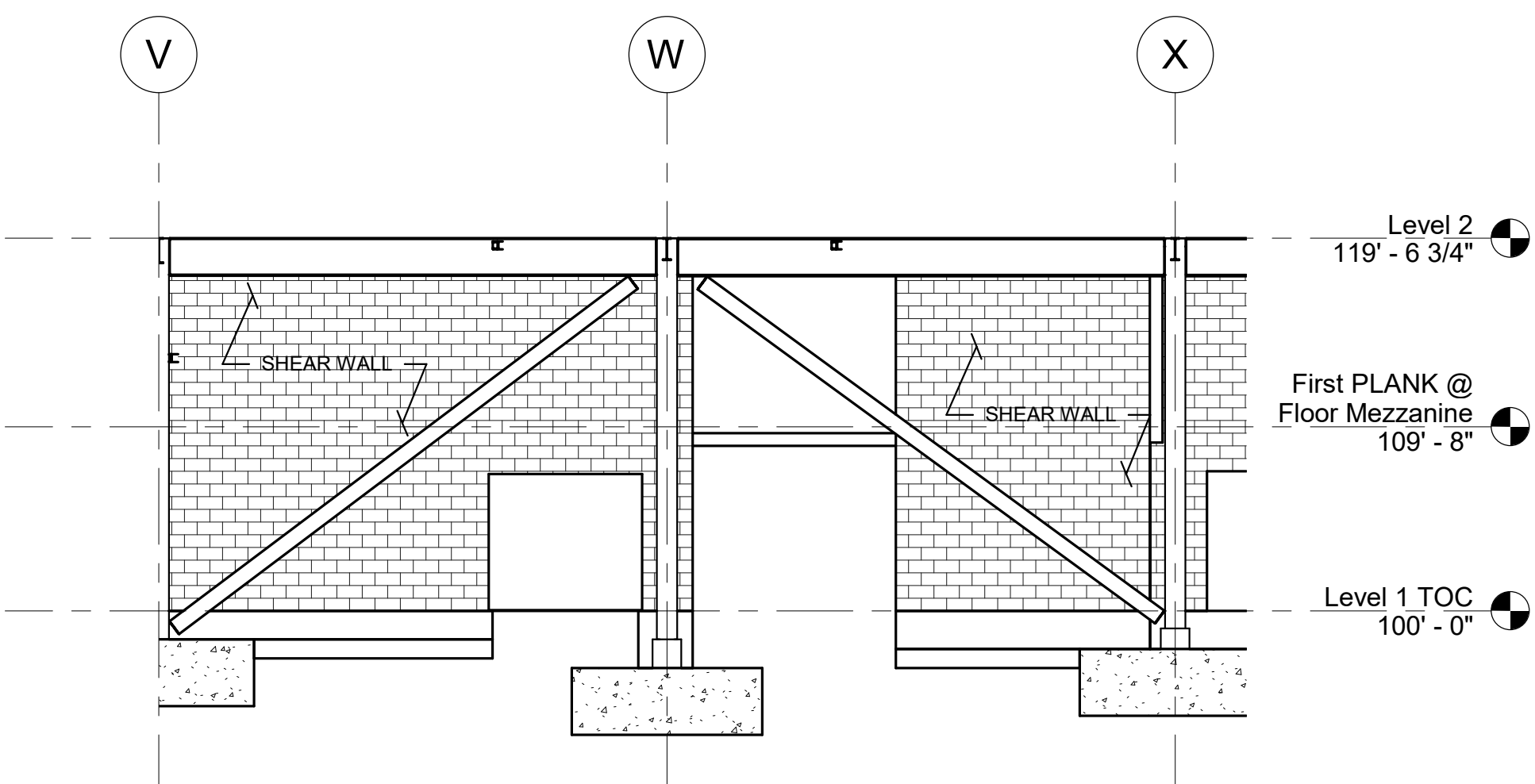
Drawn By: EDG

Date: August 28th, 2023

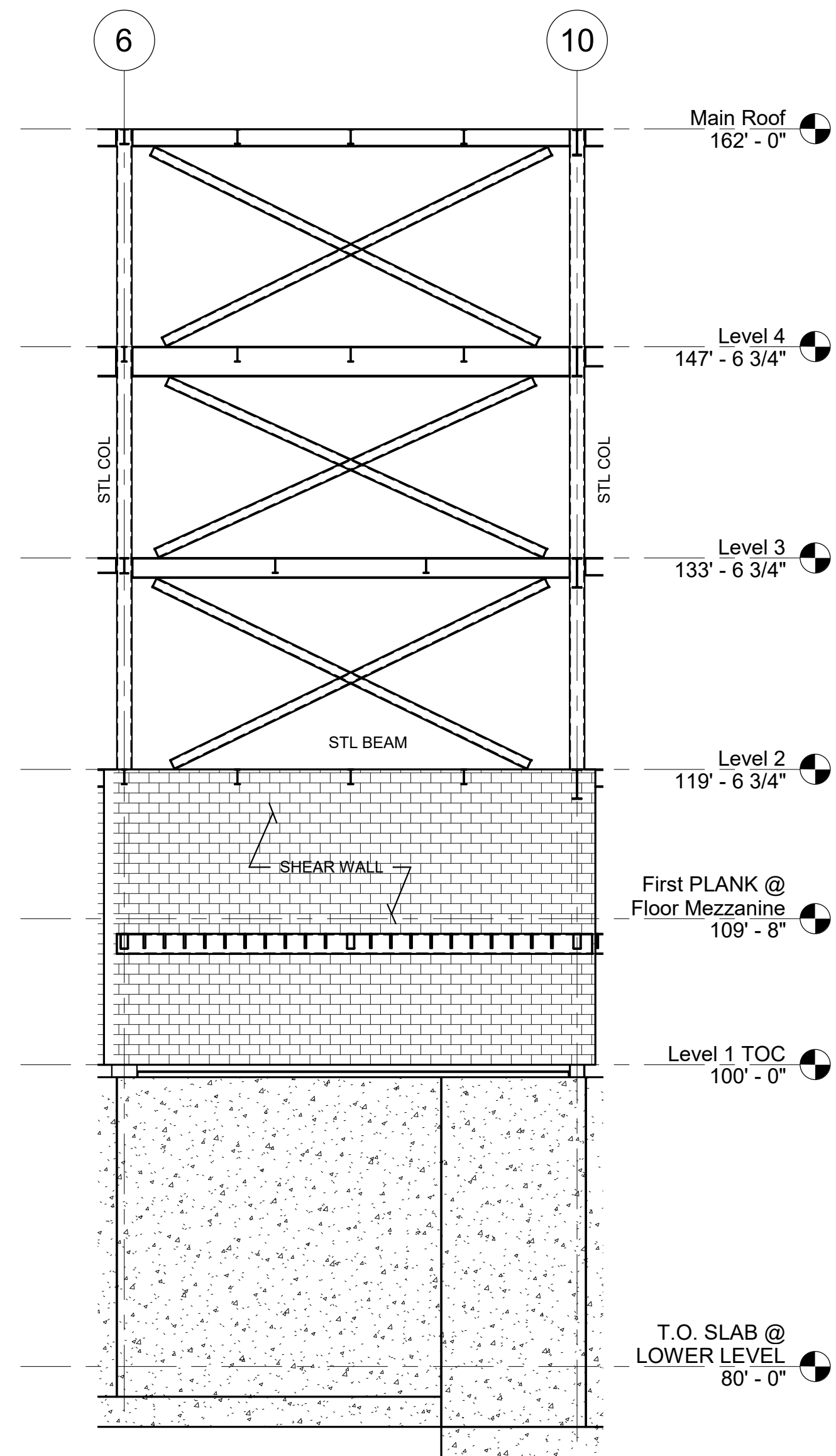
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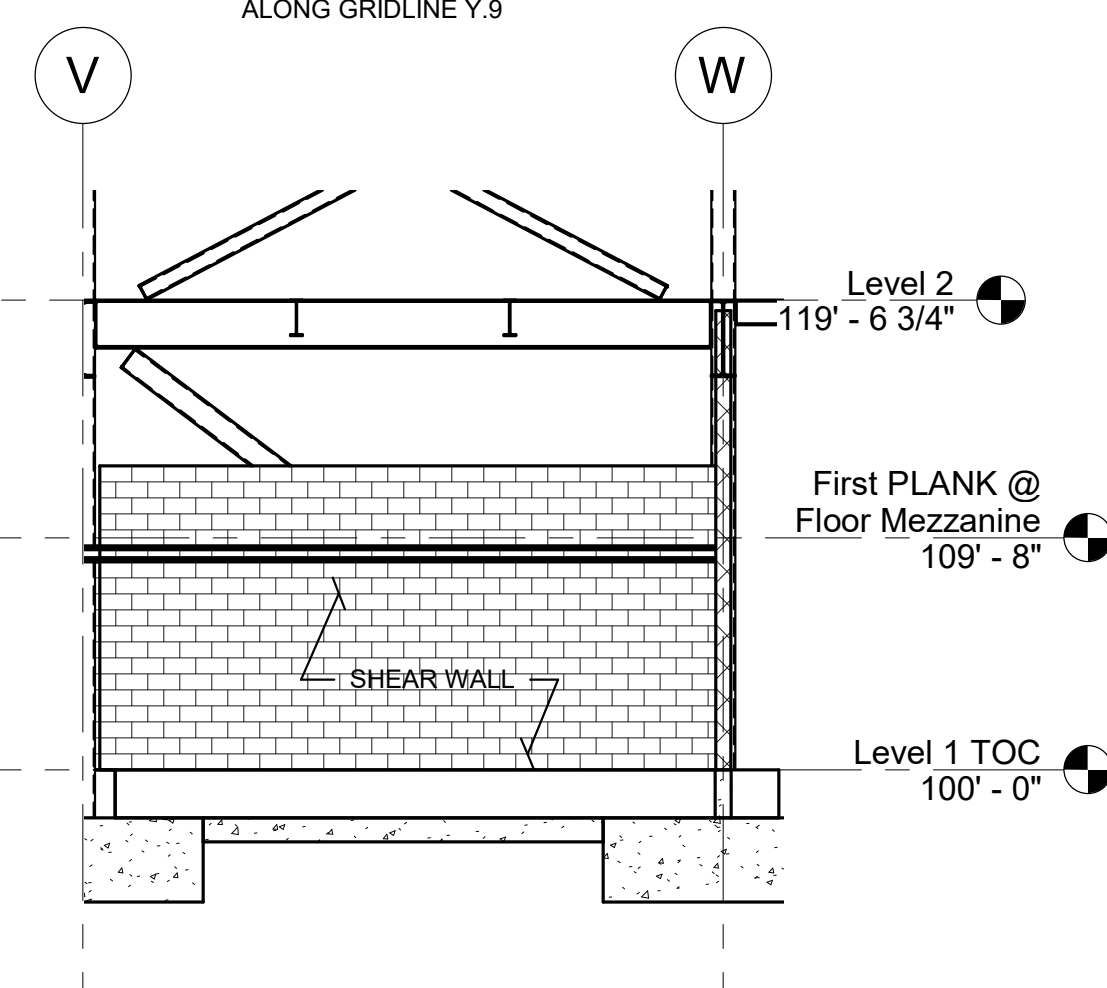
SW-B3
ALONG GRIDLINE 1



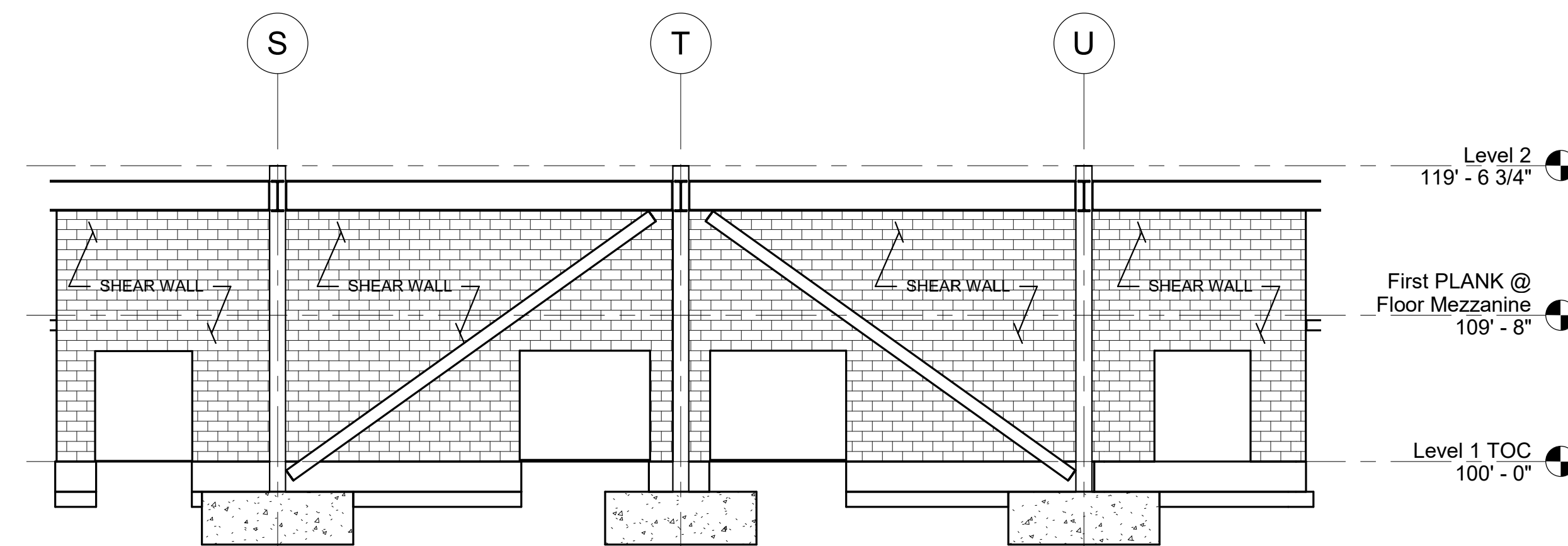
SW-B7
ALONG GRIDLINE 21



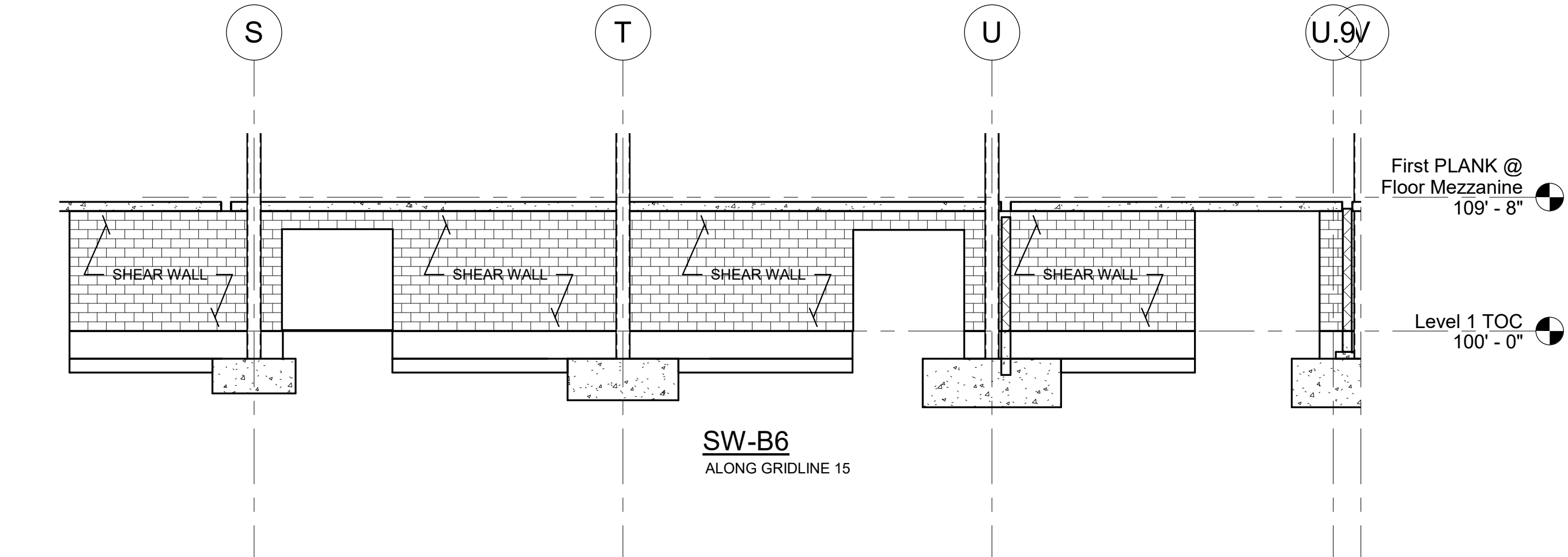
SW-B4
ALONG GRIDLINE Y.9



SW-B8
ALONG GRIDLINE 15



SW-B5
ALONG GRIDLINE 21

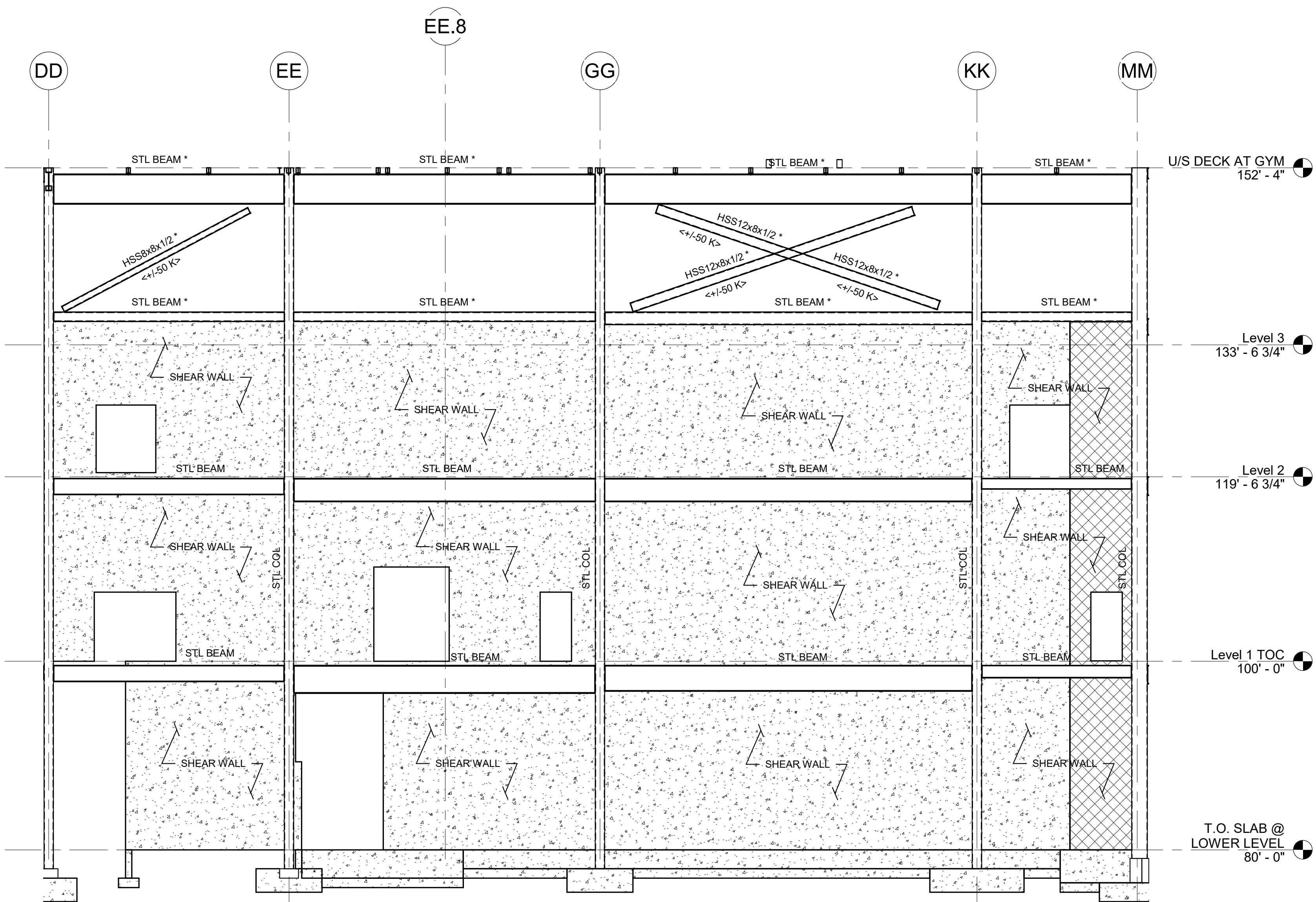


SW-B6
ALONG GRIDLINE 15

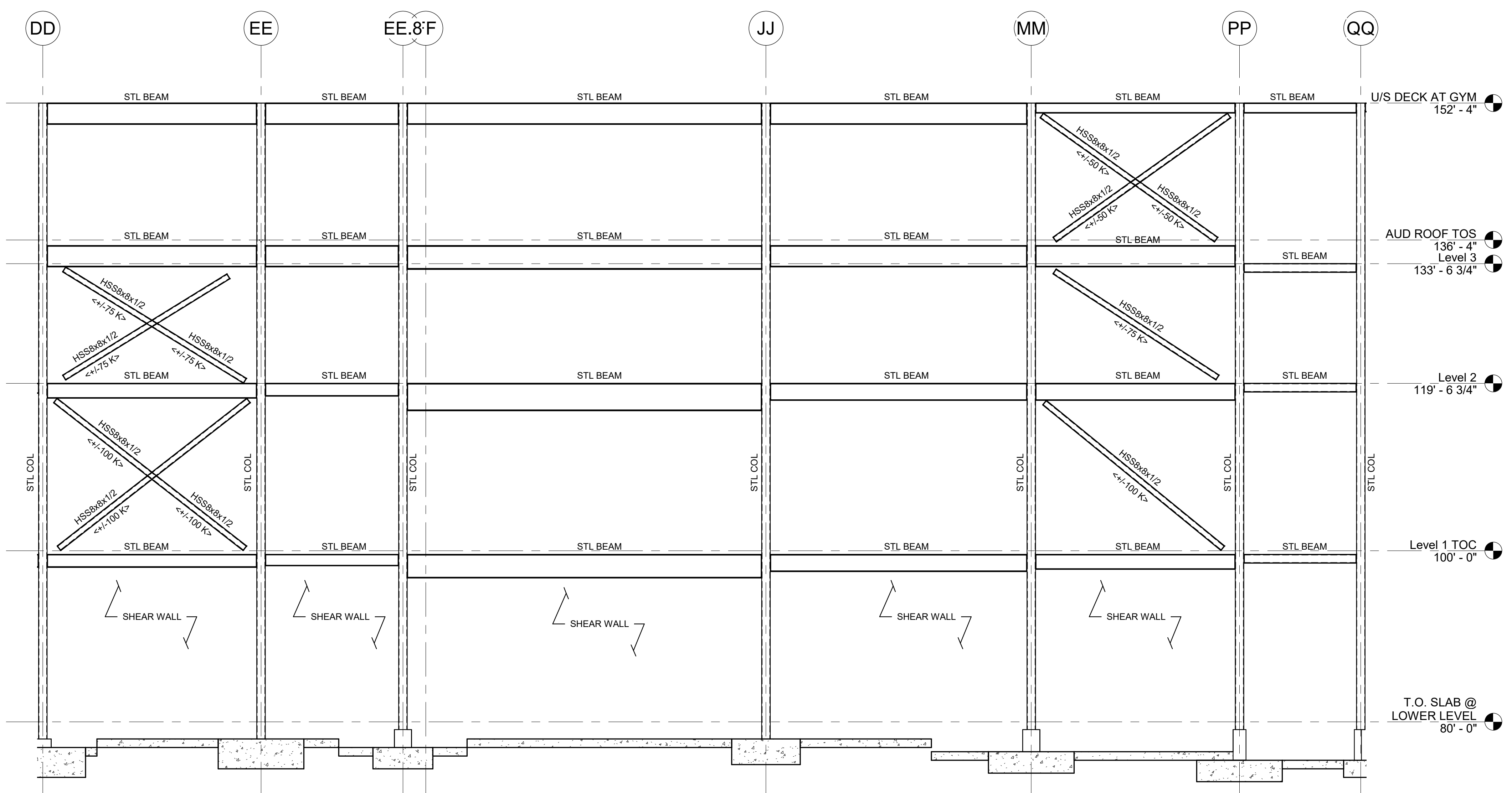
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1. STEEL COLUMNS SHOWN ON THIS DRAWING SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING UNLESS INDICATED AS A ROUND HSS COLUMN. EXPOSED TO VIEW PORTION OF ROUND HSS COLUMNS SHALL RECEIVE 2-HOUR FIRE RATING BY INTUMESCENT MASTIC FIREPROOFING. CONCEALED FROM VIEW PORTION OF ROUND HSS COLUMN SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
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 4. COORDINATE FIREPROOFING REQUIREMENTS WITH THE SPECIFICATIONS AND ARCHITECTURAL DRAWINGS.

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1. ELEVATIONS ARE SCHEMATIC ONLY AND INTENDED TO SHOW CONFIGURATION OF SHEAR WALLS.
 2. SEE PLANS FOR COLUMN AND BEAM SIZES.
 3. COORDINATE LOCATIONS OF OPENINGS WITHIN WALLS WITH ARCHITECTURAL DRAWINGS.
 4. SEE SCHEDULE ON S0-0.4 FOR REINFORCING REQUIREMENTS AT SHEAR WALLS.
 5. SEE DETAILS ON S0-0.4 FOR ADDITIONAL REINFORCING REQUIRED AT OPENINGS IN WALLS. CONTROL JOINT ARRANGEMENTS, ETC.
 6. SEE MASONRY LINTEL DETAIL AND SCHEDULE ON S0-0.4 FOR REINFORCING REQUIREMENTS.
 7. SEE DETAIL 6 ON S0-0.5 FOR REQUIREMENTS OF WELDABLE COUPLERS ALONG STRUCTURAL STEEL COLUMNS AND BEAMS AT CMU SHEAR WALLS.
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(SEE NOTE #7)
SW-C1
ALONG GRIDLINE 22



BF-C4 AND SW-C2
ALONG GRIDLINE 20

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03/31/2023 EARLY STRUCTURAL BID PACKAGE

REVISION LIST

BID SET

August 28th, 2023

KEY PLAN

PROJECT NORTH

MAGNETIC NORTH

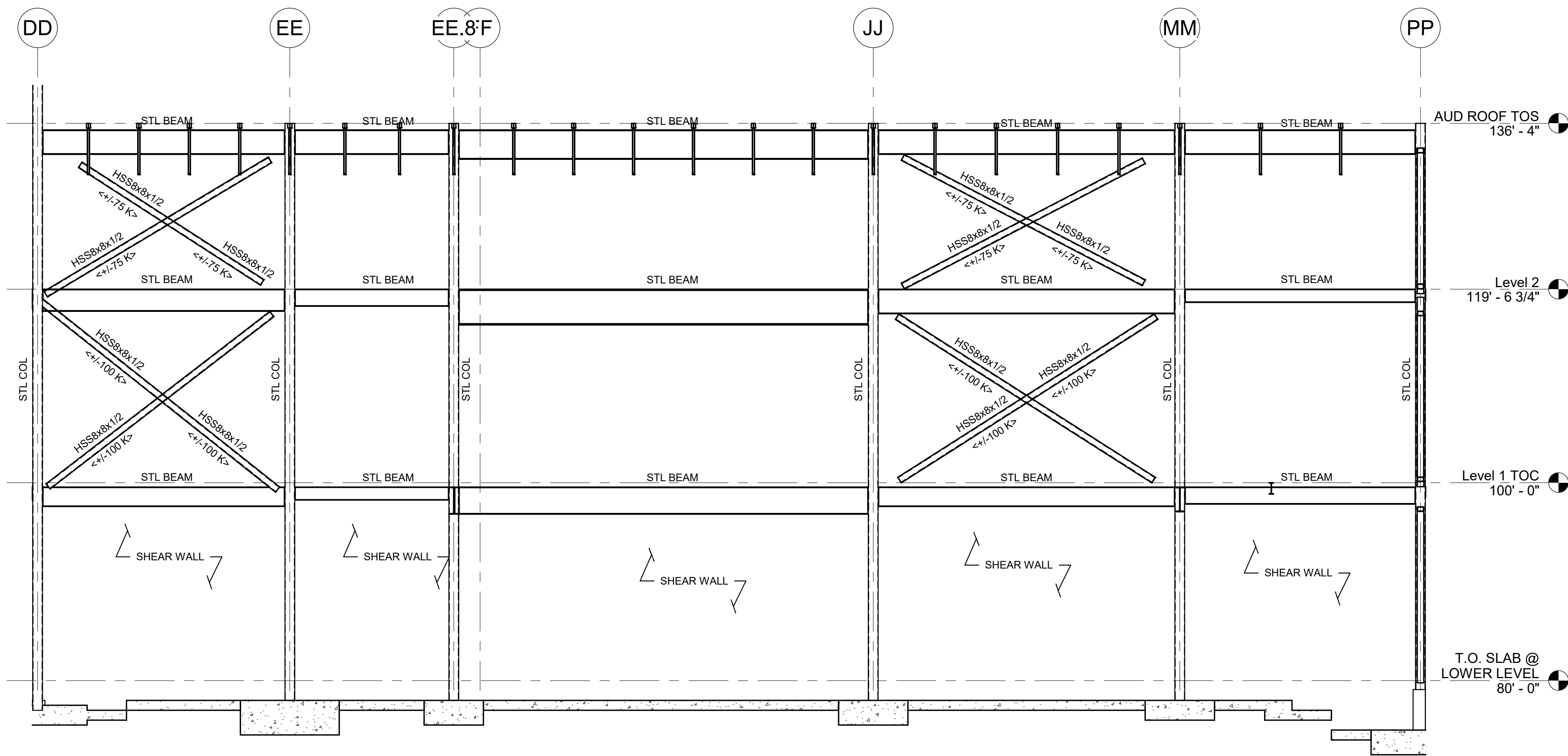
S4-0-6

Scale: 1/8" = 1'-0"

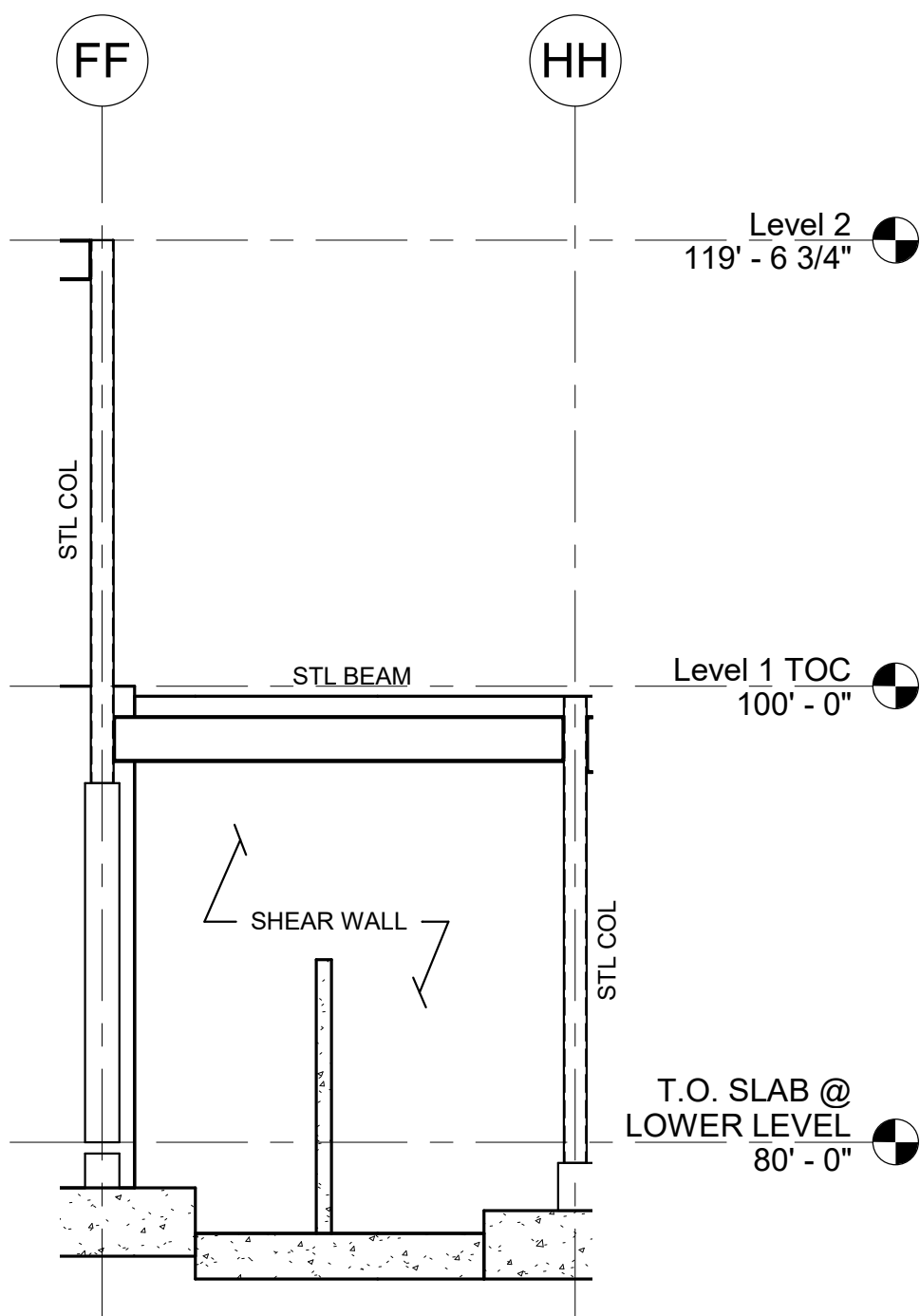
Job No.: 20202

Drawn By: EDG

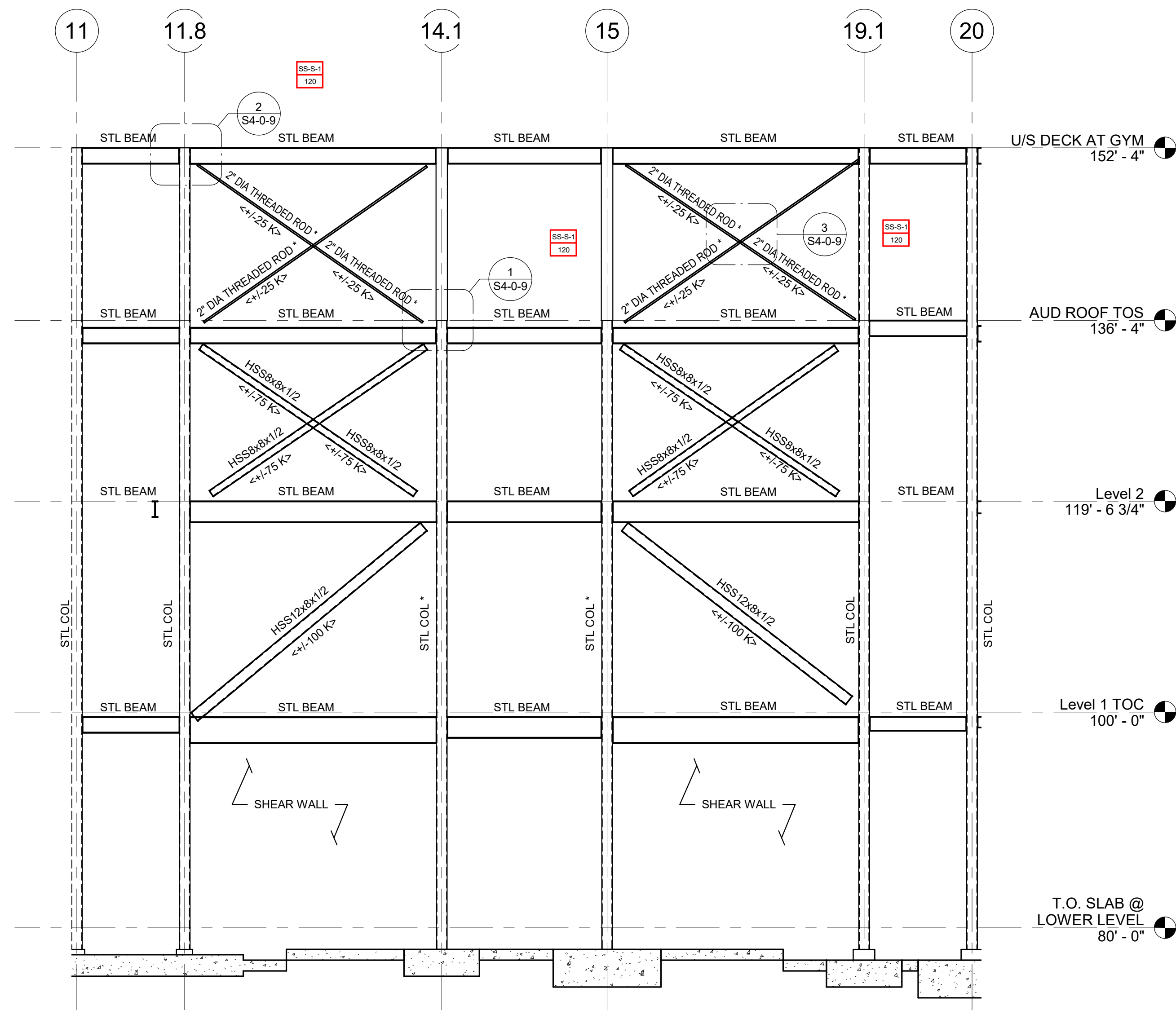
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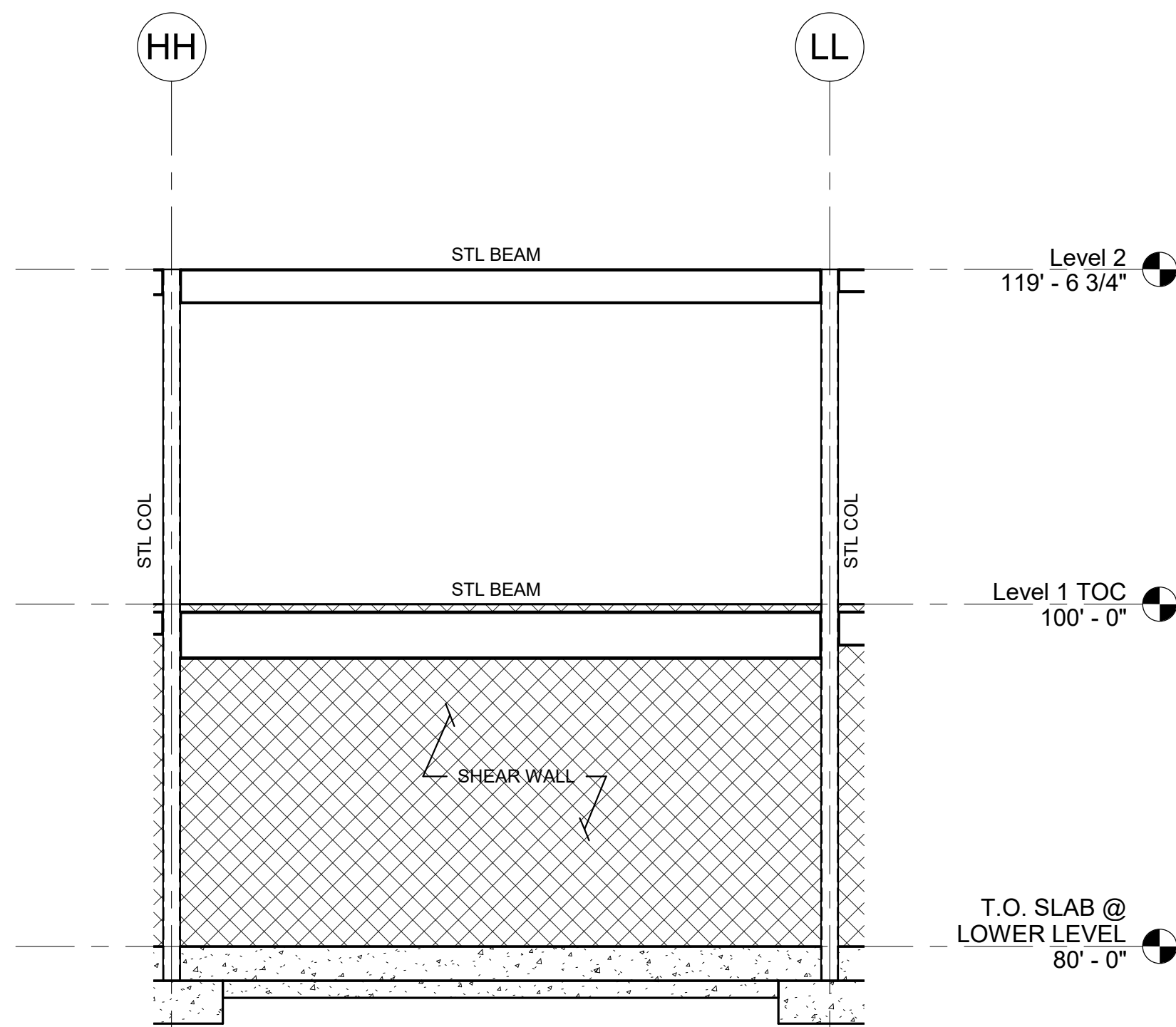
BF-C5 AND SW-C3
ALONG GRIDLINE 11



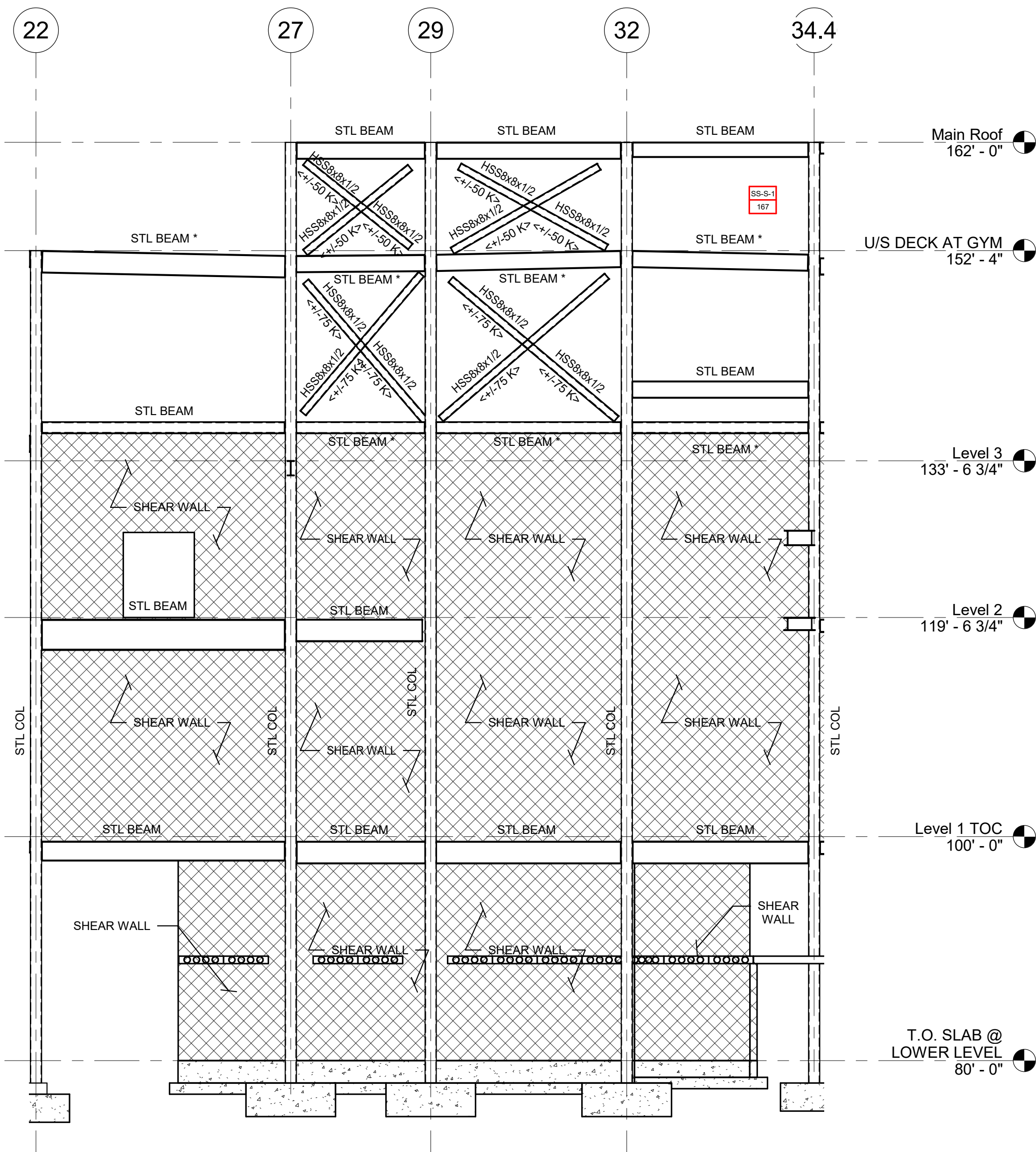
SW-C4
ALONG GRIDLINE 2



BF-C6 AND SW-C5
ALONG GRIDLINE DD



SW-C6
ALONG GRIDLINE 9



BF-D3 AND SW-D1
ALONG GRIDLINE DD

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- CONCEALED FROM VIEW BRACE FRAME MEMBERS SHALL RECEIVE 2-HOUR FIRE RATING BY CEMENTITIOUS FIREPROOFING.
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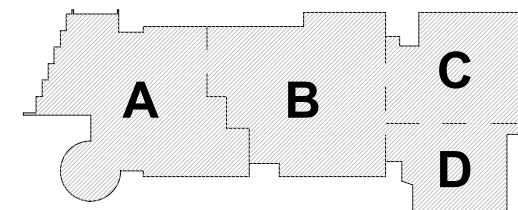
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REVISION LIST	
SS-S-1	4/14/2023 STRUCTURAL STEEL ADDENDUM 1

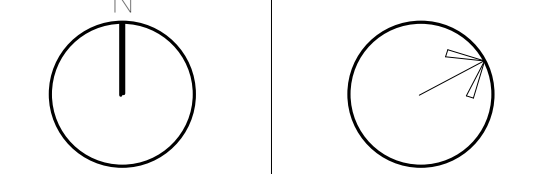
BID SET

August 28th, 2023



KEY PLAN

PROJECT NORTH MAGNETIC NORTH



SHEAR WALLS

Scale: 1/8" = 1'-0"
Job No.: 20202
Drawn By: EDG
Date: August 28th, 2023

S4-0-7

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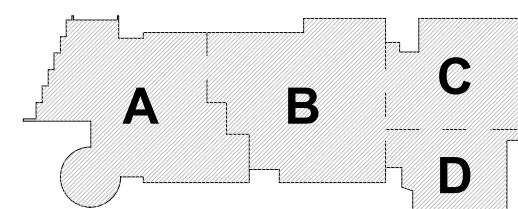
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03/31/2023	EARLY STRUCTURAL BID PACKAGE
REVISION LIST	
SS-S-1	4/14/2023 [STRUCTURAL STEEL ADDENDUM 1]

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BID SET

August 28th, 2023



KEY PLAN

PROJECT NORTH MAGNETIC NORTH

**BRACED FRAME
DETAILS**

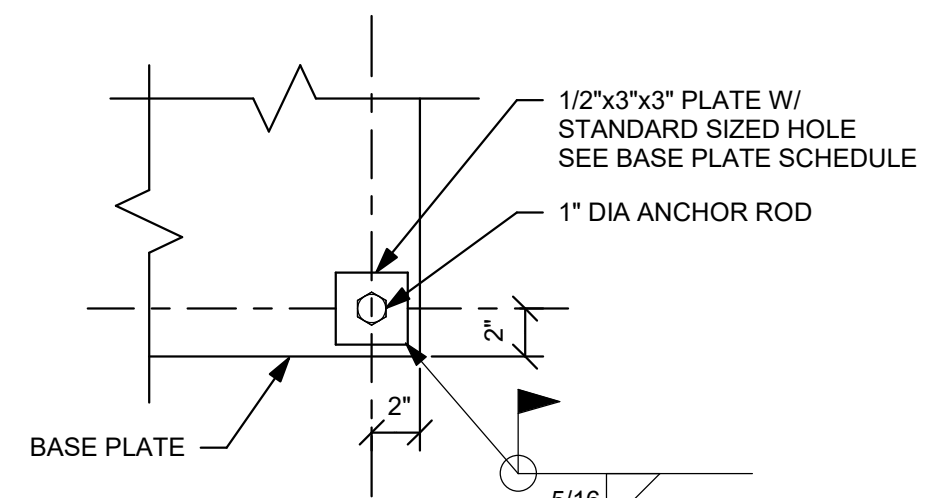
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Job No.: 20202

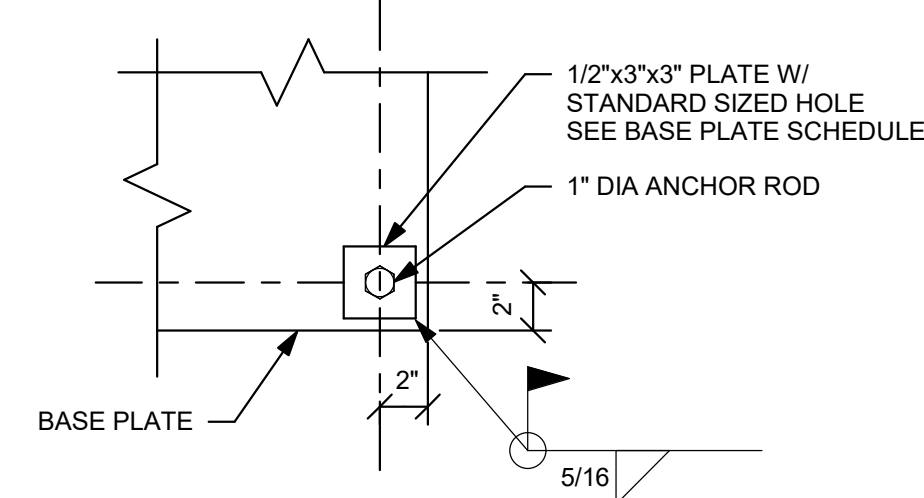
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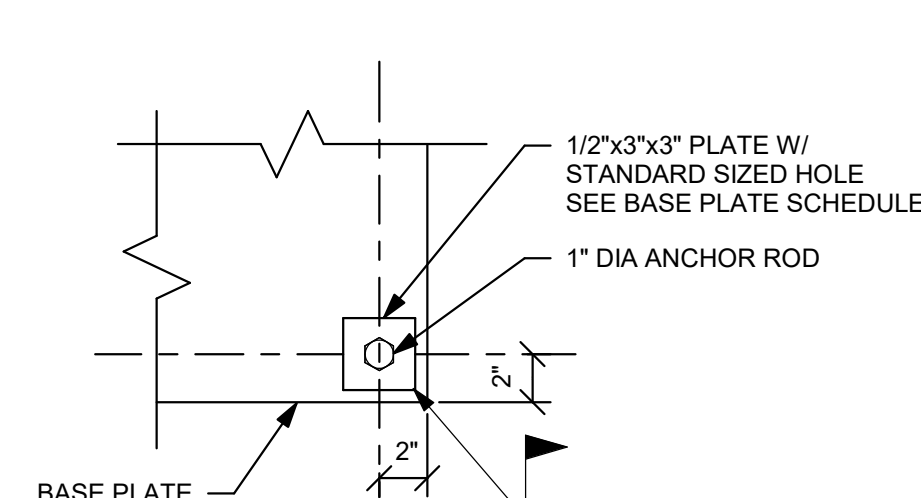
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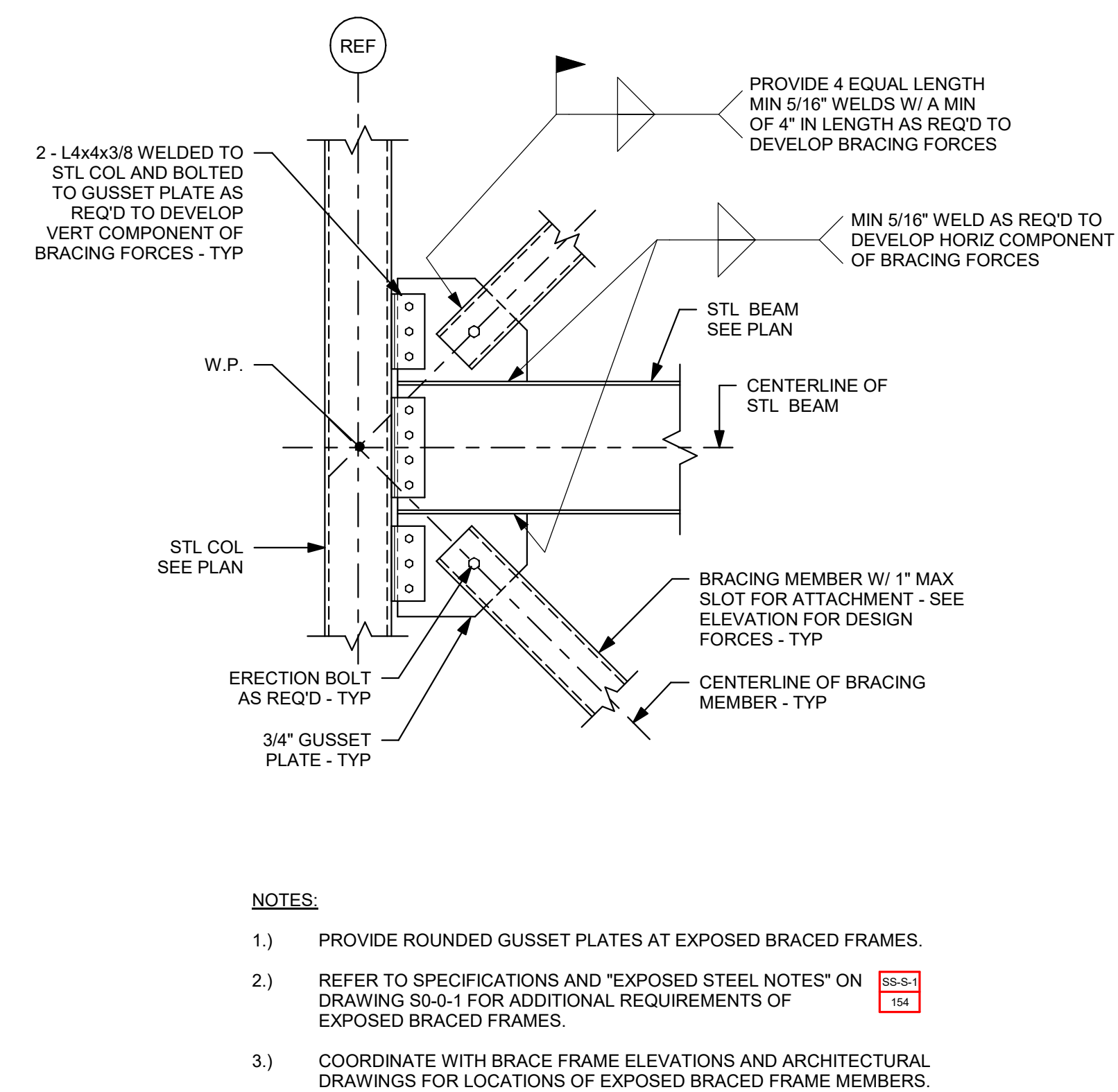
BASE PLATE DETAIL



BASE PLATE DETAIL



BASE PLATE DETAIL



NOTES:

- 1.) PROVIDE ROUNDED GUSSET PLATES AT EXPOSED BRACED FRAMES.
- 2.) REFER TO SPECIFICATIONS AND "EXPOSED STEEL NOTES" ON DRAWING S0-0-1 FOR ADDITIONAL REQUIREMENTS OF EXPOSED BRACED FRAMES.
- 3.) COORDINATE WITH BRACE FRAME ELEVATIONS AND ARCHITECTURAL DRAWINGS FOR LOCATIONS OF EXPOSED BRACED FRAME MEMBERS.

1

3/4" = 1'-0"

2

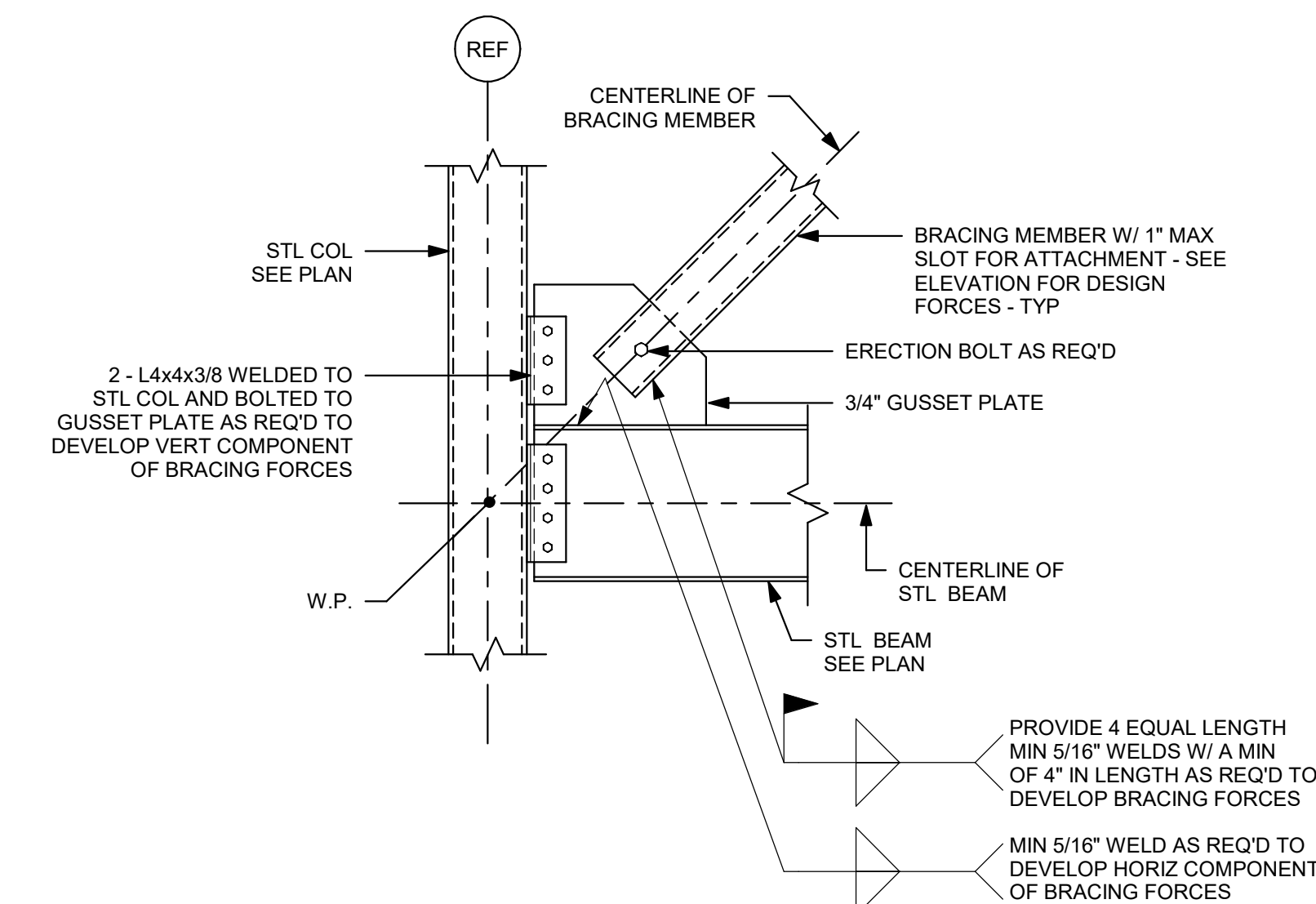
3/4" = 1'-0"

3

3/4" = 1'-0"

4

3/4" = 1'-0"



NOTES:

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5

3/4" = 1'-0"

6

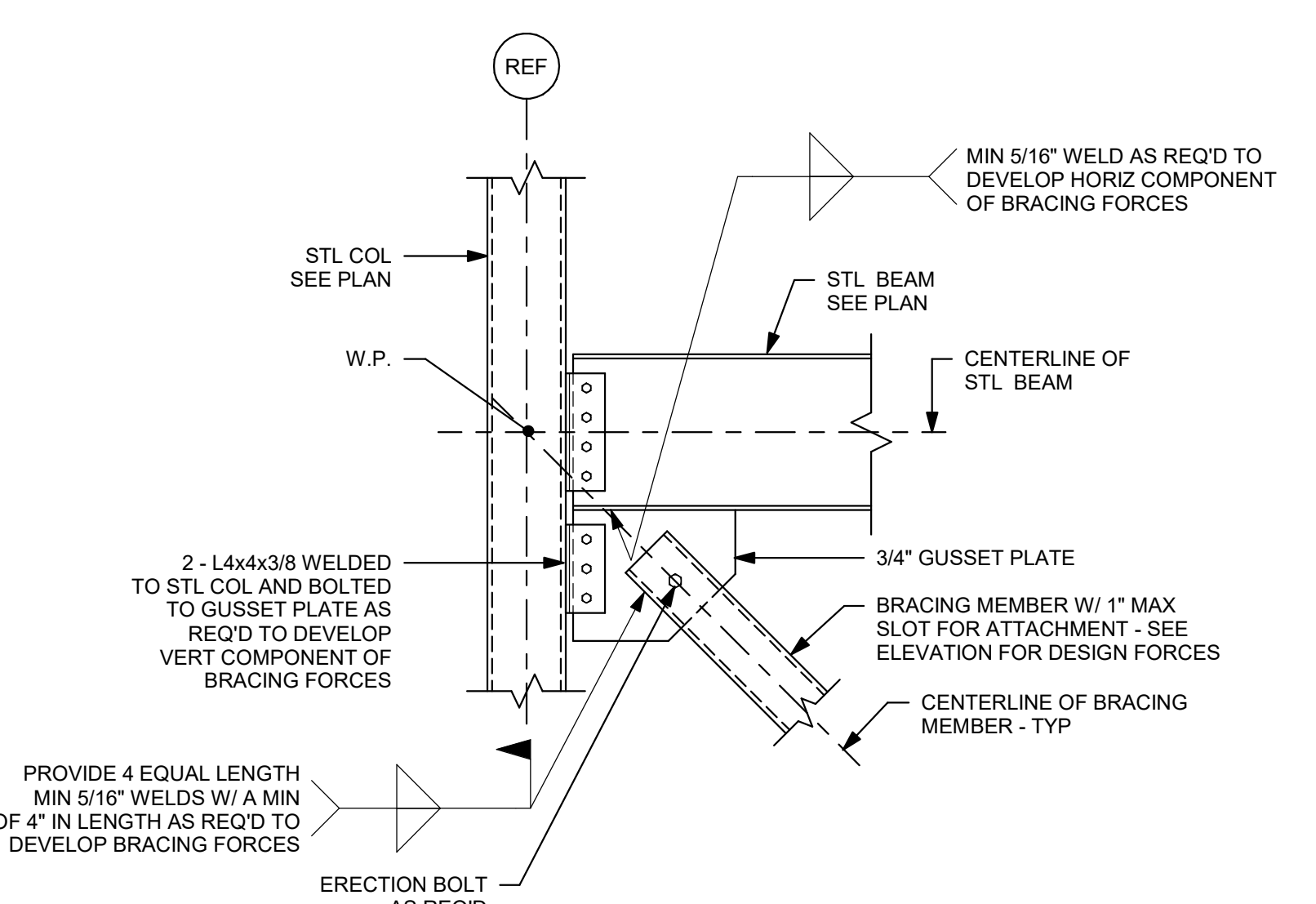
3/4" = 1'-0"

7

3/4" = 1'-0"

8

3/4" = 1'-0"



NOTES:

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9

3/4" = 1'-0"

10

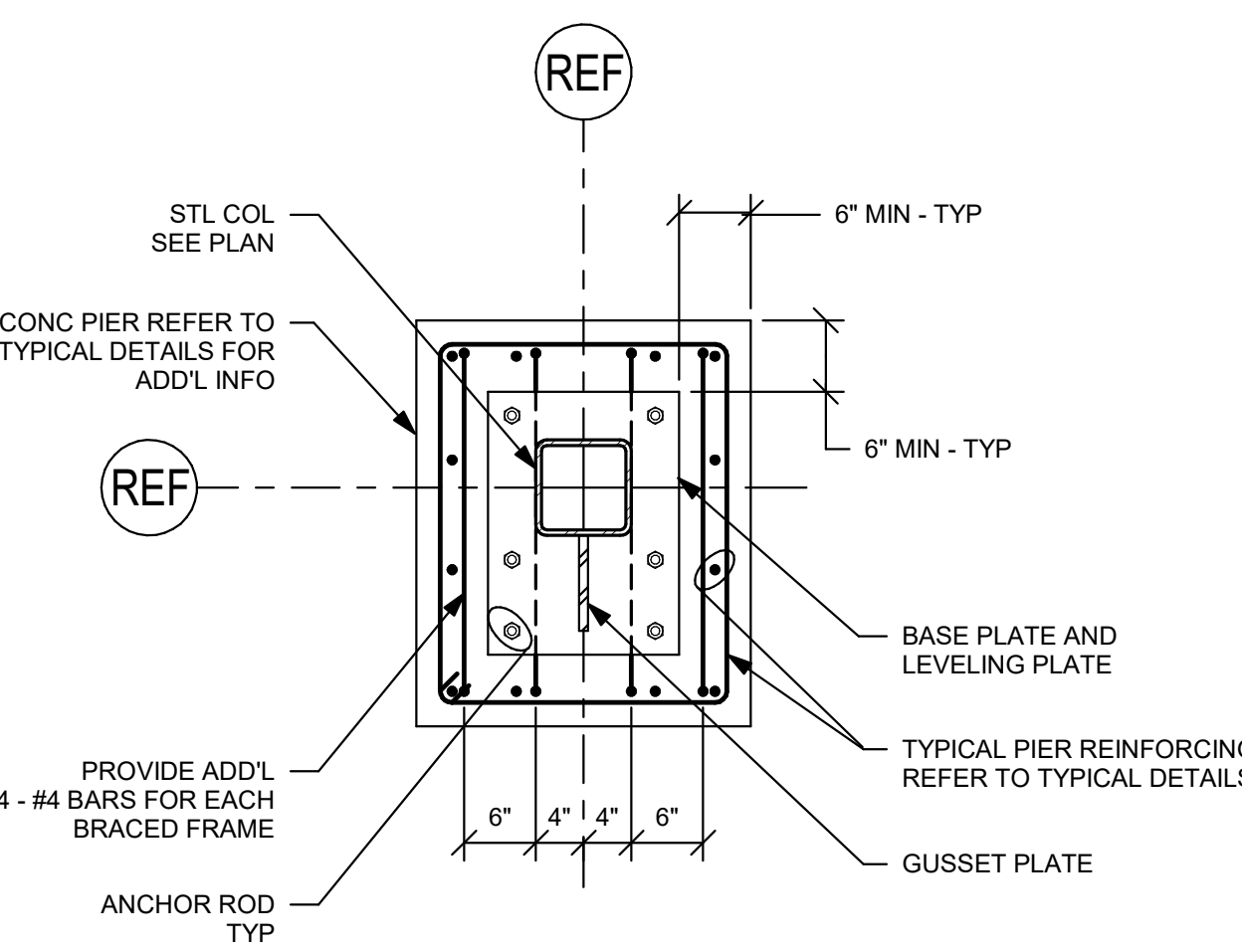
3/4" = 1'-0"

11

3/4" = 1'-0"

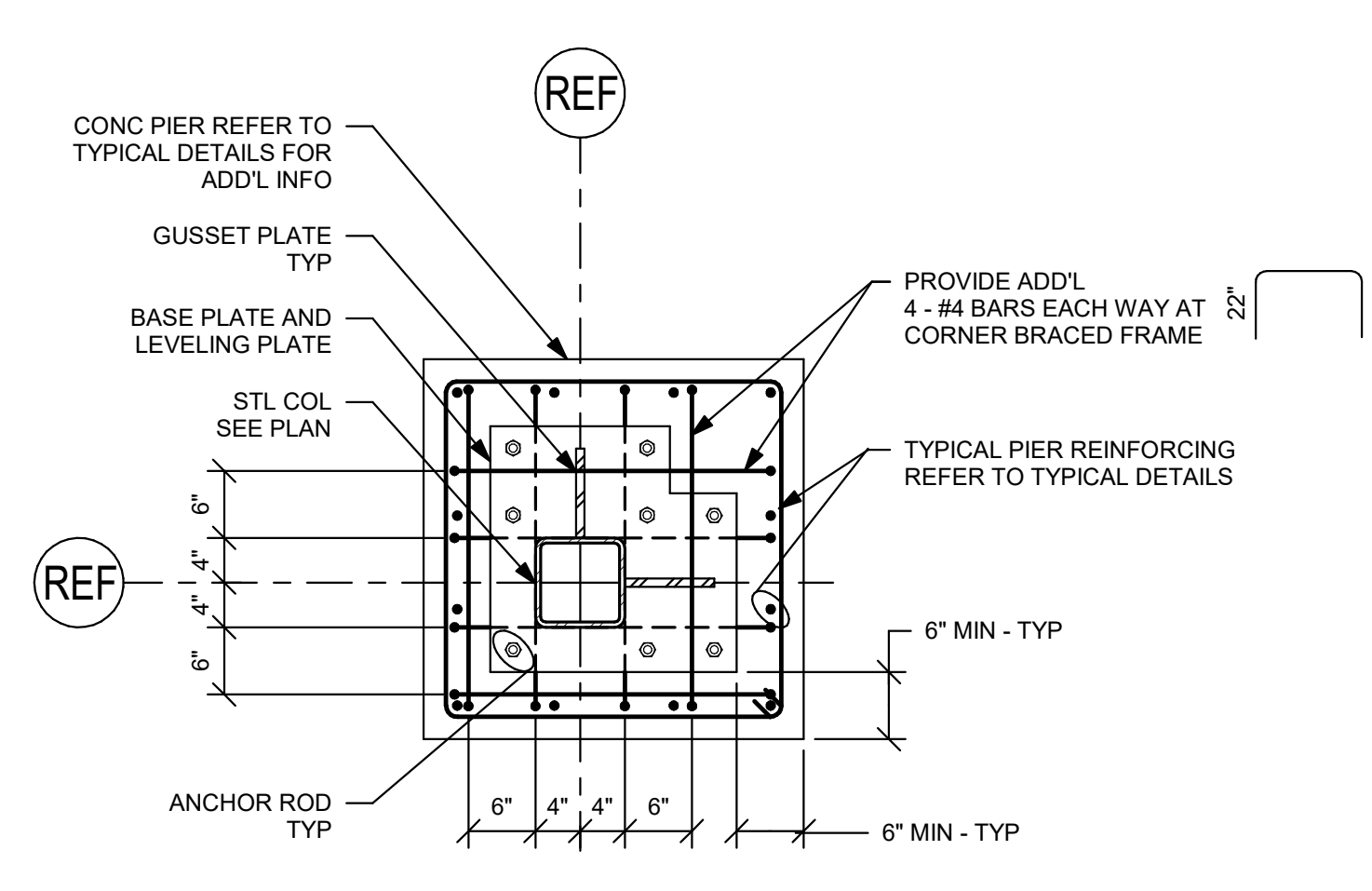
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3/4" = 1'-0"



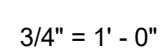
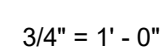
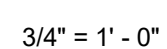
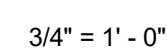
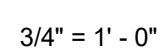
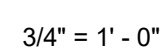
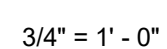
BRACE IN ONE DIRECTION AT PIER

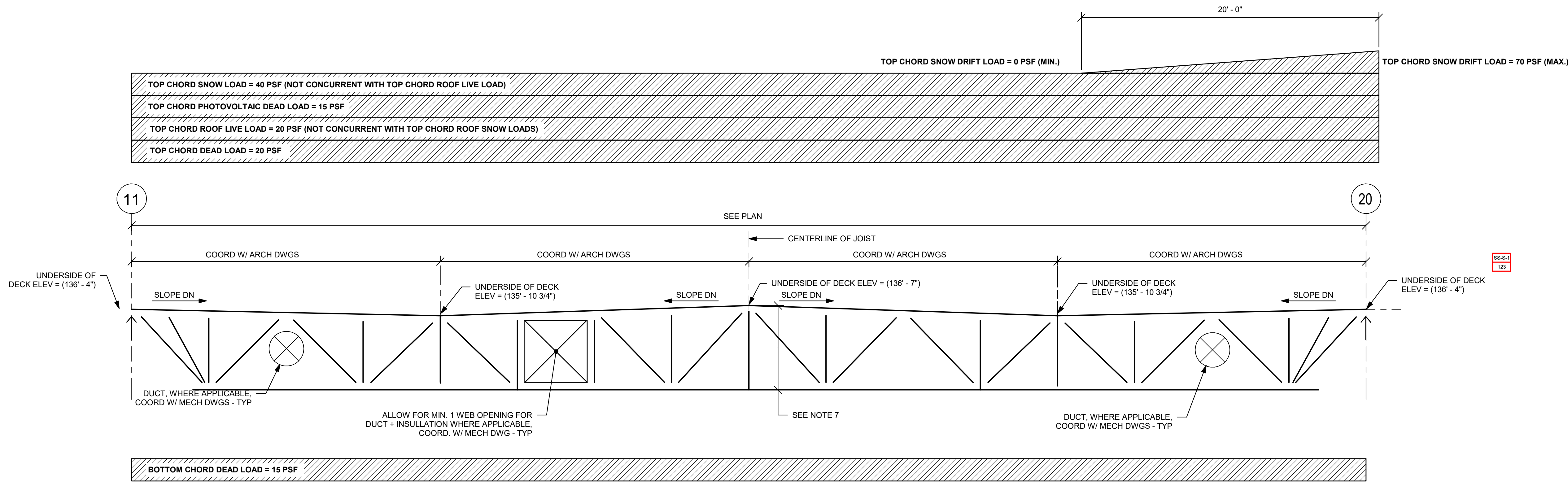
NOTE:
THIS DETAIL APPLIES TO BRACE FRAMED
COLUMNS BEARING DIRECTLY ON PIERS.



BRACE IN TWO DIRECTIONS AT PIER

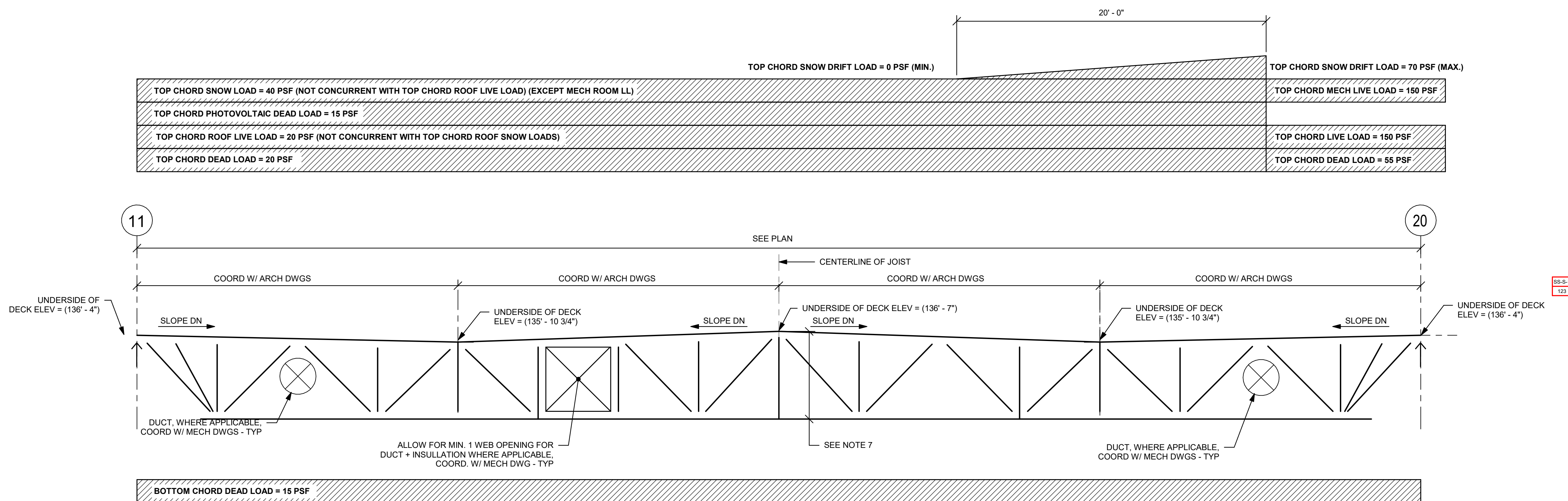
NOTE:
THIS DETAIL APPLIES TO BRACE FRAMED
COLUMNS BEARING DIRECTLY ON PIERS.



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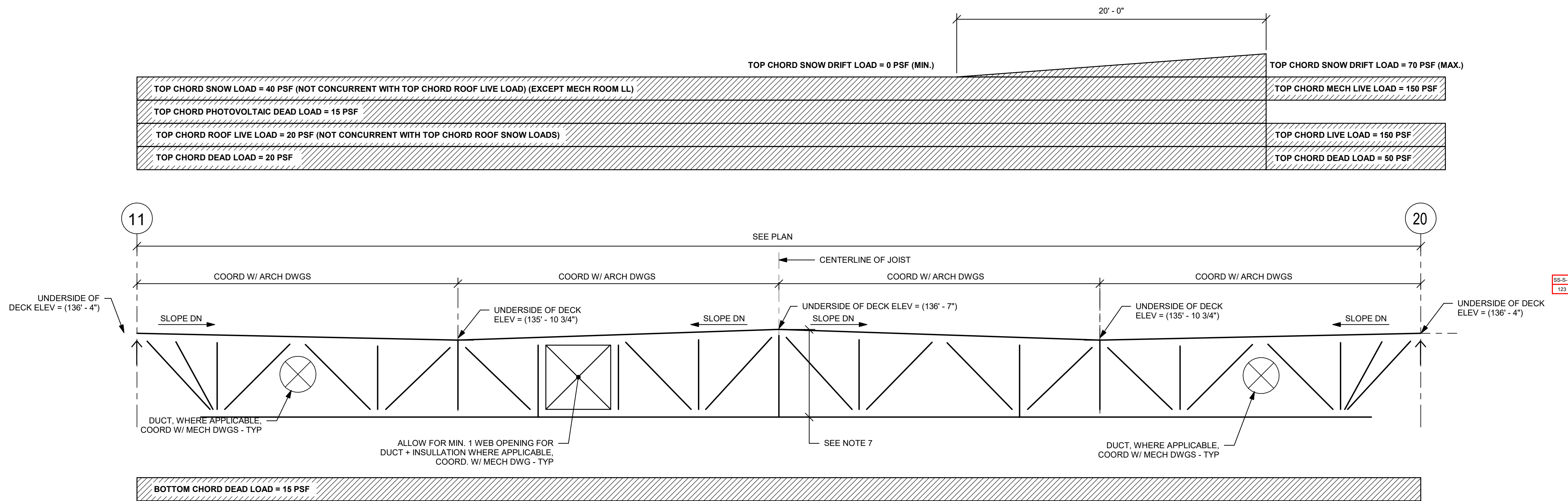
JOIST PROFILE AND LOAD DIAGRAM FOR 68DLHSP1

NOTE: SELF WEIGHT OF JOIST NOT INCLUDED IN LOADS



JOIST PROFILE AND LOAD DIAGRAM FOR 68DLHSP2

NOTE: SELF WEIGHT OF JOIST NOT INCLUDED IN LOADS



JOIST PROFILE AND LOAD DIAGRAM FOR 68DLHSP3

NOTE: SELF WEIGHT OF JOIST NOT INCLUDED IN LOADS

JOIST NOTES:

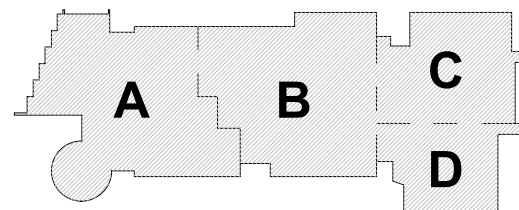
- JOIST SEAT DEPTH VARIES (8" MIN) AT CENTERLINE OF BEARING. COORDINATE WITH TOP OF STEEL BEAM.
- IN ADDITION TO THE LOADS SHOWN IN THE DIAGRAMS, DESIGN JOISTS AT THE GYMNASIUM FOR CONCENTRATED LOADS FROM MOTORIZED GYMNASIUM CURTAIN BATTING CAGE AND BASKETBALL BACKSTOP SUPPORT POINTS. REFER TO THE ARCHITECTURAL AND CEILING DRAWINGS. REFER TO MANUFACTURERS INFORMATION FOR LOAD MAGNITUDES AND LOCATIONS.
- REFER TO SPECIFICATIONS FOR UPLIFT LOAD ON THE JOISTS. DO NOT USE DESIGN DEAD LOAD TO OFFSET UPLIFT LOADS. ONLY SELF WEIGHT OF THE JOIST AND METAL ROOF DECK CAN BE USED TO OFFSET ANY UPLIFT LOADS.
- DESIGN FOR MAXIMUM LIVE LOADS DEFLECTION OF $L/360$.
- DESIGN ALL JOISTS FOR ADDITIONAL UPWARD LOAD OF 200 POUNDS AT FIRST PANEL POINT AT EACH END OF JOIST.
- IN ADDITION TO THE SLOPE, PROVIDE CAMBER PER SJI.
- JOIST DEPTH DESIGNATION INDICATED AT THE HIGHEST POINT (AT MID-SPAN).
- JOIST SHALL BE TOP CHORD, QUADRUPLE PITCHED UNDER-SLUNG JOIST. JOISTS ARE SYMMETRICAL ABOUT THE MIDSPAN.
- JOIST WEB CONFIGURATION IS BY JOIST SUPPLIER. WEB CONFIGURATION SHALL BE COMPATIBLE WITH MECHANICAL DUCT LAYOUT AND CATWALK SUPPORTS.
- JOIST MANUFACTURER SHALL DESIGN AND ACCOUNT FOR JOIST CAMBER AND JOIST DEFLECTION TO LIMIT DIFFERENTIAL DEFLECTION OF ADJACENT JOISTS TO ALLOW FOR PROPER INSTALLATION OF MULTIPLE SPAN ROOF DECK WITHOUT FIELD CUTTING OF DECK. REDUCE CAMBER BY HALF AT JOISTS ADJACENT TOP STRUCTURAL STEEL FRAMING.
- ALIGN PANEL POINTS OF ALL JOISTS AS SHOWN IN JOIST PROFILES.

03/31/2023 EARLY STRUCTURAL BID PACKAGE

REVISION LIST
SS-S-1 4/14/2023 STRUCTURAL STEEL ADDENDUM 1

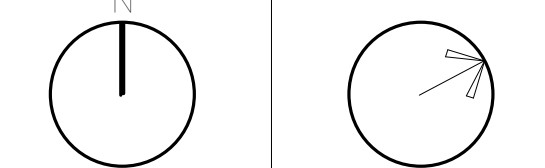
BID SET

August 28th, 2023



KEY PLAN

PROJECT NORTH MAGNETIC NORTH

JOIST LOADING
DIAGRAMS

Scale: As indicated

Job No.: 20202

Drawn By: EDG

Date: August 28th, 2023

S5-0-1

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Studio 300
Waltham, MA 02453

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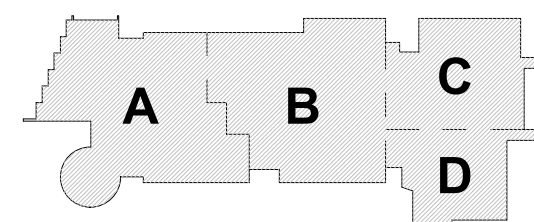
100 Hemlock Rd.
Wakefield, MA 01880

EDG
Engineers Design Group Inc.
Structural Engineers
389 Main Street, Suite 401
Malden, MA 02148
(781)396-9007
EDG@EDGINC.COM

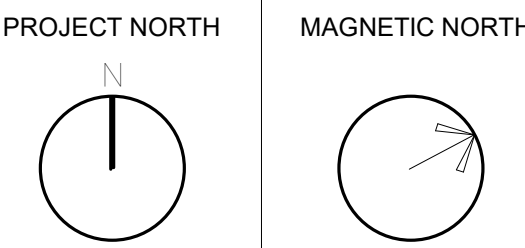
03/31/2023	EARLY STRUCTURAL BID PACKAGE
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SS-S-1	4/14/2023 STRUCTURAL STEEL ADDENDUM 1

BID SET

August 28th, 2023



KEY PLAN

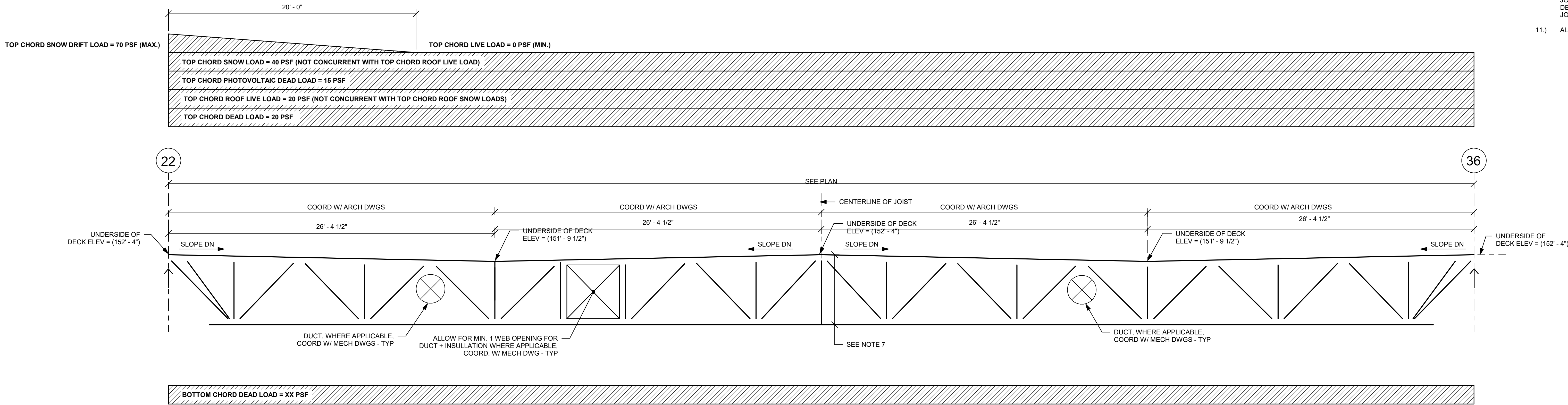


JOIST LOADING
DIAGRAMS

Scale: As indicated
Job No.: 20202
Drawn By: EDG
Date: August 28th, 2023

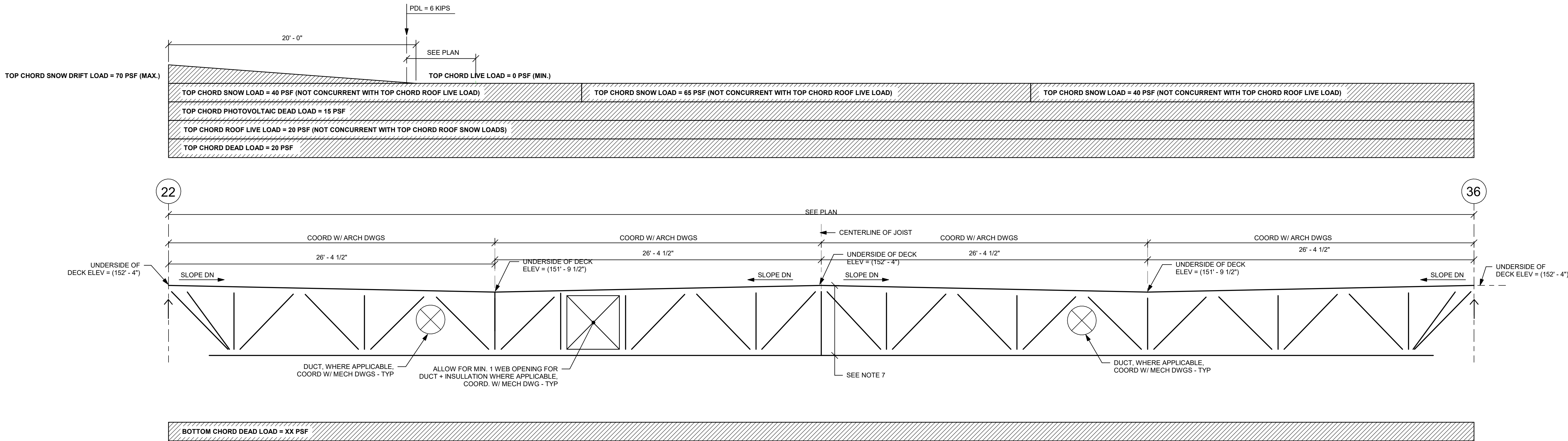
S5-0-2

- JOIST NOTES:
- JOIST SEAT DEPTH VARIES (8" MIN) AT CENTERLINE OF BEARING. COORDINATE WITH TOP OF STEEL BEAM.
 - IN ADDITION TO THE LOADS SHOWN IN THE DIAGRAMS, DESIGN JOISTS AT THE GYMNASIUM FOR CONCENTRATED LOADS FROM MOTORIZED GYMNASIUM CURTAIN BATTING CAGE AND BASKETBALL BACKSTOP SUPPORT POINTS. REFER TO THE ARCHITECTURAL AND CEILING DRAWINGS. REFER TO MANUFACTURERS INFORMATION FOR LOAD MAGNITUDES AND LOCATIONS.
 - REFER TO SPECIFICATIONS FOR UPLIFT LOAD ON THE JOISTS. DO NOT USE DESIGN DEAD LOAD TO OFFSET UPLIFT LOADS. ONLY SELF WEIGHT OF THE JOIST AND METAL ROOF DECK CAN BE USED TO OFFSET ANY UPLIFT LOADS.
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 - DESIGN ALL JOISTS FOR ADDITIONAL UPWARD LOAD OF 200 POUNDS AT FIRST PANEL POINT AT EACH END OF JOIST.
 - IN ADDITION TO THE SLOPE, PROVIDE CAMBER PER S.J.
 - JOIST DEPTH DESIGNATION INDICATED AT THE HIGHEST POINT (AT MID-SPAN). JOISTS ARE SYMMETRICAL ABOUT THE MIDSPAN.
 - JOIST SHALL BE TOP CHORD, QUADRUPLE PITCHED UNDER-SLUNG JOIST. JOISTS ARE SYMMETRICAL ABOUT THE MIDSPAN.
 - JOIST WEB CONFIGURATION IS BY JOIST SUPPLIER. WEB CONFIGURATION SHALL BE COMPATIBLE WITH MECHANICAL DUCT LAYOUT AND CATWALK SUPPORTS.
 - JOIST MANUFACTURER SHALL DESIGN AND ACCOUNT FOR JOIST CAMBER AND JOIST DEFLECTION TO LIMIT DIFFERENTIAL DEFLECTION OF ADJACENT JOISTS TO ALLOW FOR PROPER INSTALLATION OF MULTIPLE SPAN ROOF DECK WITHOUT FIELD CUTTING OF DECK. REDUCE CAMBER BY HALF AT JOISTS ADJACENT TOP STRUCTURAL STEEL FRAMING.
 - ALIGN PANEL POINTS OF ALL JOISTS AS SHOWN IN JOIST PROFILES.



JOIST PROFILE AND LOAD DIAGRAM FOR 68DLHSP4

NOTE: SELF WEIGHT OF JOIST IS NOT INCLUDED IN LOADS



JOIST PROFILE AND LOAD DIAGRAM FOR 60DLHSP5

NOTE: SELF WEIGHT OF JOIST IS NOT INCLUDED IN LOADS

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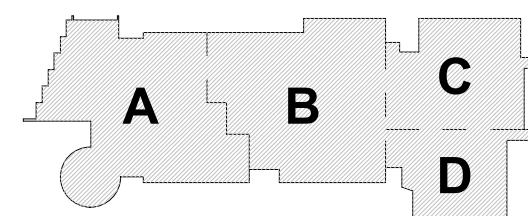


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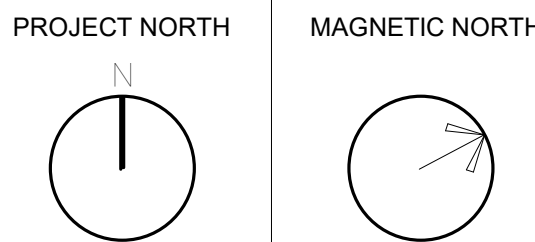
03/31/2023	EARLY STRUCTURAL BID PACKAGE
REVISION LIST	
SS-S-1	4/14/2023 STRUCTURAL STEEL ADDENDUM 1

BID SET

August 28th, 2023



KEY PLAN



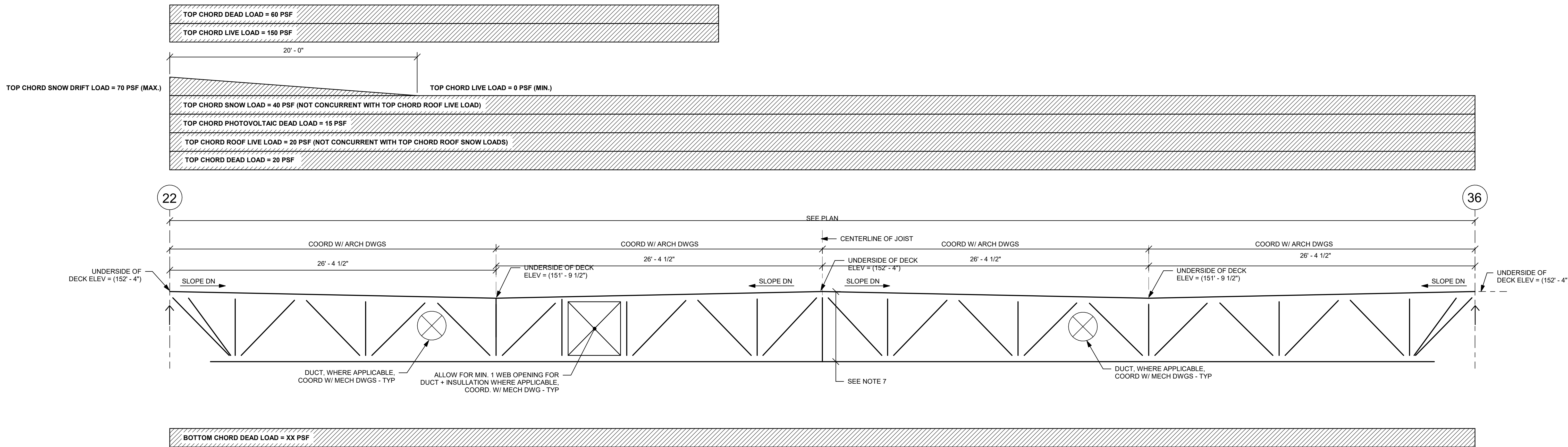
JOIST LOADING
DIAGRAMS

Scale: As indicated
Job No.: 20202
Drawn By: EDG
Date: August 28th, 2023

S5-0-3

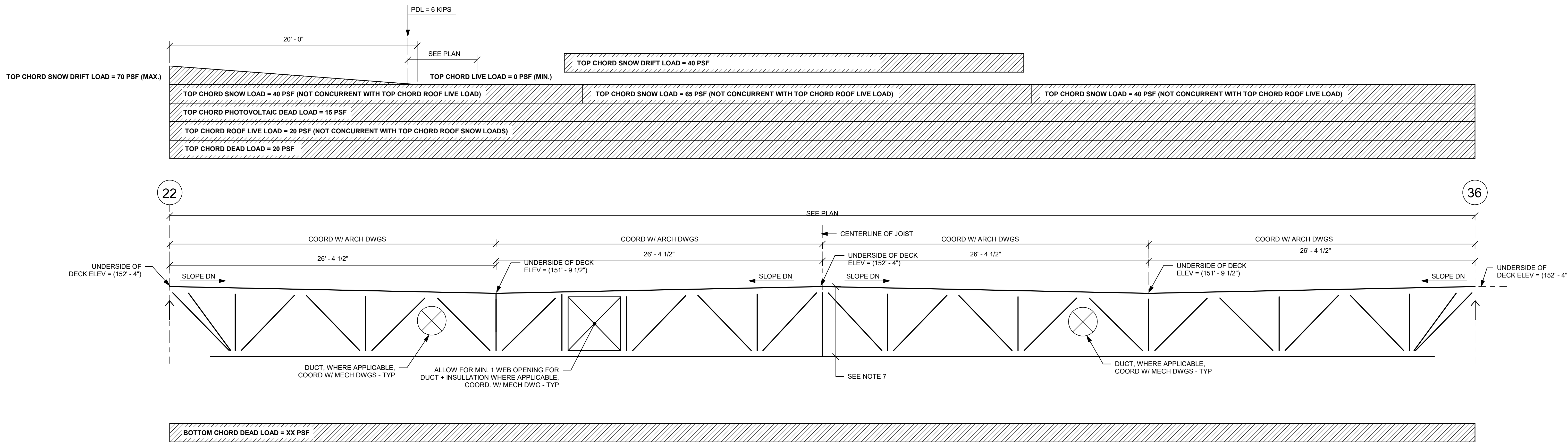
JOIST NOTES:

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- IN ADDITION TO THE LOADS SHOWN IN THE DIAGRAMS, DESIGN JOISTS AT THE GYMNASIUM FOR CONCENTRATED LOADS FROM MOTORIZED GYMNASIUM CURTAIN BATTING CAGE AND BASKETBALL BACKSTOP SUPPORT POINTS. REFER TO THE ARCHITECTURAL AND CEILING DRAWINGS. REFER TO MANUFACTURERS INFORMATION FOR LOAD MAGNITUDES AND LOCATIONS.
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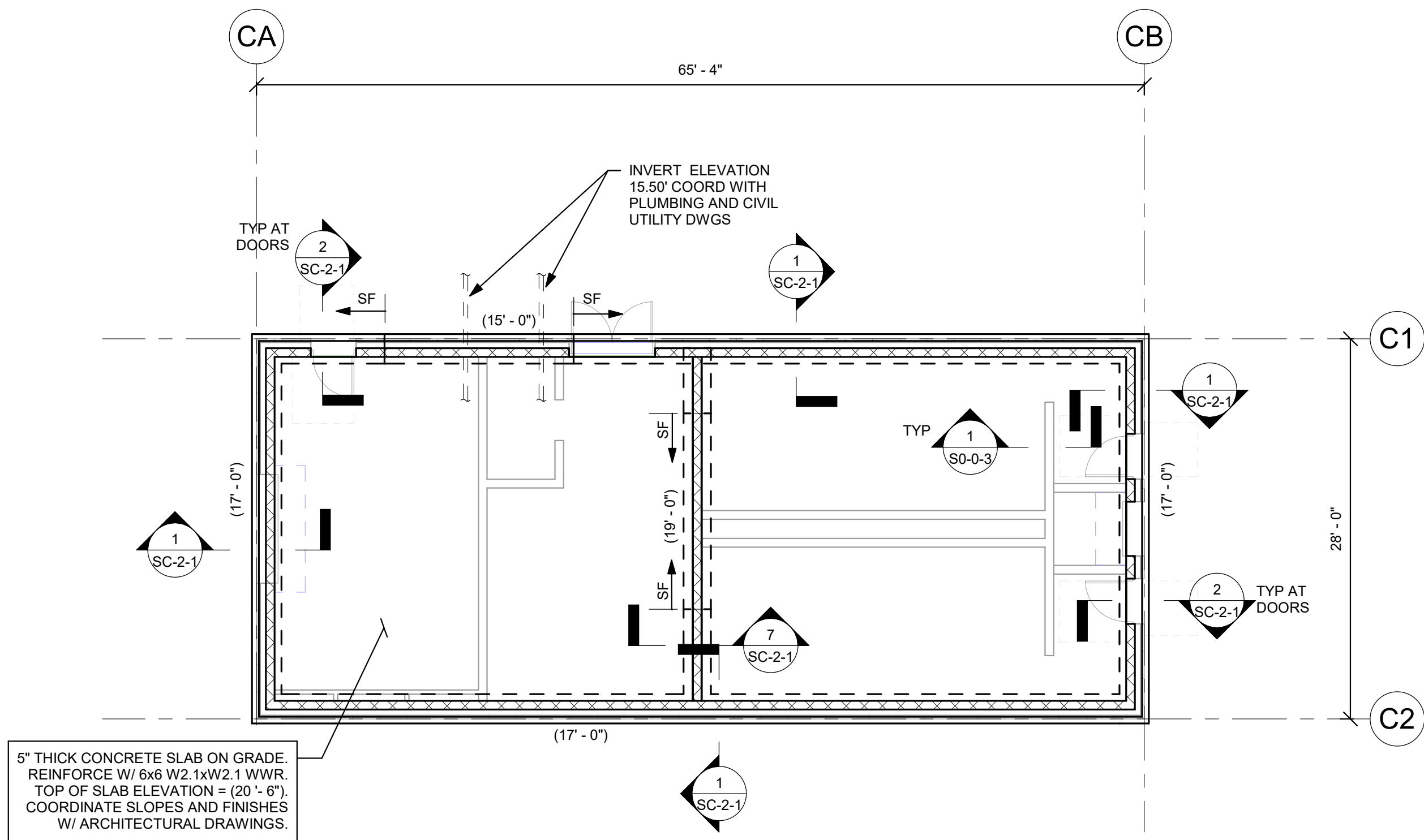
JOIST PROFILE AND LOAD DIAGRAM FOR 68DLHSP6

NOTE: SELF WEIGHT OF JOIST IS NOT INCLUDED IN LOADS

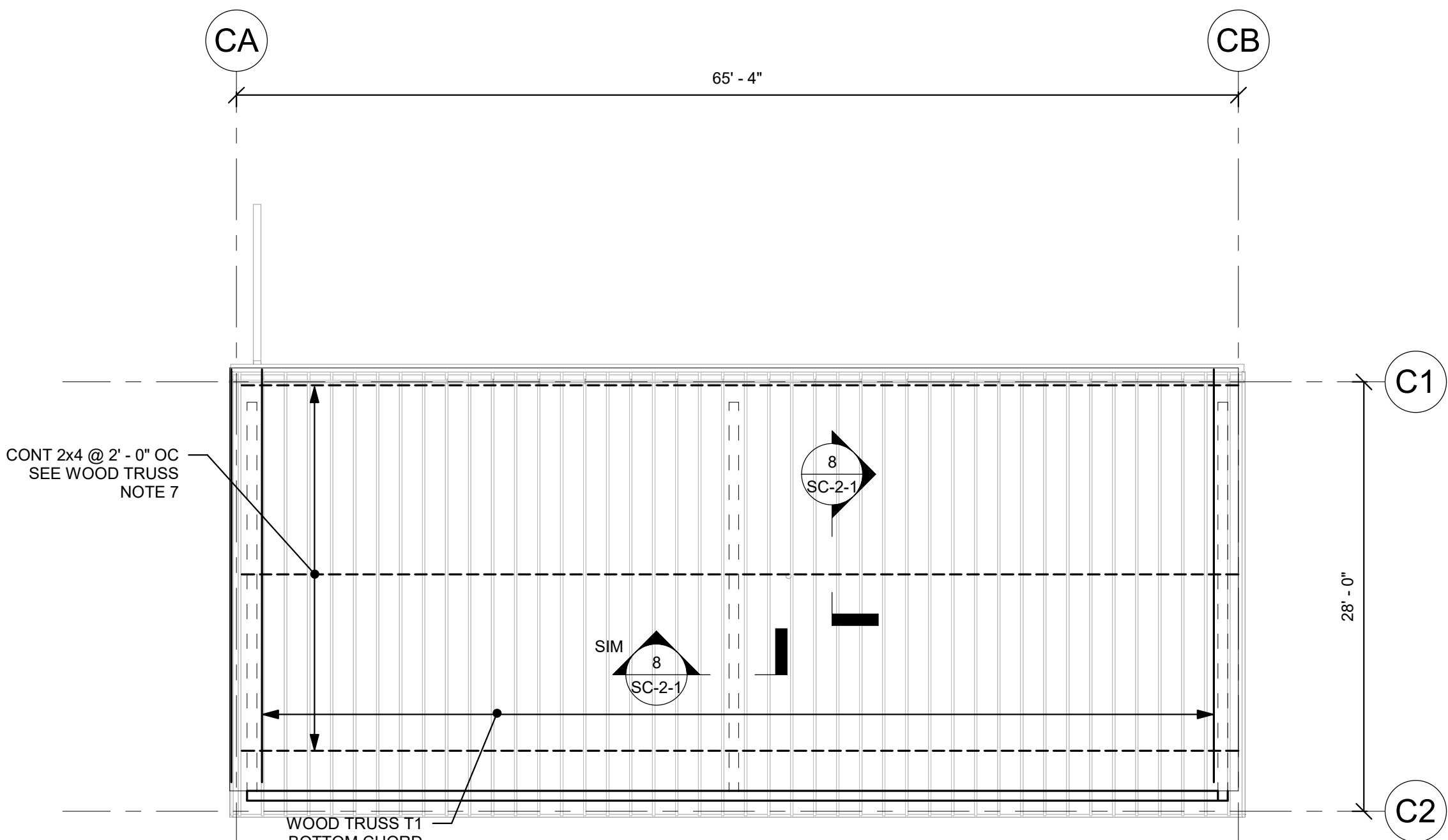


JOIST PROFILE AND LOAD DIAGRAM FOR 60DLHSP7

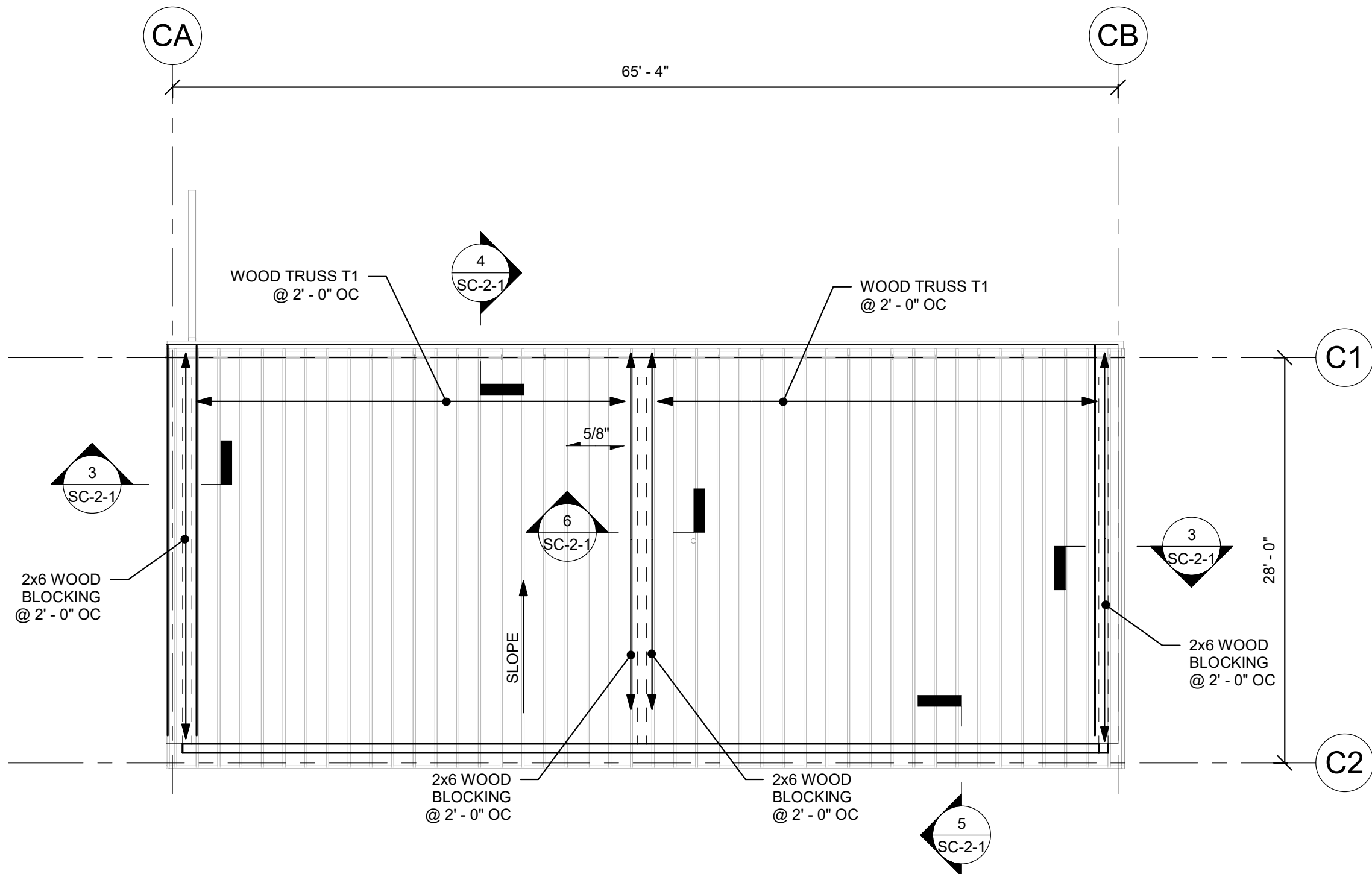
NOTE: SELF WEIGHT OF JOIST IS NOT INCLUDED IN LOADS



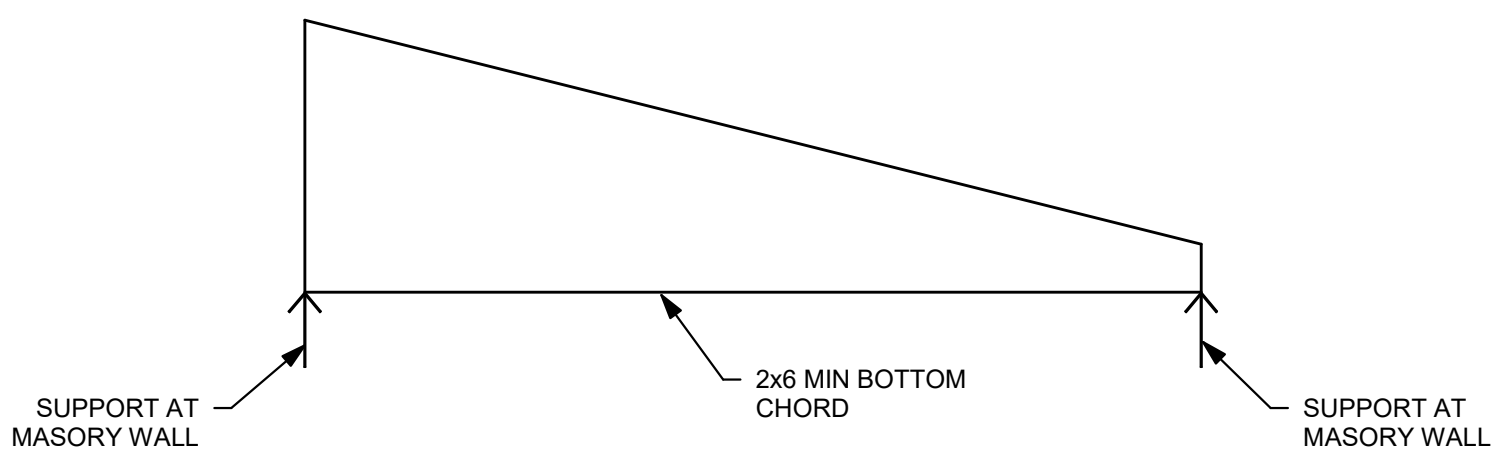
CONCESSION BUILDING GROUND FLOOR PLAN



CONCESSION BUILDING CEILING PLAN



CONCESSION BUILDING ROOF PLAN



WOOD TRUSS TYPE T1

WOOD TRUSS NOTES:

- 1.) TRUSS CONFIGURATIONS ARE DIAGRAMMATIC AND NOT TO SCALE. SEE ARCHITECTURAL DRAWINGS FOR EXACT LENGTHS AND CONDITIONS.
- 2.) REVIEW MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS FOR ADDITIONAL EQUIPMENT LOADS.
- 3.) EXCEPT AS SHOWN OTHERWISE, DESIGN TRUSSES FOR THE FOLLOWING MINIMUM UNIFORM DESIGN LOADS:
TRUSS TYPE T1
TOP CHORD DEAD LOAD 15PSF
BOTTOM CHORD DEAD LOAD 5PSF
SNOW LOAD 30PSF
(SEE GENERAL NOTES ON S0-0-1 FOR WIND AND SEISMIC DESIGN CRITERIA)
- 4.) DESIGN AND PROVIDE ALL TEMPORARY ERECTION RESTRAINTS / BRACING.
- 5.) DESIGN AND PROVIDE ALL PERMANENT RESTRAINT / BRACING FOR WEB MEMBERS AS REQUIRED FOR A PERMANENT INSTALLATION.
- 6.) OSB STRUCTURAL USE SHEATHING PROVIDES PERMANENT BRACING FOR THE TOP CHORD WHERE SHOWN.
- 7.) PROVIDE CONTINUOUS 2x4 @ 2'-0" ON CENTER (MAX.) FOR LATERAL BRACING OF TRUSS BOTTOM CHORD
- 8.) IN ADDITION TO WOOD BLOCKING SHOWN ON PLANS, PROVIDE 2x BLOCKING BETWEEN TOP CHORD OF TRUSSES AT EACH PANEL POINT. 2x BLOCKING SIZE TO MATCH TOP CHORD OF TRUSS.
- 9.) WHERE TRUSSES ARE TO BE SUPPLIED IN MORE THAN ONE PIECE, OR HINGED, DESIGN AND PROVIDE ALL NECESSARY BRACING CONNECTIONS AND ACCESSORIES.
- 10.) REFER TO GENERAL NOTES ON DRAWING S0-0-1 FOR ADDITIONAL REQUIREMENTS.
- 11.) DESIGN AND PROVIDE UPLIFT CONNECTORS. CONNECTORS SHOWN ARE MINIMUM REQUIREMENTS.

OSB STUCTURAL PANEL NAILING SCHEDULE
(EXCEPT AS NOTED OTHERWISE)

AT PANEL EDGE BOUNDARIES	10d COMMON NAILS AT 6" OC
AT OTHER PANEL EDGES	10d COMMON NAILS AT 6" OC
AT INTERMEDIATE PANEL SUPPORTS	10d COMMON NAILS AT 10" OC

FRAMING NOTES:

- 1.) FOR GENERAL NOTES AND TYPICAL DETAILS SEE DRAWINGS S0-0-1, S0-0-2, S0-0-3, S0-0-4, S0-0-5, S0-0-6, S0-0-7 AND S0-0-8.
- 2.) 5/8" INDICATES SPAN DIRECTION OF 5/8" ROOF SHEATHING. PROVIDE APA RATED STRUCTURAL SHEATHING EXPOSURE 1 PLYWOOD OR OSB.
- 3.) T1 ETC. INDICATES A PRE-FABRICATED WOOD TRUSS. SEE THIS DRAWING FOR TRUSS CONFIGURATION AND LOADING INFORMATION. MAXIMUM TRUSS SPACING EQUALS 2'-0" ON CENTER.
- 4.) FOR DIMENSIONS AND ELEVATIONS NOT GIVEN REFER TO ARCHITECTURAL DRAWINGS.

FOUNDATION NOTES:

- 1.) REFER TO GRADING DRAWINGS FOR PLAN AND GRADE ELEVATIONS. THE STRUCTURAL DRAWINGS USES A DATUM OF 100'-0" AT THE MAIN FLOOR, WHICH CORRESPONDS TO 163.50' MEAN SEA LEVEL, AS SHOWN ON THE SITE AND CIVIL DRAWINGS.
- 2.) FOR GENERAL NOTES AND TYPICAL DETAILS SEE DRAWINGS S0-0-1, S0-0-2, S0-0-3, S0-0-4, S0-0-5, S0-0-6, S0-0-7 AND S0-0-8.
- 3.) F3 ETC. INDICATES A FOOTING TYPE. FOR SIZE OF FOOTING AND REINFORCEMENT SEE SCHEDULE ON THIS DRAWING.
- 4.) TOP OF FOOTING ELEVATION TO BE 3'-6" MINIMUM BELOW LOWEST ADJACENT FINISHED GRADE AT EXTERIOR CONDITIONS AND 2'-0" BELOW TOP OF CONCRETE SLAB AT INTERIOR CONDITIONS. ALL OTHER TOP OF FOOTING ELEVATIONS ARE DENOTED AS THUS (XX'-XX") ON PLANS. CONTRACTOR TO COORDINATE AND VERIFY ALL TOP OF FOOTING ELEVATIONS WITH UNDERGROUND PLUMBING SUB-CONTRACTOR'S FIELD LAYOUT.
- 5.) ALL FOOTING ELEVATIONS NOTED ON PLAN ARE SHOWN ONLY TO ASSIST IN COORDINATION. ALL FOOTING ELEVATIONS MUST BE COORDINATED WITH STRUCTURAL REQUIREMENTS, TYPICAL DETAILS, ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS.
- 6.) SF INDICATES A STEPPED FOOTING REFER TO DETAIL 1 ON DRAWING S0-0-2.
- 7.) FOR UNDER SLAB DRAINAGE AND WALL DRAINS, COORDINATE WITH ARCHITECTURAL, STRUCTURAL, CIVIL, AND PLUMBING DRAWINGS.
- 8.) INDICATES A DEPRESSED SLAB ON GRADE. REFER TO DETAILS 6 AND 7 ON DRAWING S0-0-2 COORDINATE ALL SLAB DEPRESSIONS WITH REQUIREMENTS ON ARCHITECTURAL DRAWINGS.
- 9.) FOR TYPICAL EXTERIOR DOOR DETAIL REFER TO DETAIL 6 ON DRAWING S0-0-3 AND RELEVANT SECTIONS.
- 10.) INDICATES A CMU WALL. REFER TO TYPICAL DETAIL 3 ON DRAWING S0-0-4 FOR REINFORCEMENT AND DETAIL 4 ON DRAWING S0-0-4 FOR CONNECTIONS TO STEEL BEAMS AND CONCRETE SLABS AT THE TOP OF WALL FOR NON-STRUCTURAL WALLS. REFER TO RELEVANT SECTIONS FOR CONNECTIONS OF SHEAR WALLS TO THE STRUCTURE.
- 11.) FOR DIMENSIONS AND ELEVATIONS NOT GIVEN REFER TO ARCHITECTURAL DRAWINGS.
- 12.) INDICATES UNDERGROUND UTILITY LINES PLUMBING THROUGH CONCRETE FOUNDATION WALL TYPICAL. COORDINATE FOOTING ELEVATION WITH PIPE INVERTS AND TYPICAL STRUCTURAL DETAILS.

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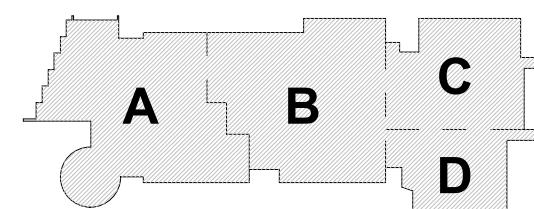
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03/31/2023	EARLY STRUCTURAL BID PACKAGE
REVISION LIST	
PR-003	8/23/2023 LOWER CAMPUS BUILDING REVISIONS

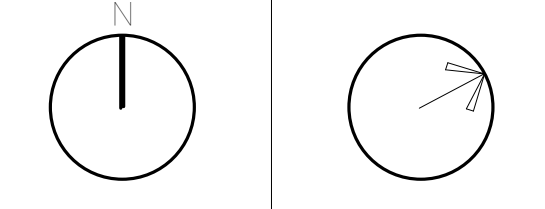
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August 28th, 2023



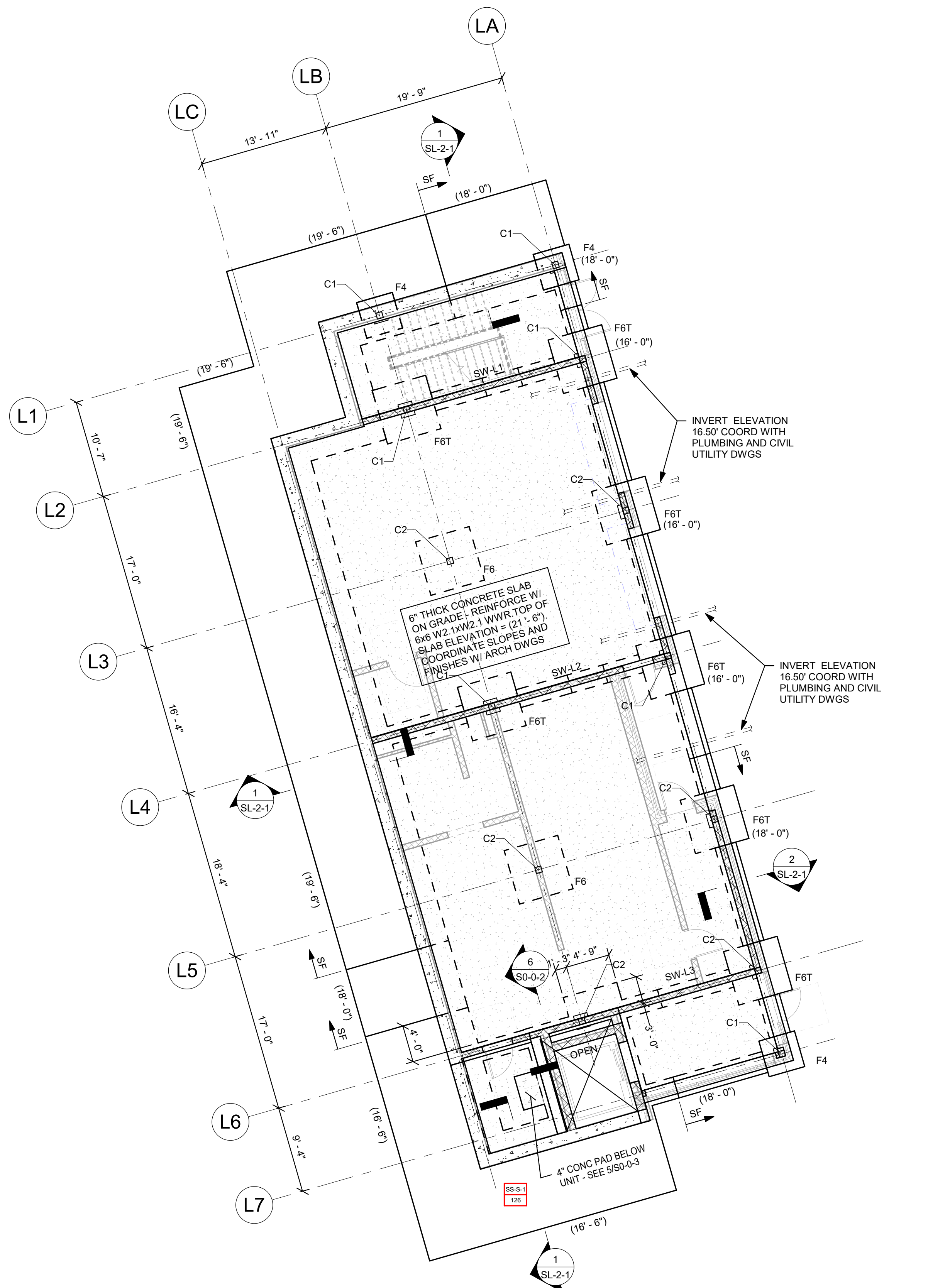
KEY PLAN

PROJECT NORTH MAGNETIC NORTH

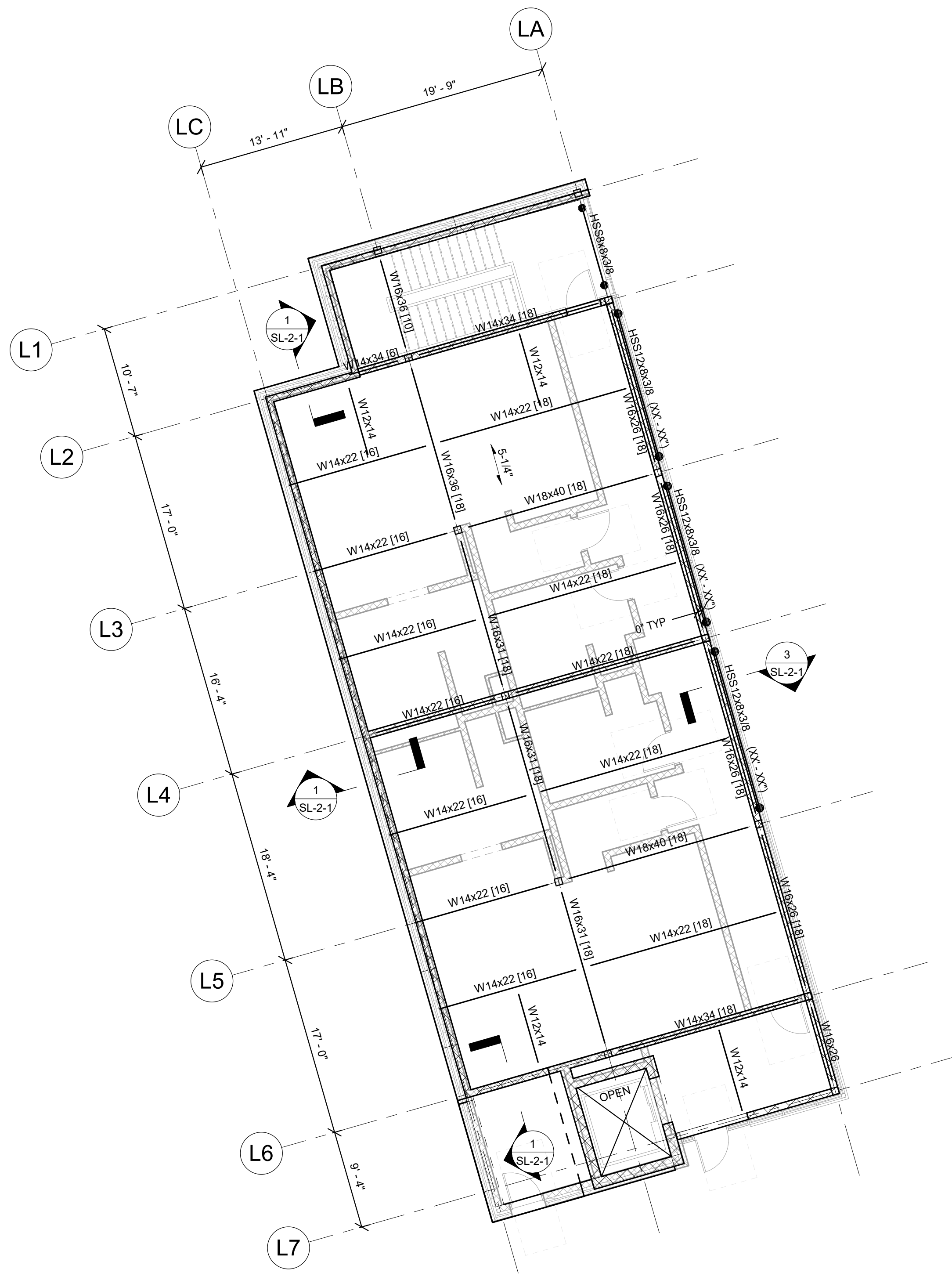


CONCESSION
BUILDING PLANS

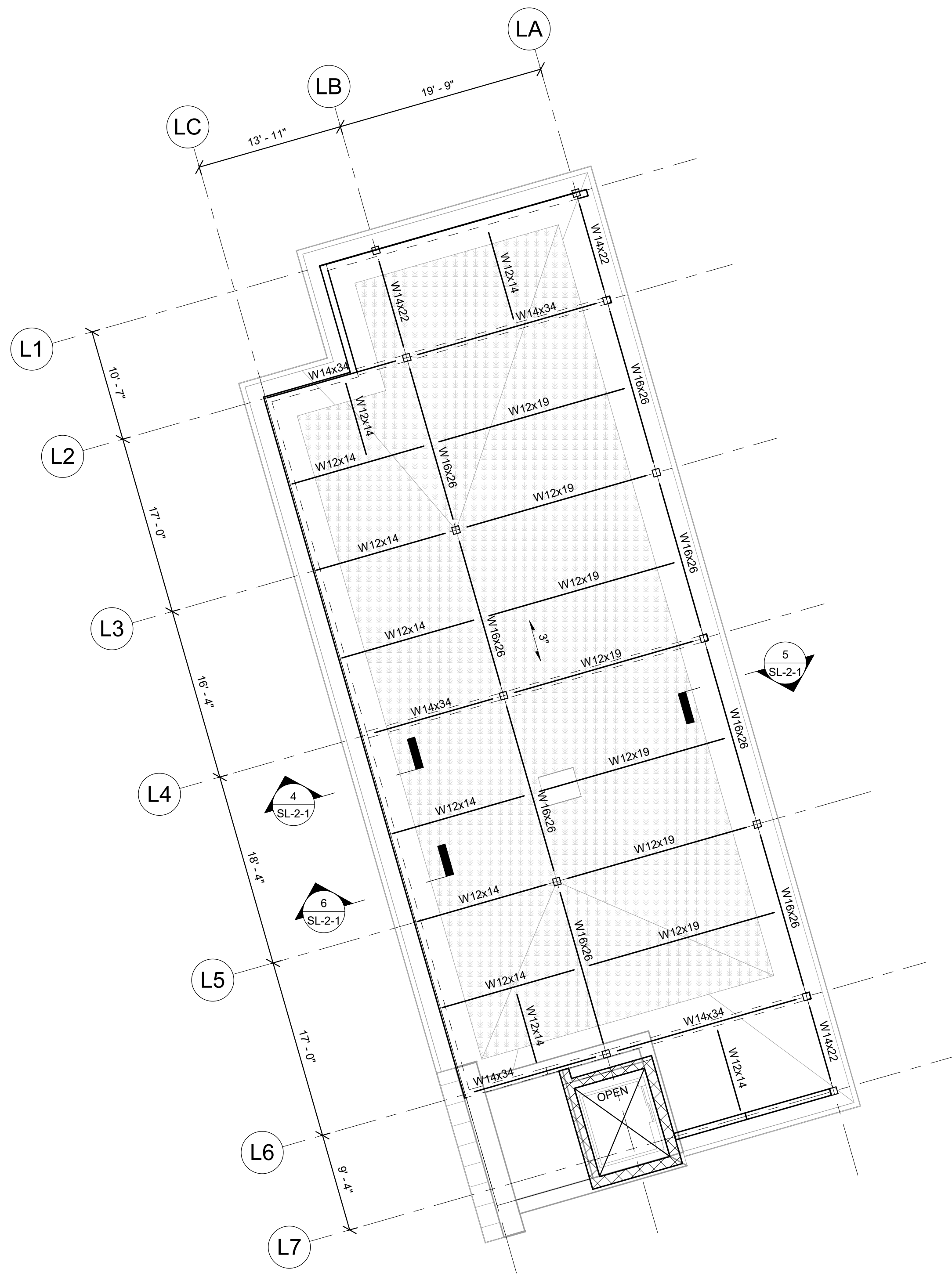
Scale: As indicated
Job No.: 20202
Drawn By: EDG
Date: August 28th, 2023
SC-1-1



LOCKER BUILDING GROUND FLOOR PLAN



LOCKER BUILDING FIRST FLOOR PLAN



LOCKER BUILDING ROOF PLAN

FOUNDATION NOTES:

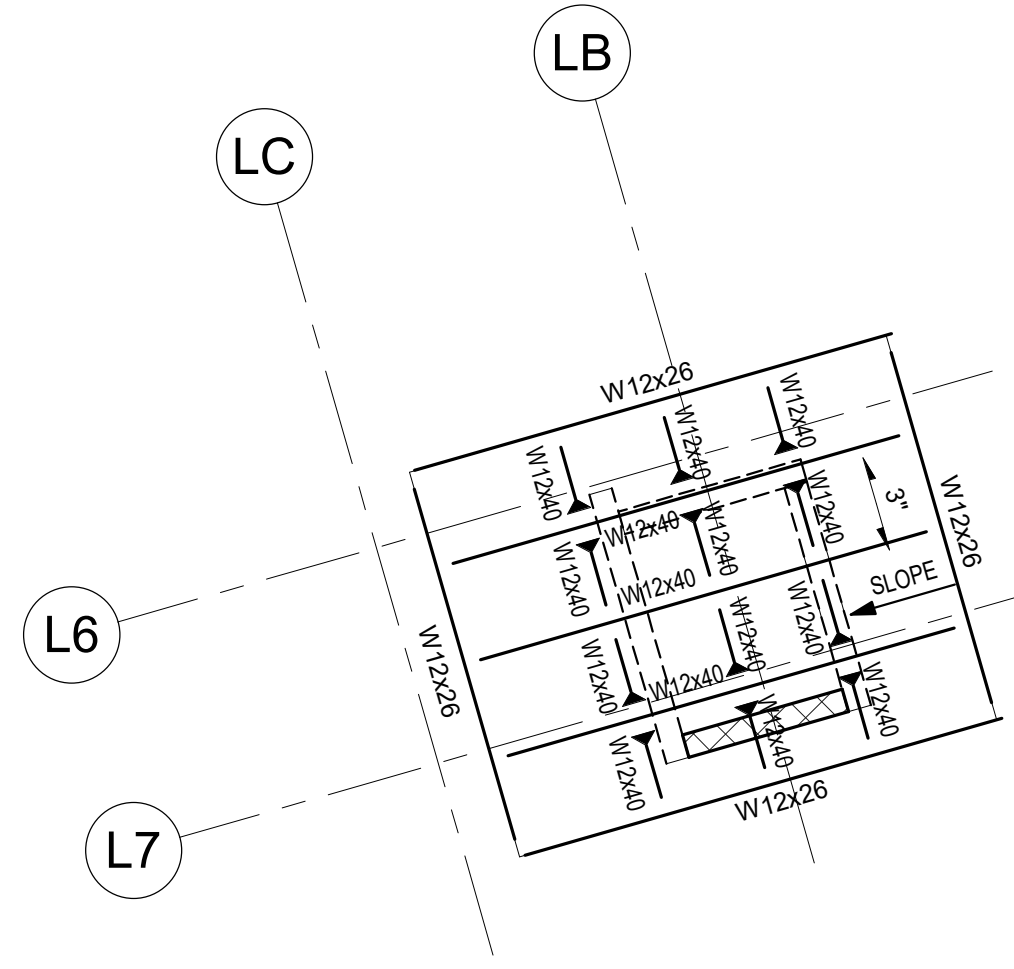
- 1) REFER TO GRADING DRAWINGS FOR PLAN AND GRADE ELEVATIONS. THE STRUCTURAL DRAWINGS USES A DATUM OF 100'-0" AT THE MAIN FLOOR, WHICH CORRESPONDS TO 163.50' MEAN SEA LEVEL, AS SHOWN ON THE SITE AND CIVIL DRAWINGS.
- 2) FOR GENERAL NOTES AND TYPICAL DETAILS SEE DRAWINGS S0-0-1, S0-0-2, S0-0-3, S0-0-4, S0-0-5, S0-0-6, S0-0-7 AND S0-0-8.
- 3) F3 ETC... INDICATES A FOOTING TYPE, FOR SIZE OF FOOTING AND REINFORCEMENT SEE SCHEDULE ON THIS DRAWING.
- 4) TOP OF FOOTING ELEVATION TO BE 3'-6" MINIMUM BELOW LOWEST ADJACENT FINISHED GRADE AT EXTERIOR CONDITIONS AND 2'-0" BELOW TOP OF CONCRETE SLAB AT INTERIOR CONDITIONS. ALL OTHER TOP OF FOOTING ELEVATIONS ARE DENOTED AS THUS (XX'-XX") ON PLANS. CONTRACTOR TO COORDINATE AND VERIFY ALL TOP OF FOOTING ELEVATIONS WITH UNDERGROUND PLUMBING SUB-CONTRACTOR'S FIELD LAYOUT.
- 5) ALL FOOTING ELEVATIONS NOTED ON PLAN ARE SHOWN ONLY TO ASSIST IN COORDINATION. ALL FOOTING ELEVATIONS MUST BE COORDINATED WITH STRUCTURAL REQUIREMENTS, TYPICAL DETAILS, ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS.
- 6) ALL FOOTINGS TO BE CENTERED UNDER COLUMNS UNLESS NOTED OTHERWISE.
- 7) SF INDICATES A STEPPED FOOTING REFER TO DETAIL 1 ON DRAWING S0-0-2.
- 8) BOTTOM OF BASE PLATE ELEVATION TO BE 1'-5" MINIMUM BELOW TOP OF CONCRETE SLAB AT INTERIOR CONDITIONS, AND 0'-11" BELOW TOP OF CONCRETE SLAB AT EXTERIOR CONDITIONS. UNLESS NOTED OTHERWISE AS (XX'-XX") REFER TO ARCHITECTURAL DRAWINGS FOR BRICK SHELF ELEVATIONS.
- 9) FOR UNDER SLAB DRAINAGE AND WALL DRAINS, COORDINATE WITH ARCHITECTURAL, STRUCTURAL, CIVIL, AND PLUMBING DRAWINGS.
- 10) INDICATES A DEPRESSED SLAB ON GRADE. REFER TO DETAILS 6 AND 7 ON DRAWING S0-0-2 COORDINATE ALL SLAB DEPRESSIONS WITH REQUIREMENTS ON ARCHITECTURAL DRAWINGS.
- 11) FOR TYPICAL EXTERIOR DOOR DETAIL, REFER TO DETAIL 6 ON DRAWING S0-0-3 AND RELEVANT SECTIONS.
- 12) INDICATES A CMU WALL. REFER TO TYPICAL DETAIL 3 ON DRAWING S0-0-4 FOR REINFORCEMENT AND DETAIL 4 ON DRAWING S0-0-6 FOR CONNECTIONS TO STEEL BEAMS AND CONCRETE SLABS AT THE TOP OF WALL FOR NON-STRUCTURAL WALLS. REFER TO RELEVANT SECTIONS FOR CONNECTIONS OF SHEAR WALLS TO THE STRUCTURE.
- 13) FOR DIMENSIONS AND ELEVATIONS NOT GIVEN REFER TO ARCHITECTURAL DRAWINGS.
- 14) INDICATES CONCRETE PIER REFER TO TYPICAL DETAIL 5 ON DRAWING S0-0-2.
- 15) INDICATES UNDERGROUND UTILITY LINES PLUMBING THROUGH CONCRETE FOUNDATION WALL TYPICAL. COORDINATE FOOTING ELEVATION WITH PIPE INVERTS AND TYPICAL STRUCTURAL DETAILS.
- 16) CONCRETE PIER REINFORCING PER DETAIL 5 ON DRAWING S0-0-2 IS TO BE PROVIDED FOR ALL CONCRETE WALLS SUPPORTING COLUMNS. HORIZONTAL WALL REINFORCING MUST REMAIN CONTINUOUS.

FRAMING NOTES:

- 1) FOR GENERAL NOTES AND TYPICAL DETAILS SEE DRAWINGS S0-0-1, S0-0-2, S0-0-3, S0-0-4, S0-0-5, S0-0-6, S0-0-7 AND S0-0-8.
- 2) REFER TO ARCHITECTURAL DRAWINGS FOR ELEVATIONS AND VERTICAL DIMENSIONS. FITCH ALL STEEL UNIFORMITY TO LOW POINTS AT THE COLUMNS AND BENT BEAMS AS SHOWN ON THE ARCHITECTURAL DRAWINGS.
- 3) [XX] INDICATES THE NUMBER OF 3/4" DIAMETER x 3 1/2" LONG HEADED STUDS WELDED TO THE TOP FLANGE OF THE BEAM. SPACE STUDS EVENLY ALONG THE BEAM UNLESS NOTED OTHERWISE.
- 4) INDICATES A MOMENT CONNECTION TO DEVELOP THE FULL CAPACITY OF THE MEMBER. REFER TO TYPICAL DETAILS 7, 8 AND 9 ON DRAWING ON DRAWING S0-0-6.
- 5) INDICATES A 5/16" FILLET WELD ALL AROUND, (HSS BEAM TO HSS COLUMN) WHERE BEAM DIMENSIONS EXCEED COLUMN DIMENSIONS PROVIDE 1/2" THICK STEEL CAP PLATE TO ACHIEVE ALL AROUND WELD. REFER TO TYPICAL DETAIL 2 ON DRAWING S0-0-7.
- 6) < X > INDICATES UPWARD CAMBER AT THE MID-SPAN OF THE MEMBER.
- 7) 4 1/2" INDICATES SPAN DIRECTION OF 2" DEEP, 20 GAGE GALVANIZED COMPOSITE STEEL DECK WITH 2 1/2" NORMAL WEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 4 1/2" REINFORCE WITH 6x6 - W2.1xW2.1 WWR.
- 8) 1 1/2" INDICATES SPAN DIRECTION OF 1 1/2" DEEP, 20 GAGE TYPE B, GALVANIZED STEEL ROOF DECK.
- 9) 3" INDICATES SPAN DIRECTION OF 3" DEEP, 20 GAGE TYPE N, GALVANIZED STEEL ROOF DECK.
- 10) FOR EXACT NUMBER, SIZE, AND LOCATION OF OPENING IN STEEL DECKING REFER TO MECHANICAL AND ARCHITECTURAL DRAWINGS. FOR FRAMING INFORMATION, REFER TO DETAIL 1 AND 2 ON DRAWING S0-0-8.
- 11) INDICATES A ROOF DRAIN. REFER TO TYPICAL STRUCTURAL DETAILS 1 AND 8 ON DRAWING S0-0-4 AND DETAIL 11 ON DRAWING S0-0-8 FOR DECKING SUPPORT. REFER TO DETAIL 4 ON DRAWING S0-0-5. REFER TO PLUMBING AND ARCHITECTURAL DRAWINGS FOR OPENING SIZES AND LOCATIONS.
- 12) CT INDICATES A COLUMN TERMINATES AT THIS LEVEL.
- 13) WE INDICATES A BEND IN THE STEEL BEAM. REFER TO TYPICAL DETAIL 9 ON DRAWING S0-0-8.
- 14) INDICATES A CMU WALL. REFER TO TYPICAL DETAIL 3 ON DRAWING S0-0-4 FOR REINFORCEMENT AND DETAIL 4 ON DRAWING S0-0-6 FOR CONNECTIONS TO STEEL BEAMS AND CONCRETE SLABS AT THE TOP OF WALL FOR NON-STRUCTURAL WALLS. REFER TO RELEVANT SECTIONS FOR CONNECTIONS OF SHEAR WALLS TO THE STRUCTURE.
- 15) FOR DIMENSIONS AND ELEVATIONS NOT GIVEN REFER TO ARCHITECTURAL DRAWINGS.

FOOTING SCHEDULE		
DESIGN SOIL BEARING CAPACITY = 2 TSF		
MARK	SIZE	REINFORCEMENT
F6T	6'-0" x 8'-0" x 2'-0"	7 - #6 BOT EA WAY
F7T	7'-0" x 7'-0" x 2'-0"	8 - #6 BOT EA WAY

T INDICATES TOP REINFORCING TO MATCH BOTTOM REINFORCING



LOCKER BUILDING ELEVATOR ROOF PART PLAN

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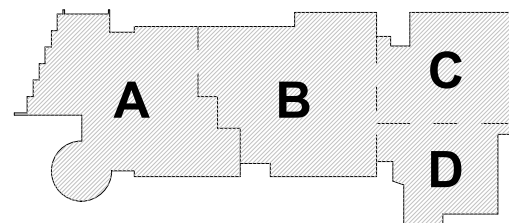
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SS-S-1	4/14/2023 STRUCTURAL STEEL ADDENDUM 1

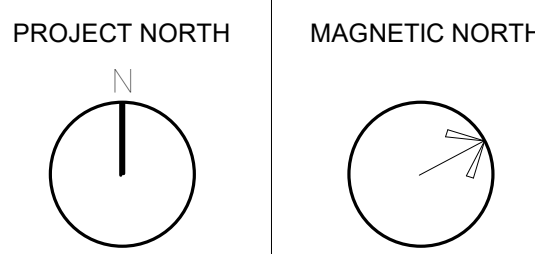
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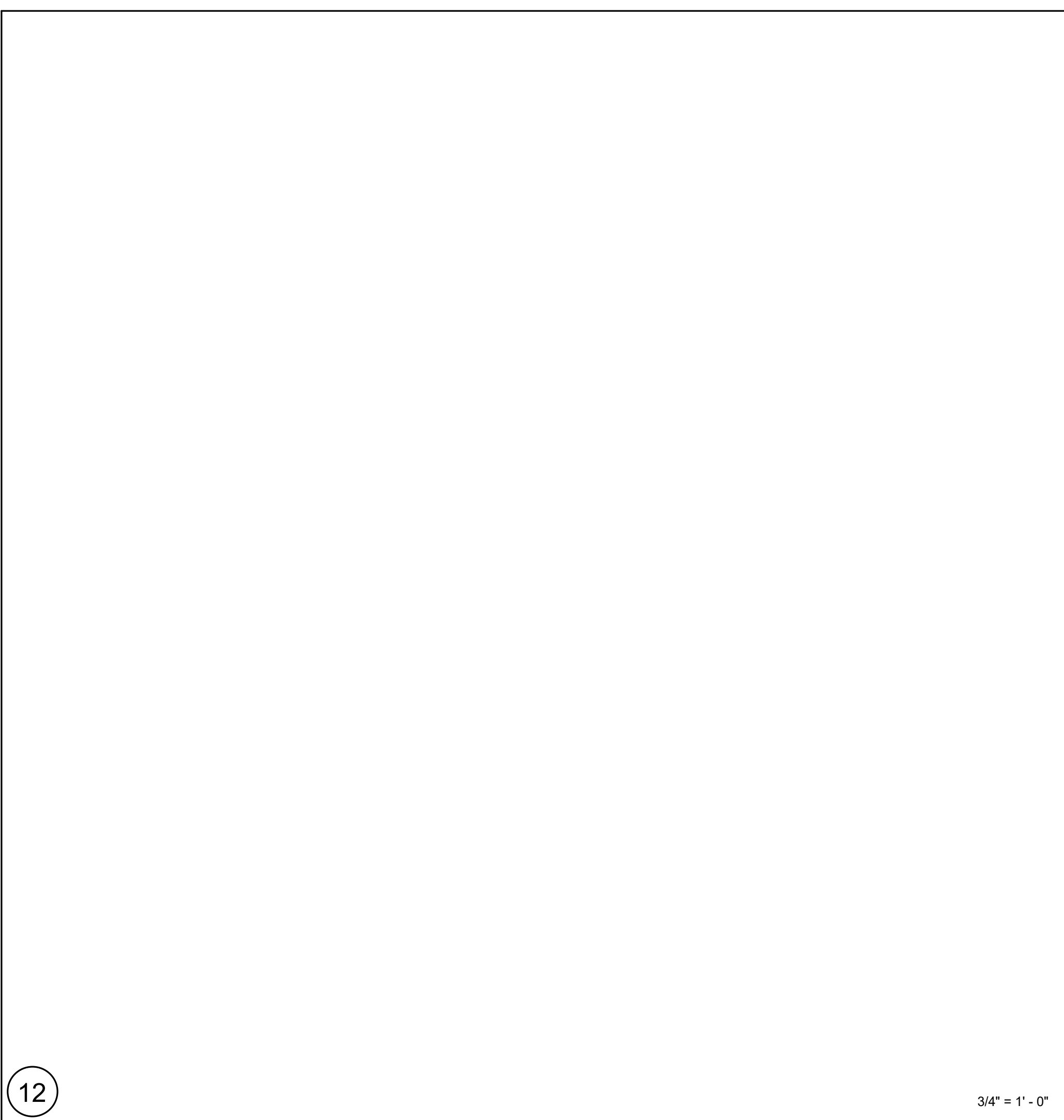
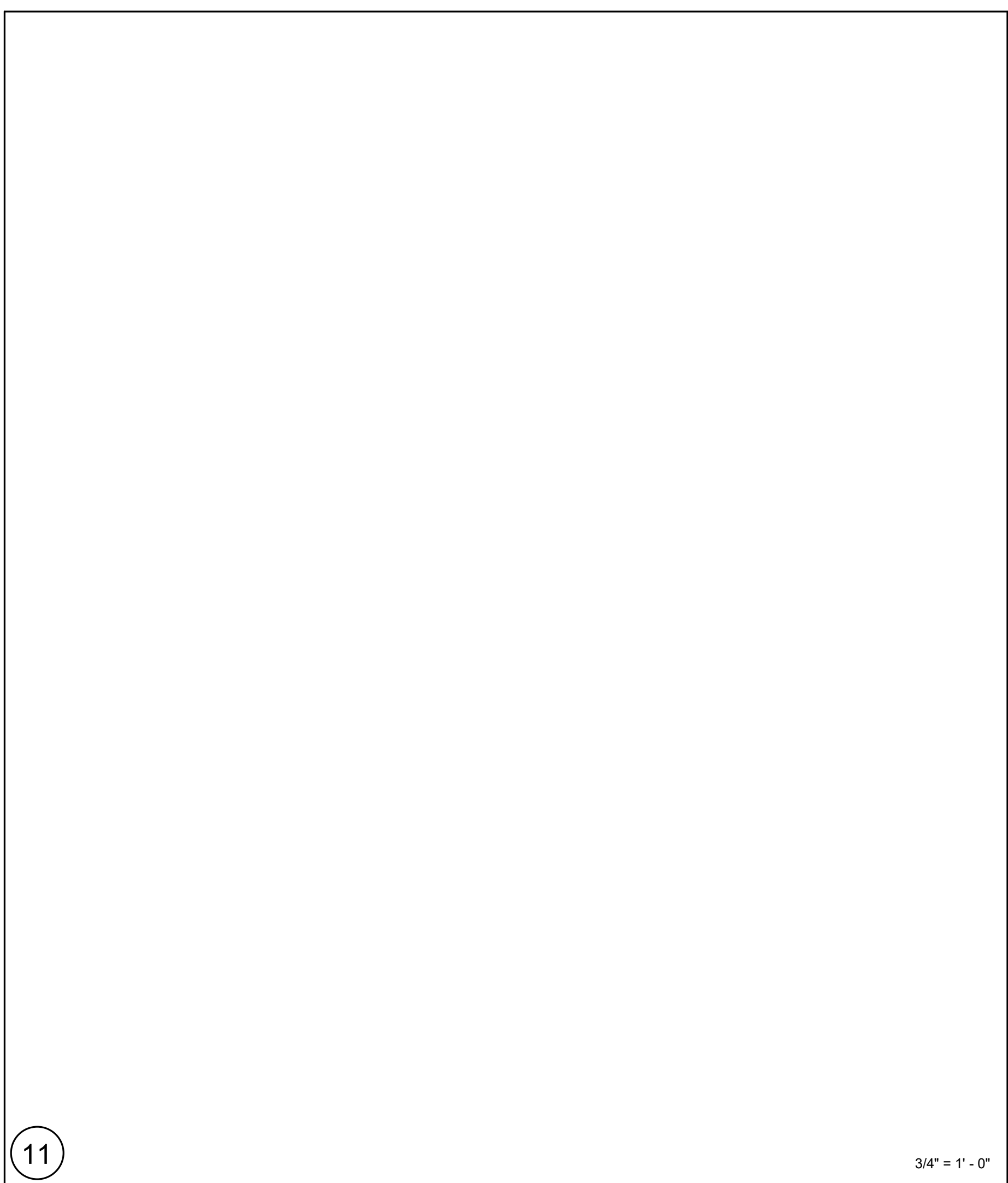
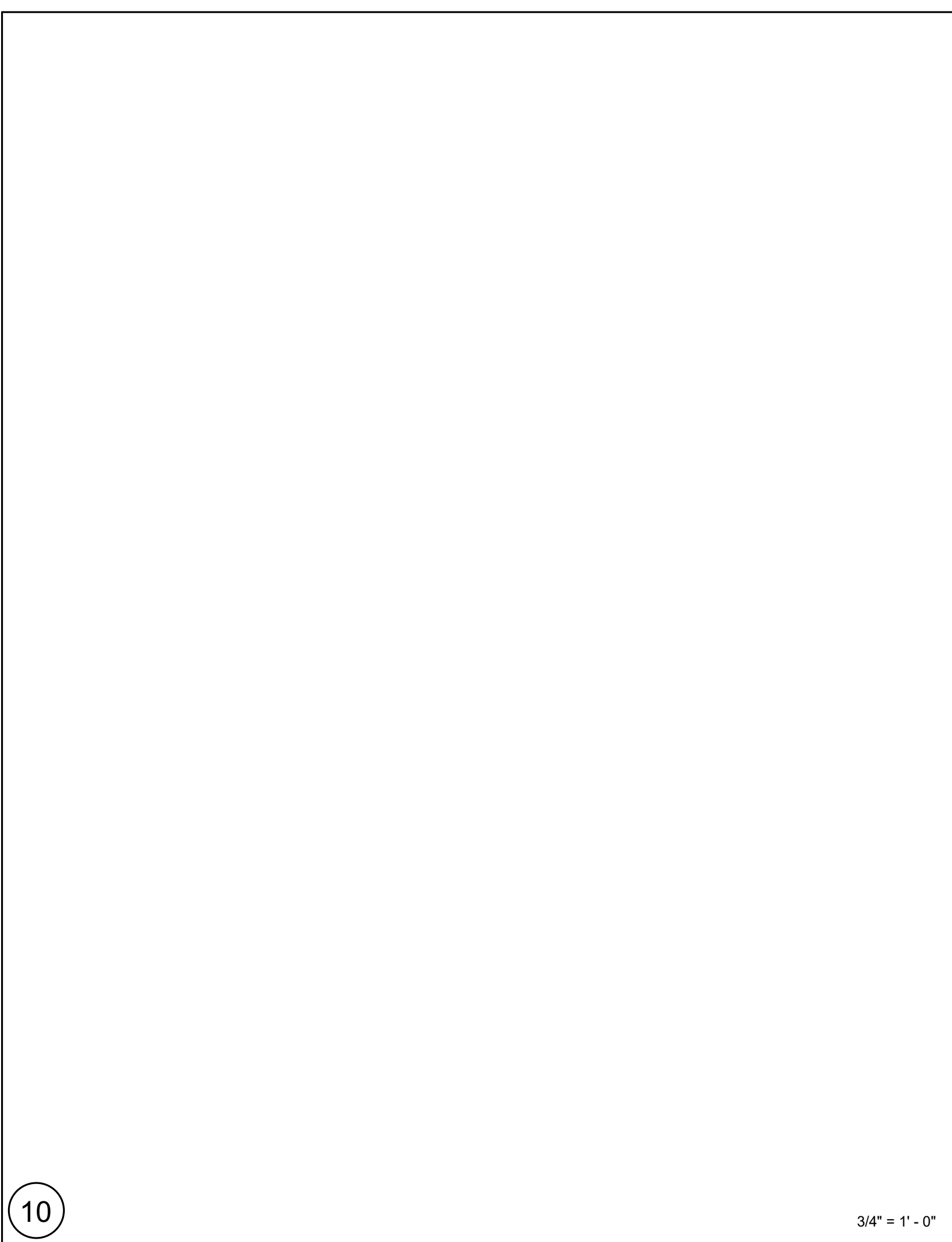
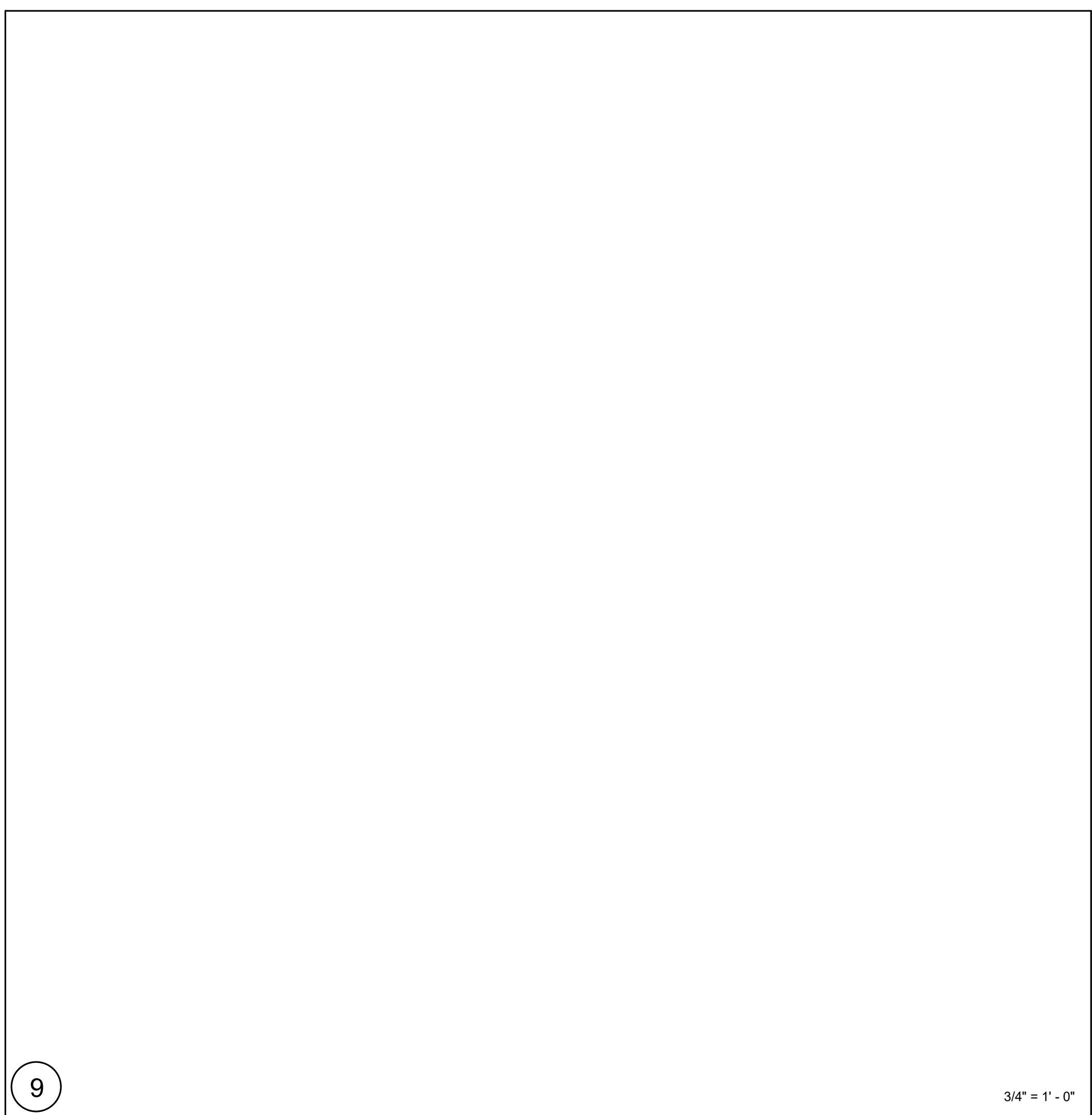
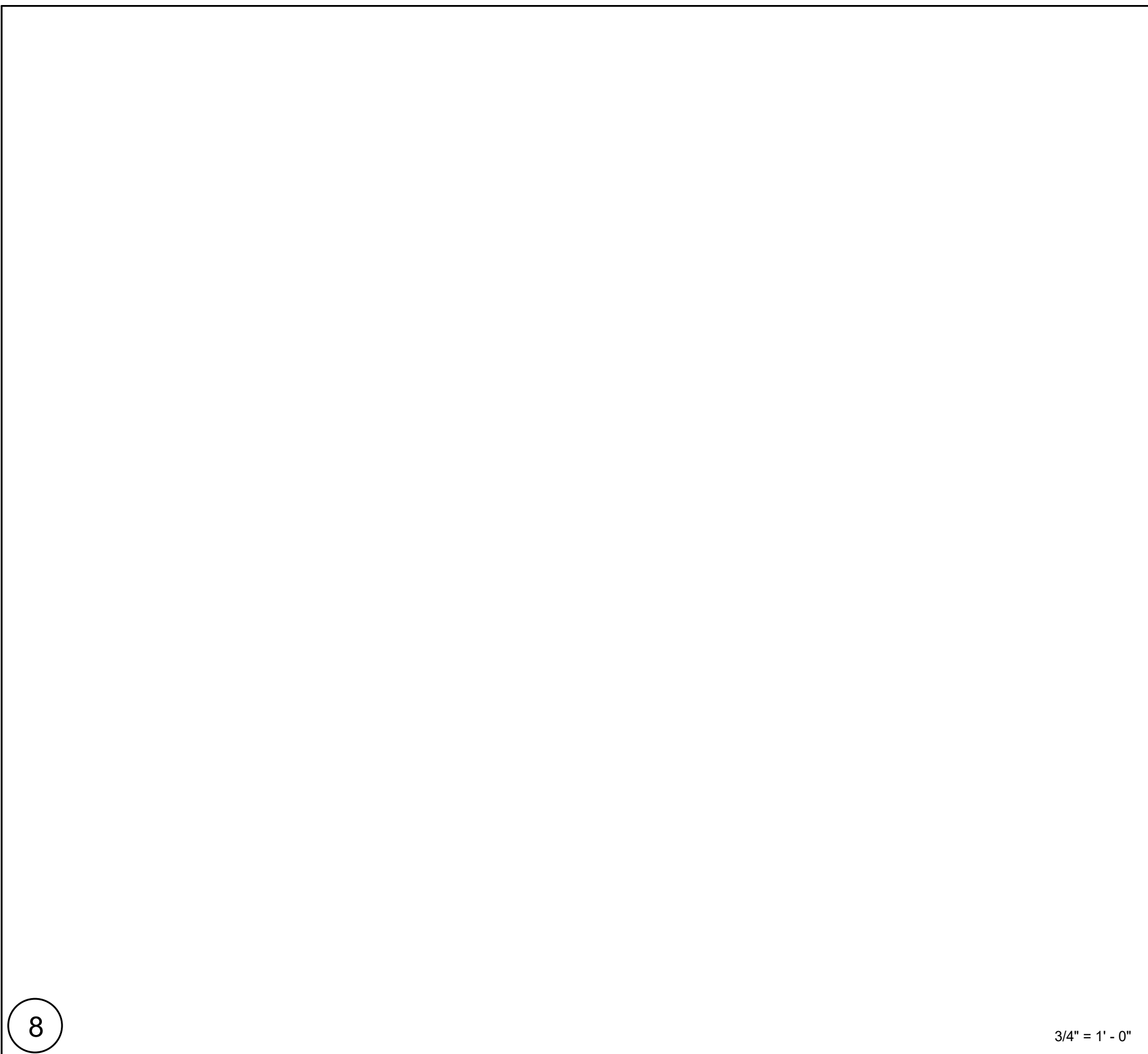
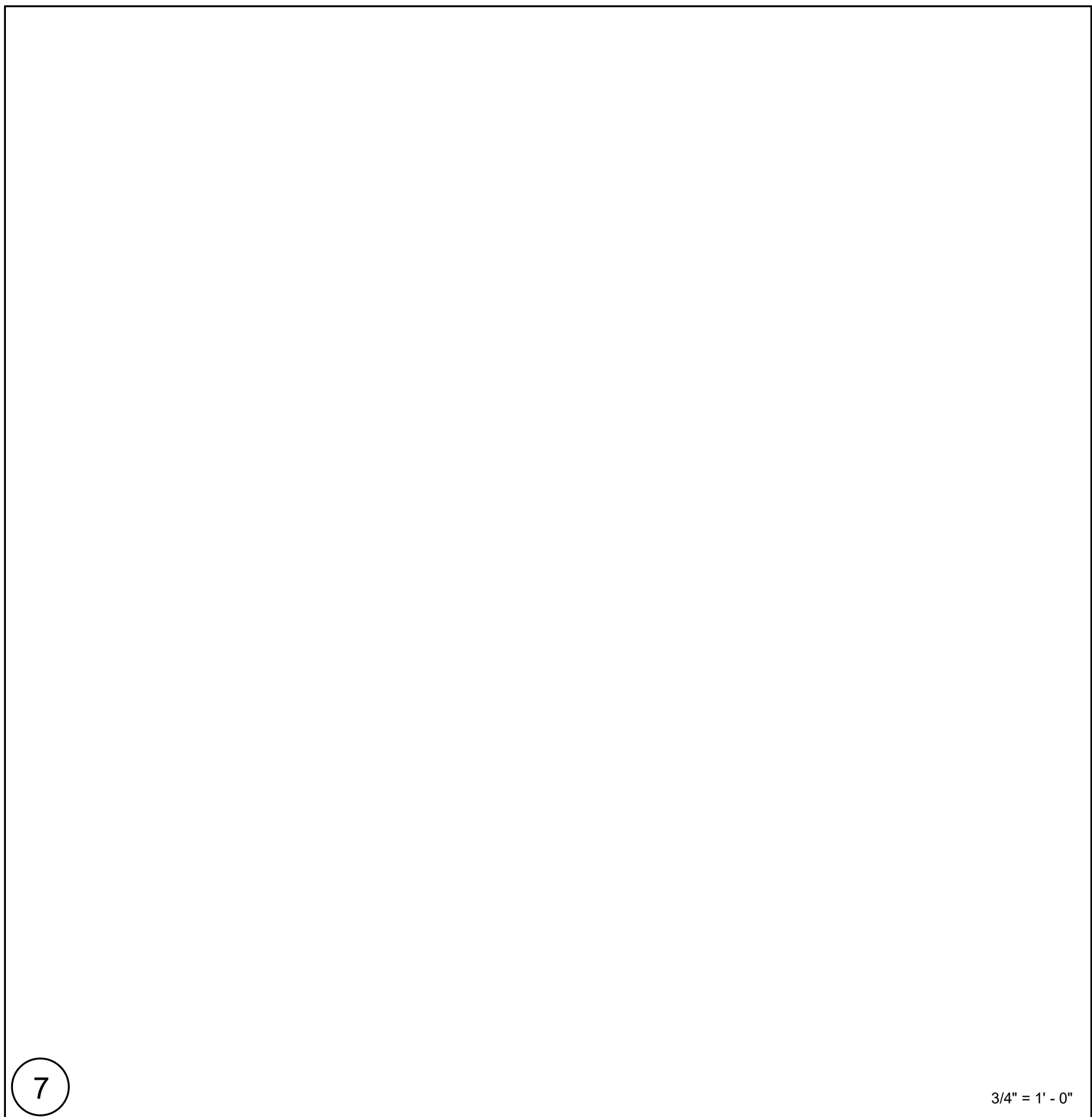
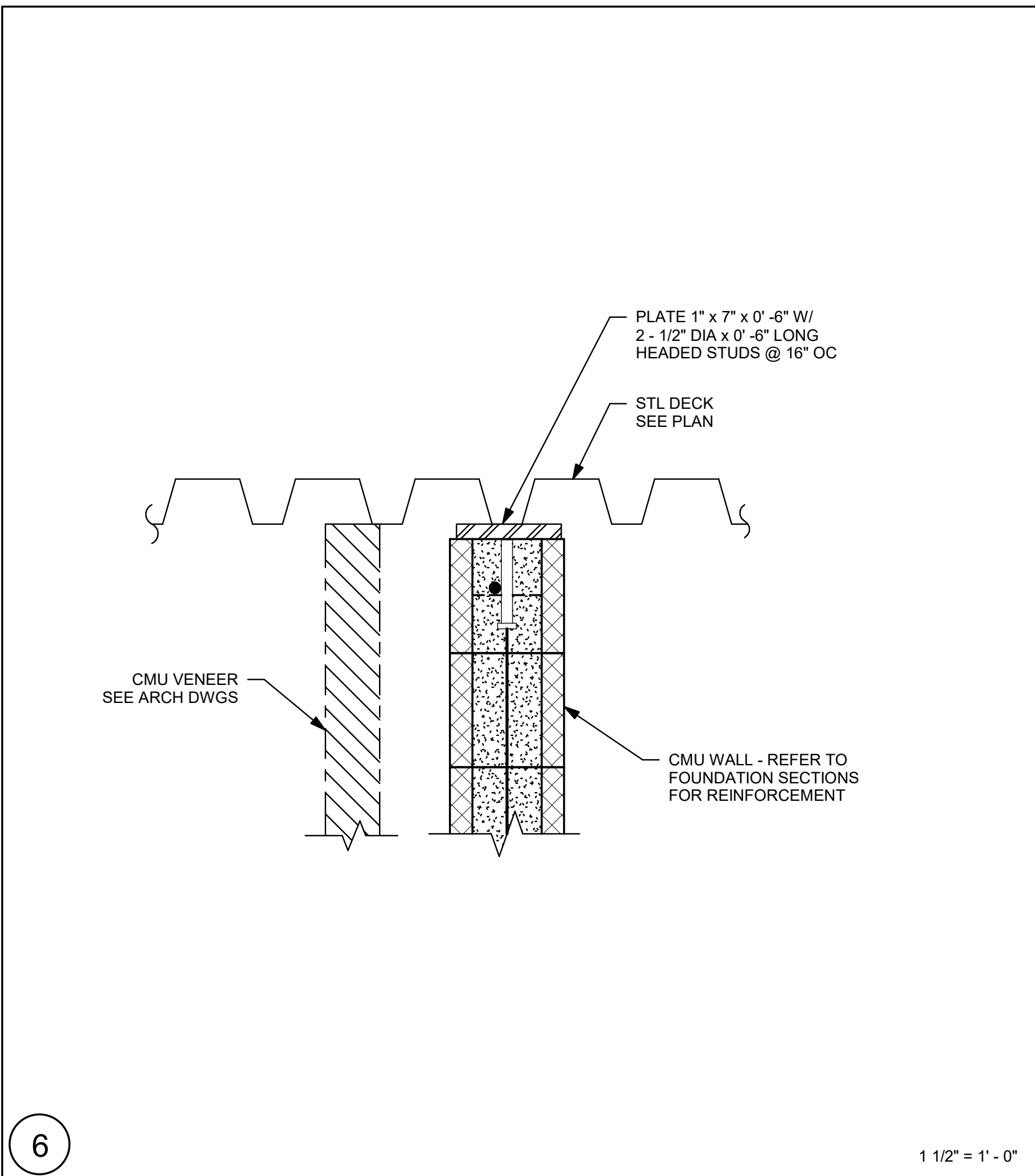
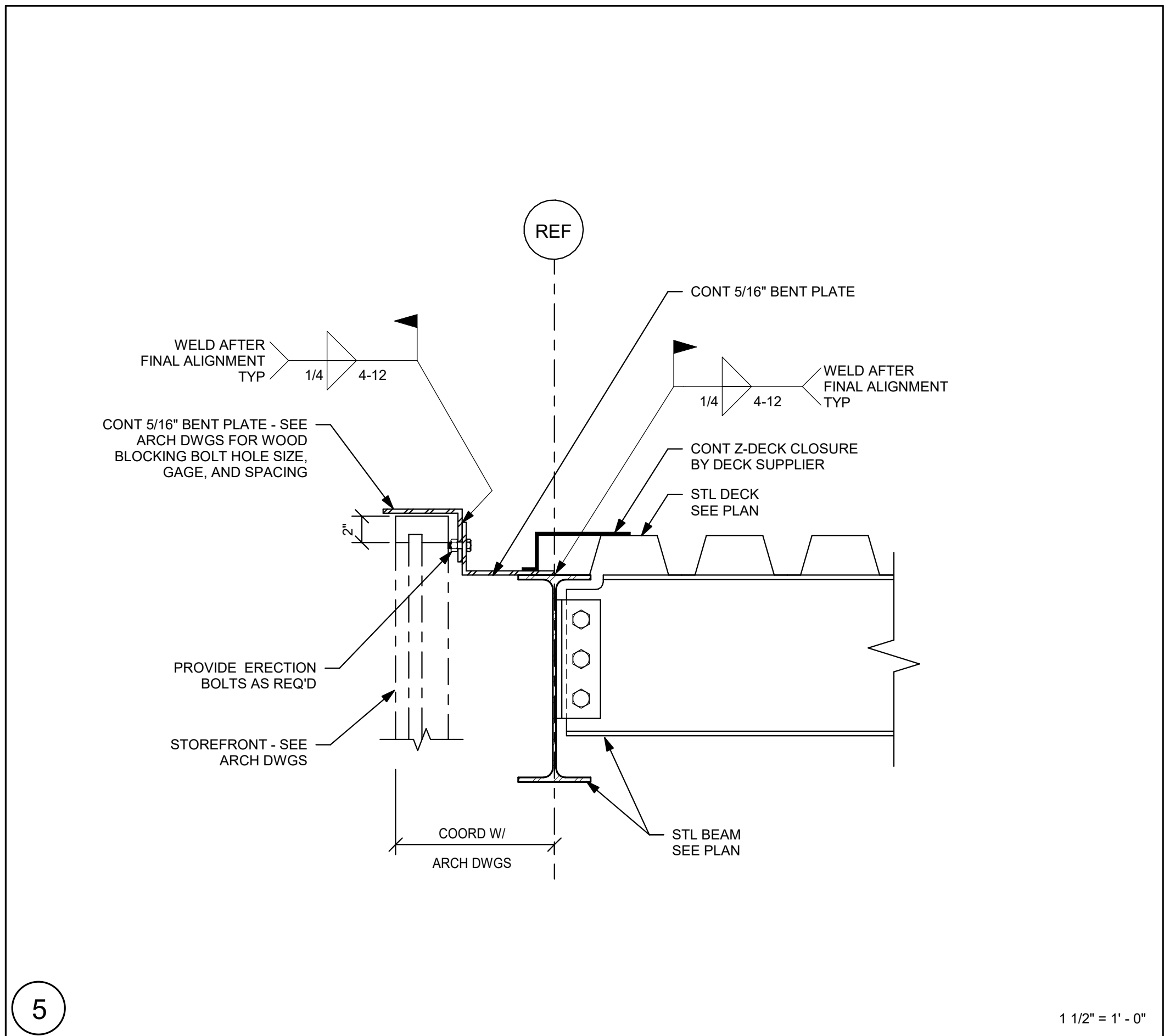
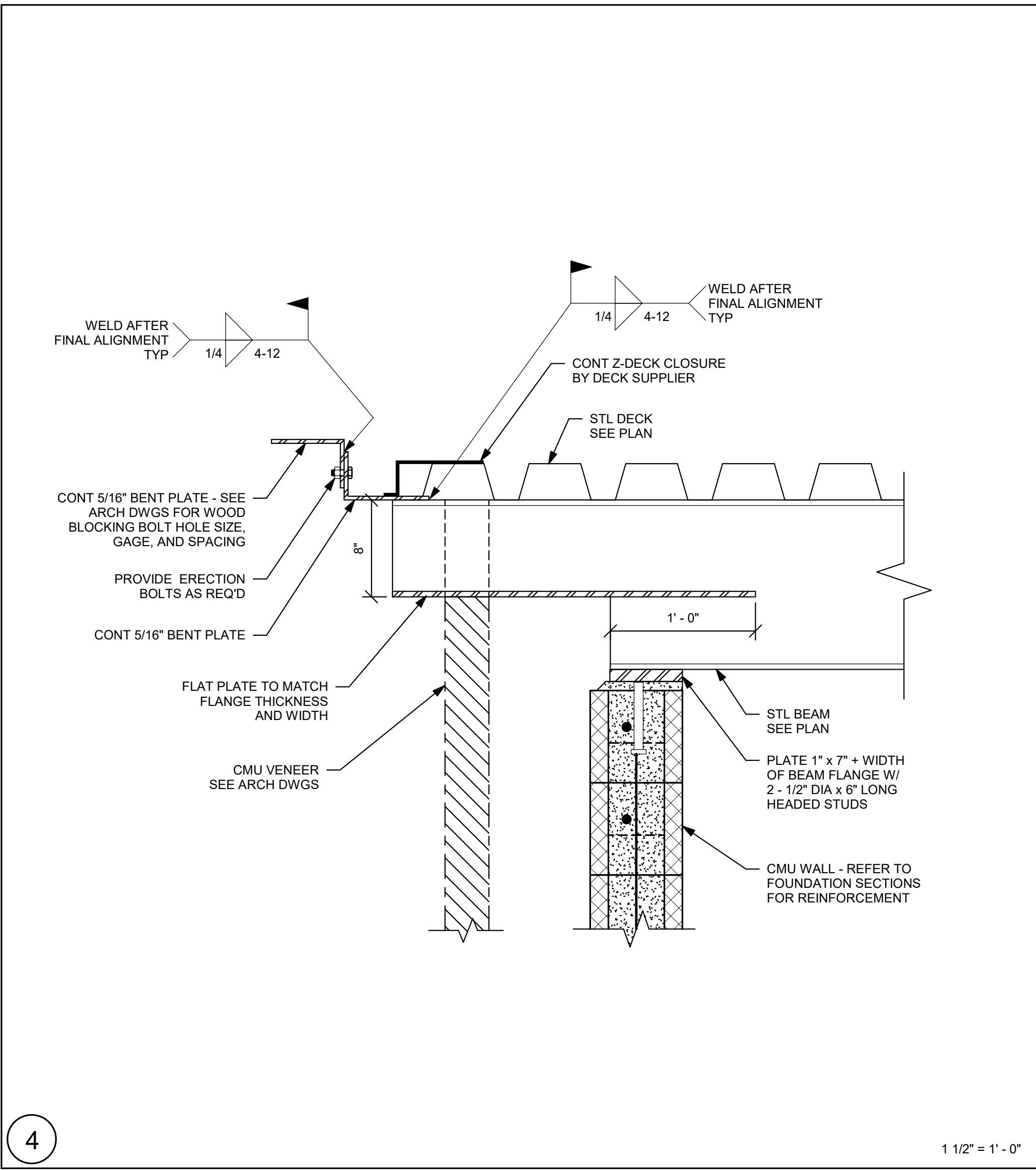
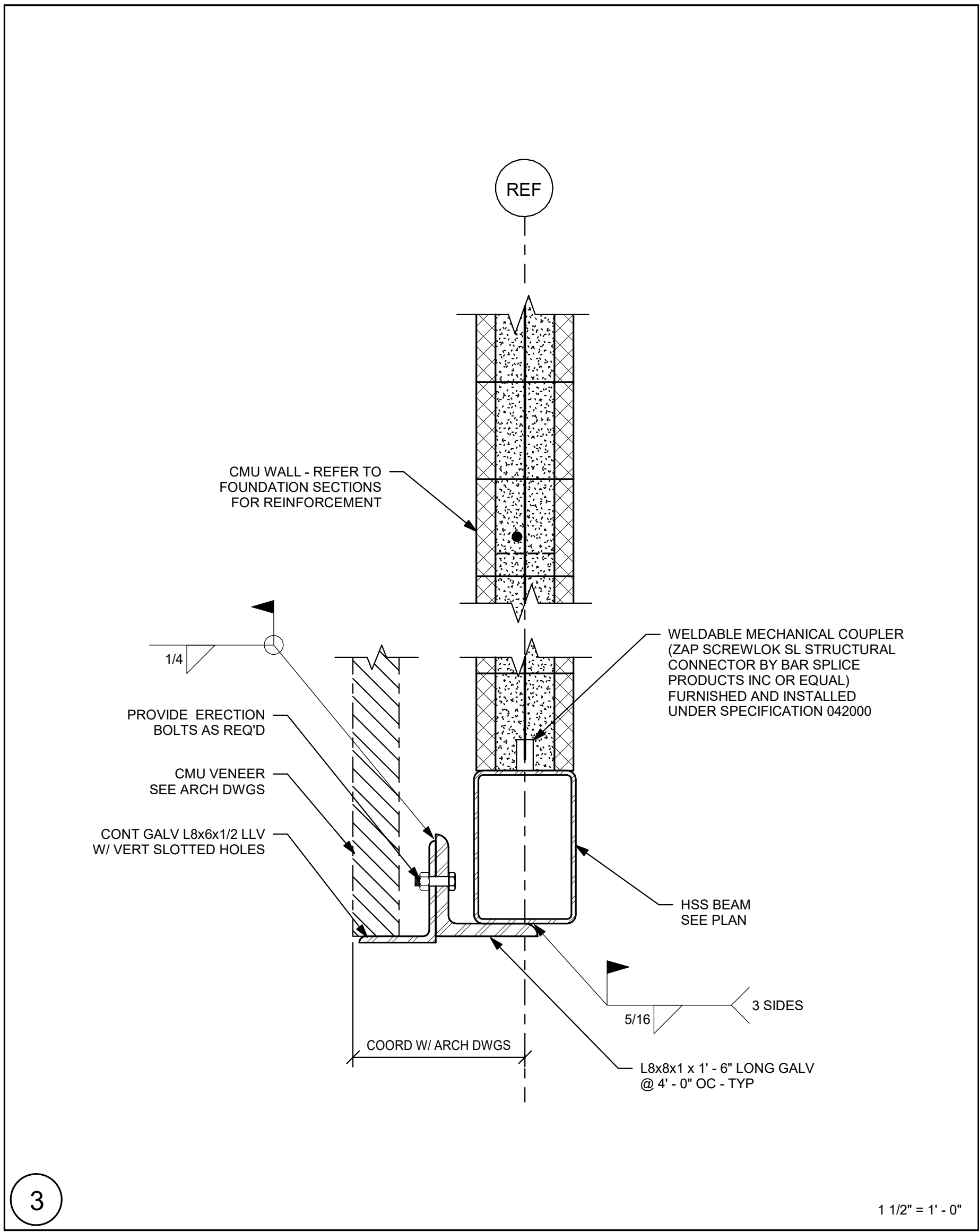
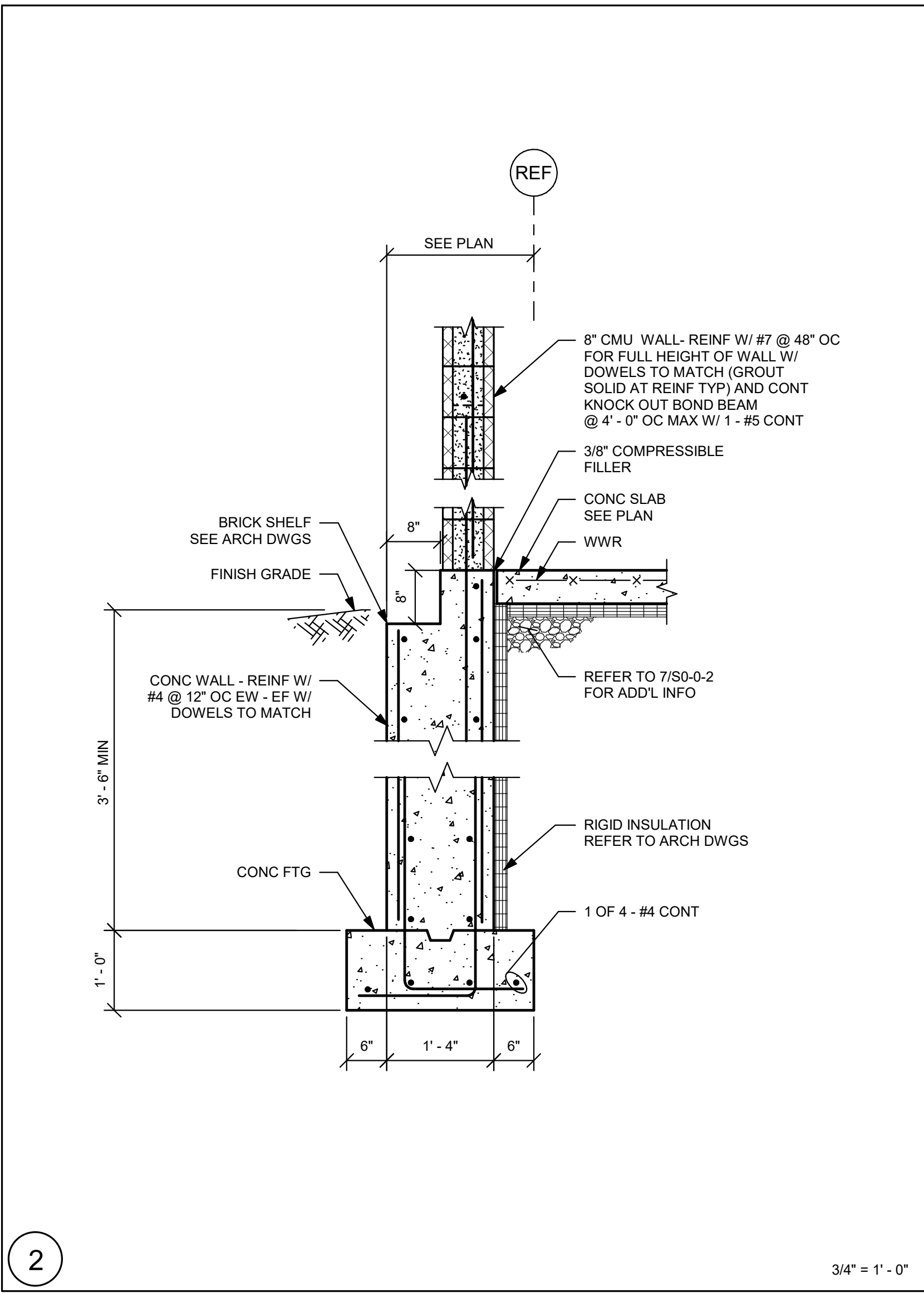
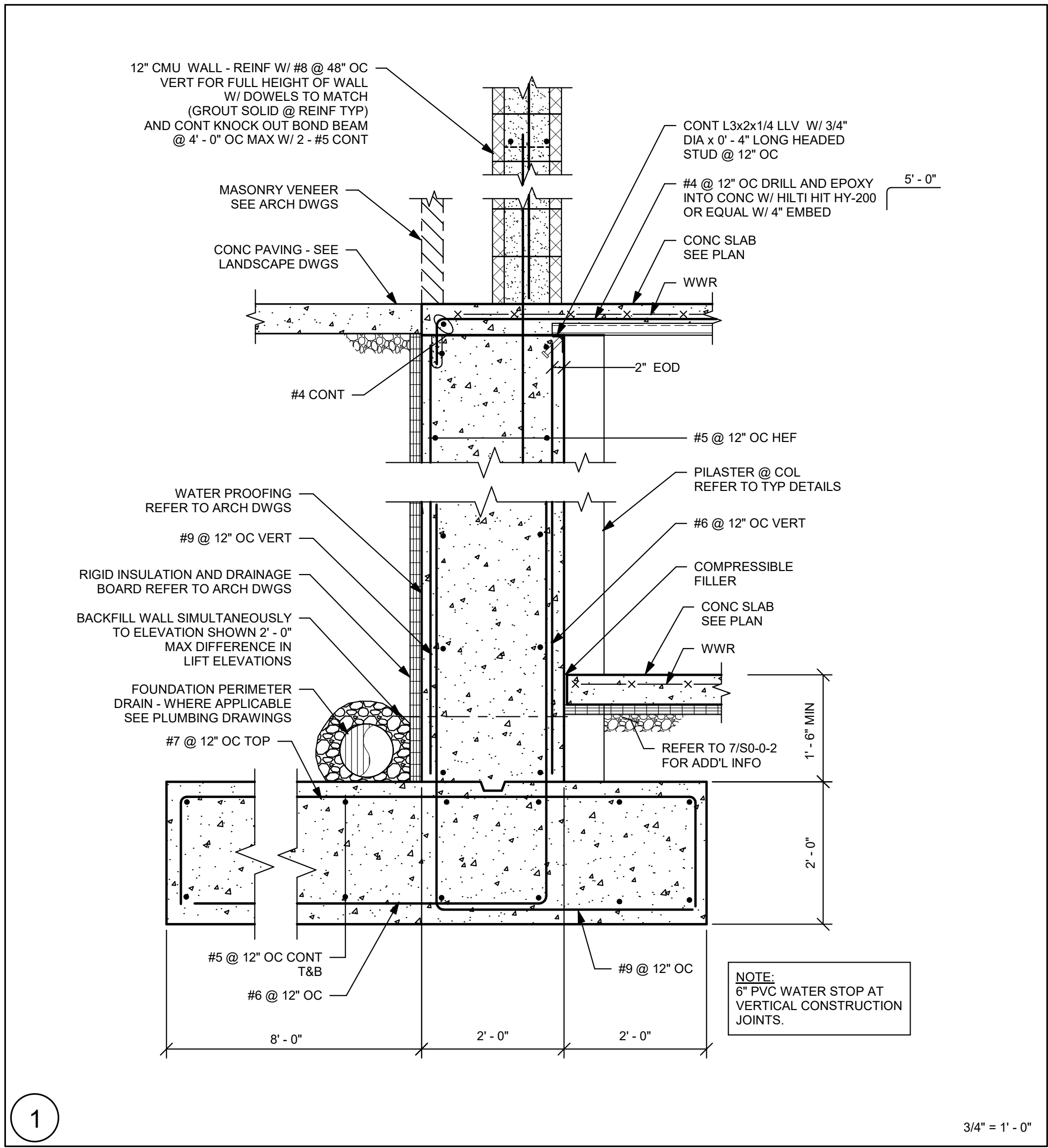
KEY PLAN



LOCKER BUILDING PLANS

Scale: 1/8" = 1'-0"
Job No.: 20202
Drawn By: EDG
Date: August 28th, 2023

SL-1-1



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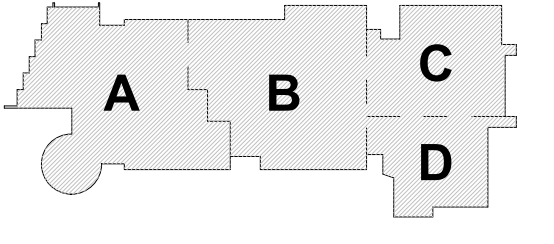
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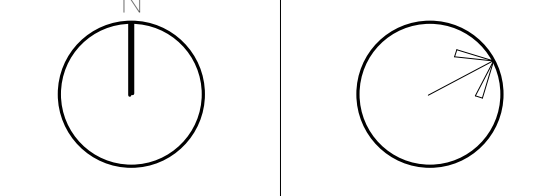
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August 28th, 2023



KEY PLAN

PROJECT NORTH MAGNETIC NORTH



LOCKER ROOM BUILDING SECTIONS

Scale: As indicated
Job No.: 20202
Drawn By: EDG
Date: August 28th, 2023

SL-2-1



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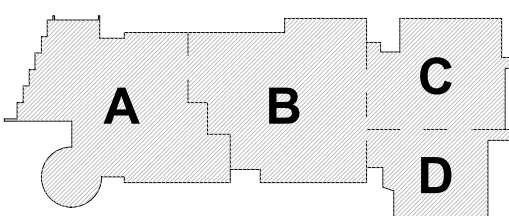
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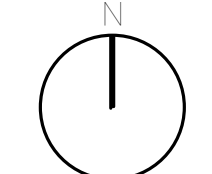
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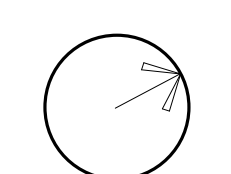


KEY PLAN

PROJECT NORTH



MAGNETIC NORTH

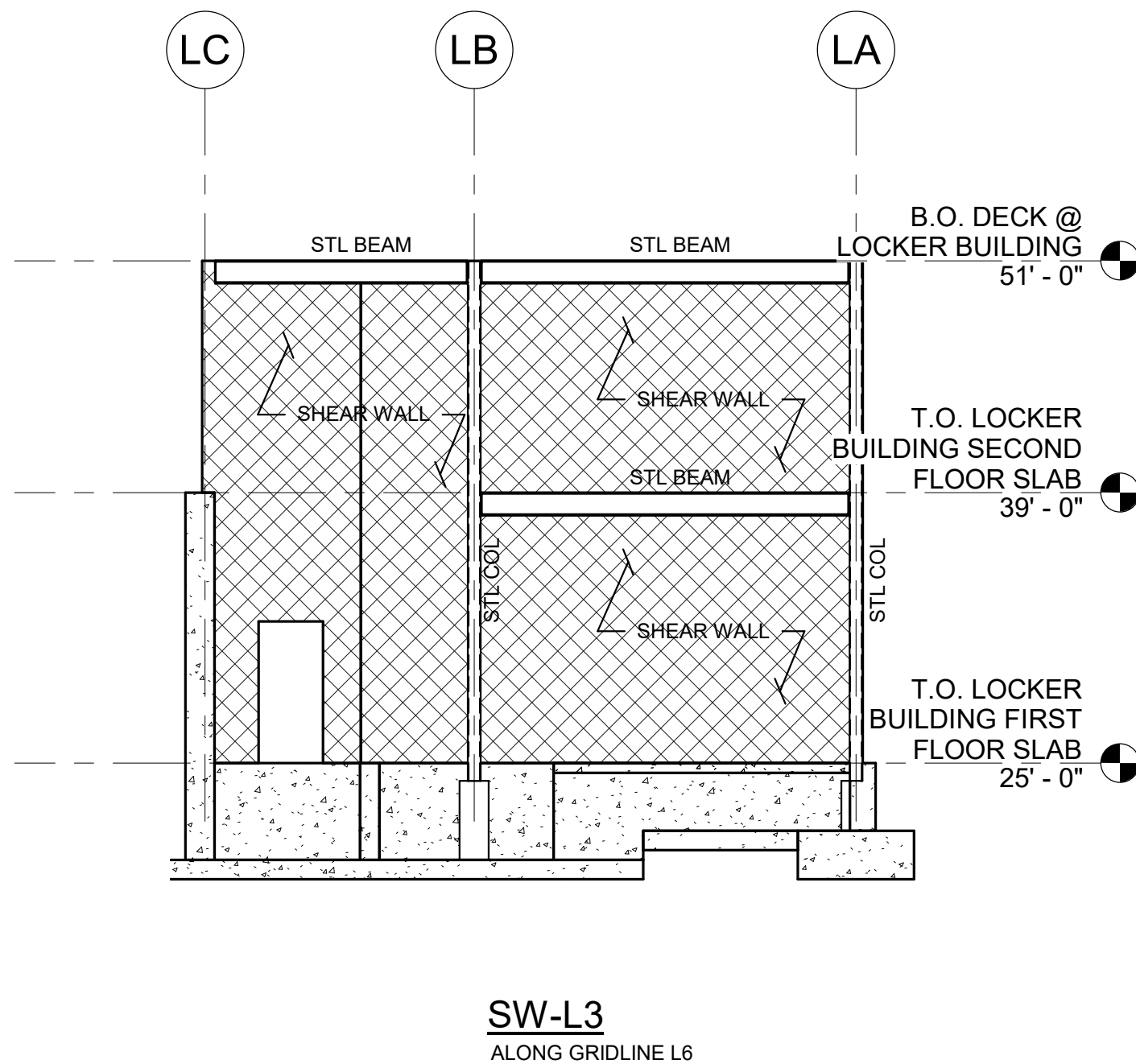
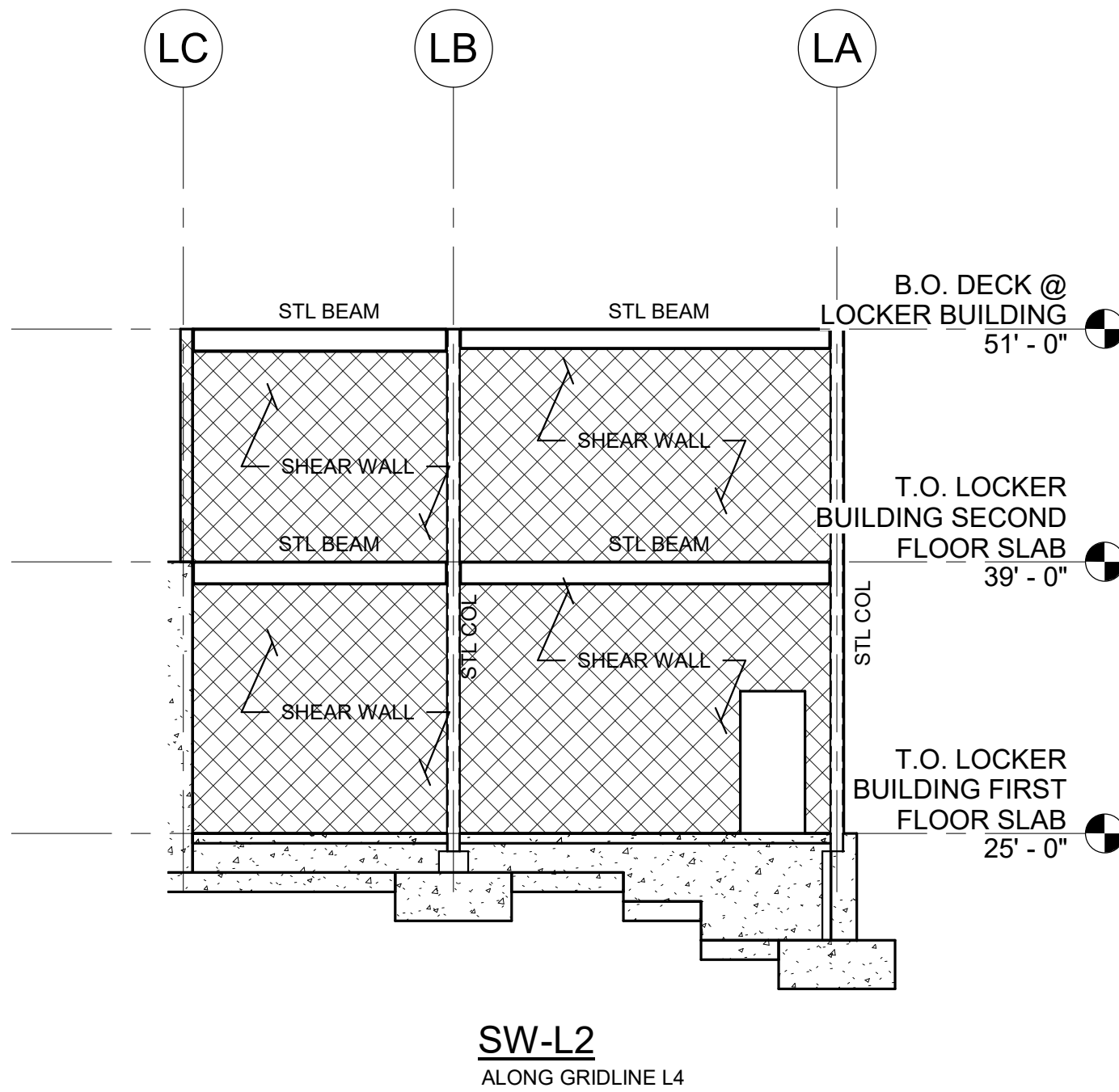
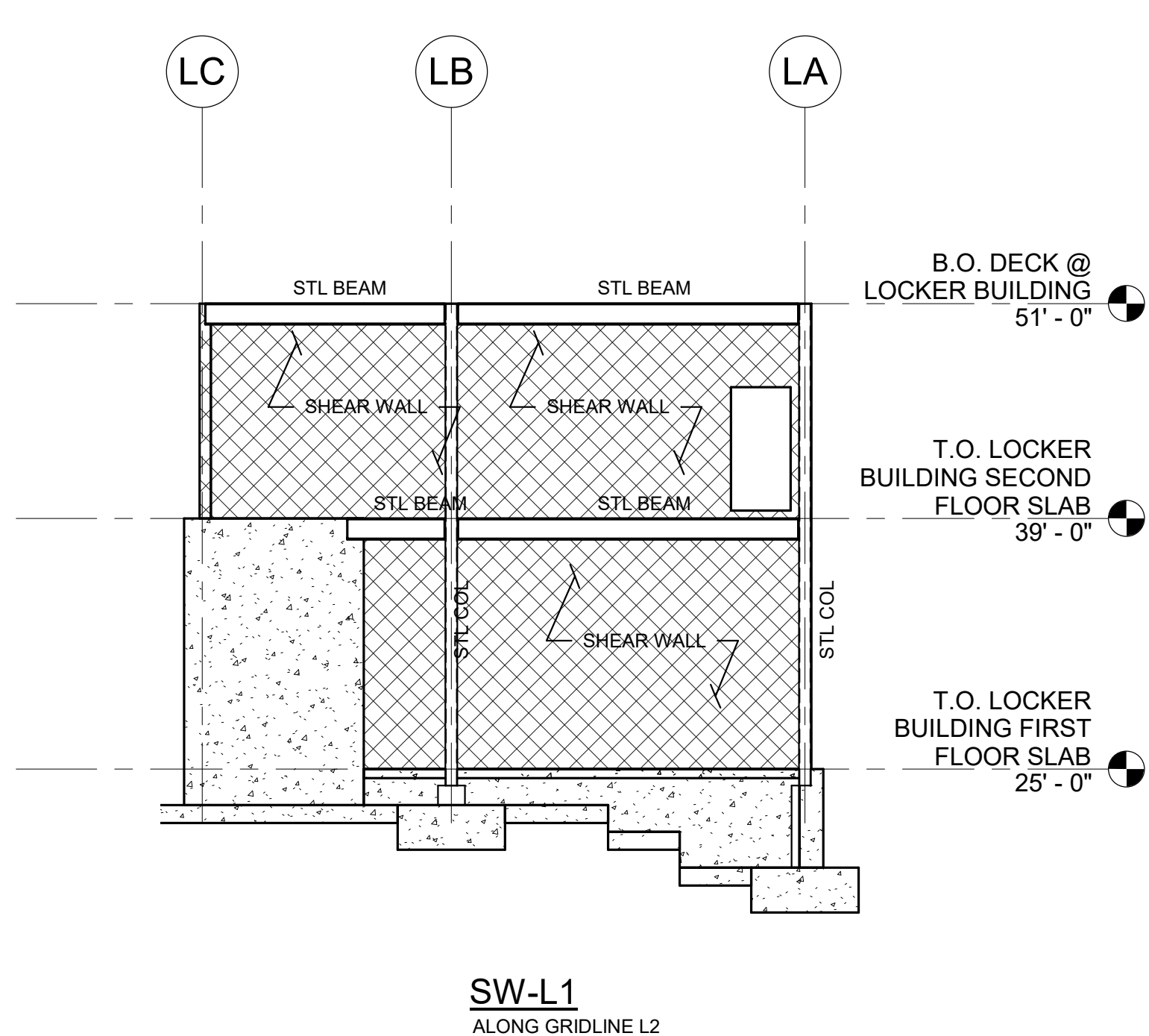


LOCKER ROOM
BUILDING
SHEAR WALLS

Scale: 1/8" = 1'-0"
Job No.: 20202
Drawn By: EDG
Date: August 28th, 2023

SL-3-1

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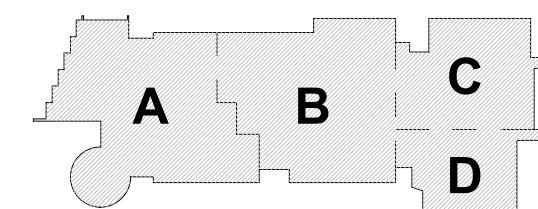
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PR-003	8/23/2023 LOWER CAMPUS BUILDING REVISIONS

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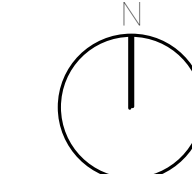
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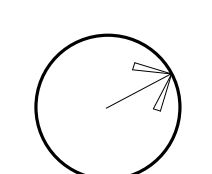


KEY PLAN

PROJECT NORTH



MAGNETIC NORTH

MAINTENANCE
BUILDING
FOUNDATION
PLAN

Scale: As indicated

Job No.: 20202

Drawn By: EDG

Date: August 28th, 2023

SM-1-1

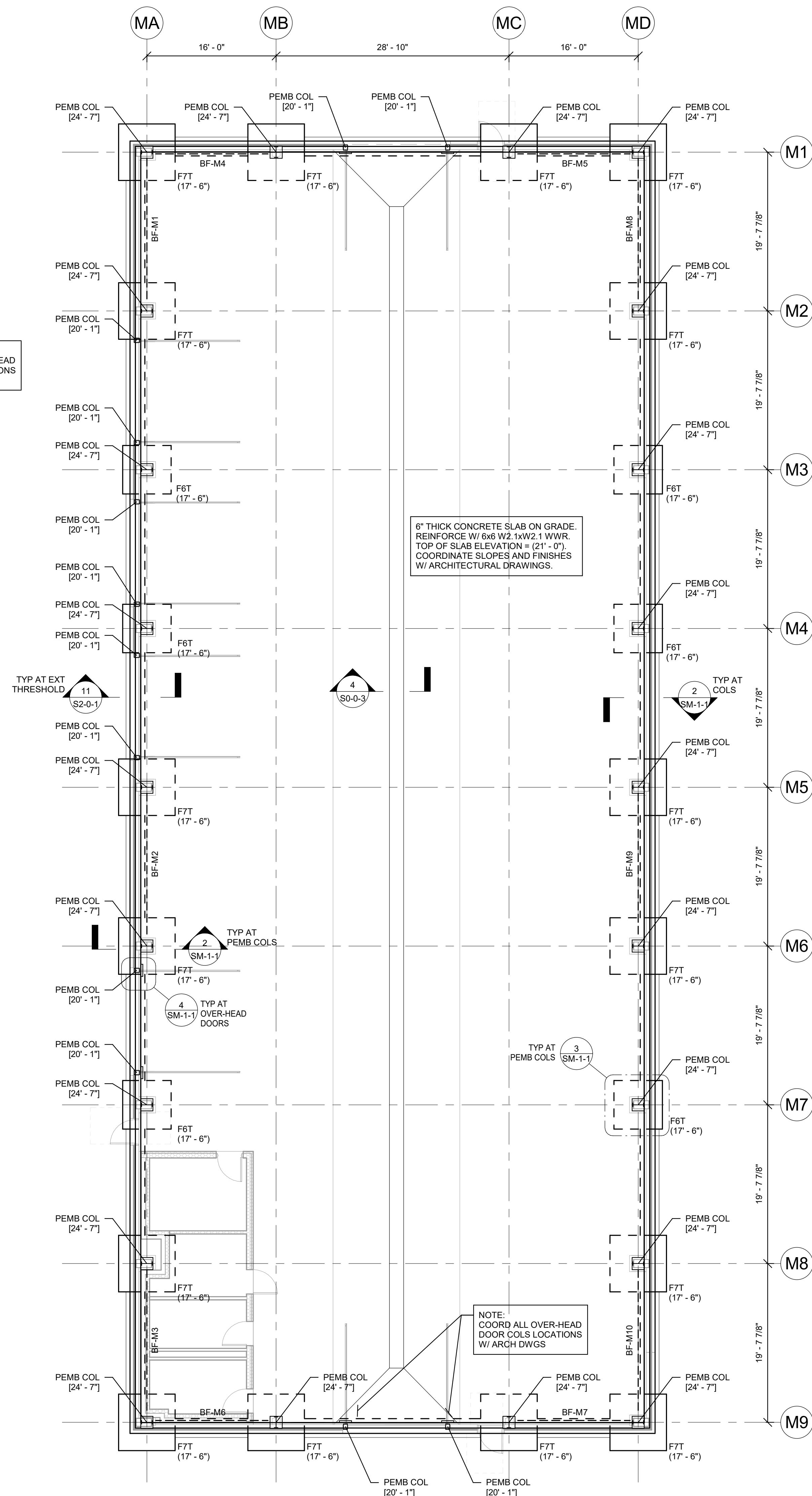
FOUNDATION NOTES:

- 1.) REFER TO GRADING DRAWINGS FOR PLAN AND GRADE ELEVATIONS. THE STRUCTURAL DRAWINGS USES A DATUM OF 100'-0" AT THE MAIN FLOOR, WHICH CORRESPONDS TO 163.50' MEAN SEA LEVEL, AS SHOWN ON THE SITE AND CIVIL DRAWINGS.
- 2.) FOR GENERAL NOTES AND TYPICAL DETAILS SEE DRAWINGS S0-0-1, S0-0-2, S0-0-3, S0-0-4, S0-0-5, S0-0-6, S0-0-7 AND S0-0-8.
- 3.) F3, ETC., INDICATES A FOOTING TYPE. FOR SIZE OF FOOTING AND REINFORCEMENT SEE SCHEDULE ON THIS DRAWING.
- 4.) TOP OF FOOTING ELEVATION TO BE 3'-6" MINIMUM BELOW LOWEST ADJACENT FINISHED GRADE AT EXTERIOR CONDITIONS AND 2'-0" BELOW TOP OF CONCRETE SLAB AT EXTERIOR CONDITIONS. ALL OTHER TOP OF FOOTING ELEVATIONS ARE DENOTED AS THUS: (XX'-XX") ON PLANS. CONTRACTOR TO COORDINATE AND VERIFY ALL TOP OF FOOTING ELEVATIONS WITH UNDERGROUND PLUMBING SUB-CONTRACTOR'S FIELD LAYOUT.
- 5.) ALL FOOTING ELEVATIONS NOTED ON PLAN ARE SHOWN ONLY TO ASSIST IN COORDINATION. ALL FOOTING ELEVATIONS MUST BE COORDINATED WITH STRUCTURAL REQUIREMENTS, TYPICAL DETAILS, ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS.
- 6.) ALL FOOTINGS TO BE CENTERED UNDER COLUMNS UNLESS NOTED OTHERWISE.
- 7.) **SF** → INDICATES A STEPPED FOOTING REFER TO DETAIL 1 ON DRAWING S0-0-2.
- 8.) BOTTOM OF BASE PLATE ELEVATION TO BE 1'-5" MINIMUM BELOW TOP OF CONCRETE SLAB AT INTERIOR CONDITIONS, AND 0'-11" BELOW TOP OF CONCRETE SLAB AT EXTERIOR CONDITIONS. UNLESS NOTED OTHERWISE AS (XX'-XX") REFER TO ARCHITECTURAL DRAWINGS FOR BRICK SHELF ELEVATIONS.
- 9.) FOR UNDER SLAB DRAINAGE AND WALL DRAINS, COORDINATE WITH ARCHITECTURAL, STRUCTURAL, CIVIL, AND PLUMBING DRAWINGS.
- 11.) **mm** INDICATES A DEPRESSED SLAB ON GRADE. REFER TO DETAILS 6 AND 7 ON DRAWING S0-0-2 COORDINATE ALL SLAB DEPRESSIONS WITH REQUIREMENTS ON ARCHITECTURAL DRAWINGS.
- 12.) FOR TYPICAL EXTERIOR DOOR DETAIL REFER TO DETAIL 6 ON DRAWING S0-0-3 AND RELEVANT SECTIONS.
- 13.) **C** OR **3** ON DRAWING S0-0-4 FOR REINFORCEMENT AND DETAIL 4 ON DRAWING S0-0-6 FOR CONNECTIONS TO STEEL BEAMS AND CONCRETE SLABS AT THE TOP OF WALL FOR NON-STRUCTURAL OR **W** WALLS. REFER TO RELEVANT SECTIONS FOR CONNECTIONS OF SHEAR WALLS TO THE STRUCTURE.
- 14.) FOR DIMENSIONS AND ELEVATIONS NOT GIVEN REFER TO ARCHITECTURAL DRAWINGS.
- 15.) **C** OR **5** ON DRAWING S0-0-2 INDICATES CONCRETE PIER REFER TO TYPICAL DETAIL 5 ON DRAWING S0-0-2.
- 16.) **C** OR **5** ON DRAWING S0-0-2 INDICATES UNDERGROUND UTILITY LINES PLUMBING THROUGH CONCRETE FOUNDATION WALL. TYPICAL COORDINATE FOOTING ELEVATION WITH PIPE INVERTS AND TYPICAL STRUCTURAL DETAILS.

PRE-ENGINEERED METAL BUILDING NOTES:

- 1.) PRE-ENGINEERED METAL BUILDING (PEMB) MANUFACTURER IS RESPONSIBLE FOR THE DESIGN, FABRICATION, AND ERECTION OF THE ENTIRE MAINTENANCE BUILDING SUPERSTRUCTURE, INCLUDING BUT NOT LIMITED TO: STEEL COLUMNS, BEAMS, BRACED FRAME MEMBERS, GIRTS, BASE PLATES, ROOF DECKING, METAL PANEL SIDING, ETC.
- 2.) ALL FOOTINGS TO BE CENTERED UNDER COLUMN BASE PLATE, UNLESS NOTED OTHERWISE.
- 3.) BF-M1, ETC. INDICATES BRACED BAY PEMB MANUFACTURER IS RESPONSIBLE FOR DESIGN, DETAILING, FABRICATION, ETC. OF BRACE FRAME MEMBER-TO-STEEL FRAME CONNECTIONS.
- 4.) CONTRACTOR TO COORDINATE ALL CONCRETE PIER SIZES WITH BASE PLATE SIZES PER PEMB DESIGN DRAWINGS.
- 5.) CONTRACTOR TO COORDINATE ALL PEMB DESIGN DRAWINGS WITH ARCHITECTURAL DRAWINGS AS REQUIRED.

NOTE:
COORD ALL OVER-HEAD
DOOR COLS LOCATIONS
W/ ARCH DWGS

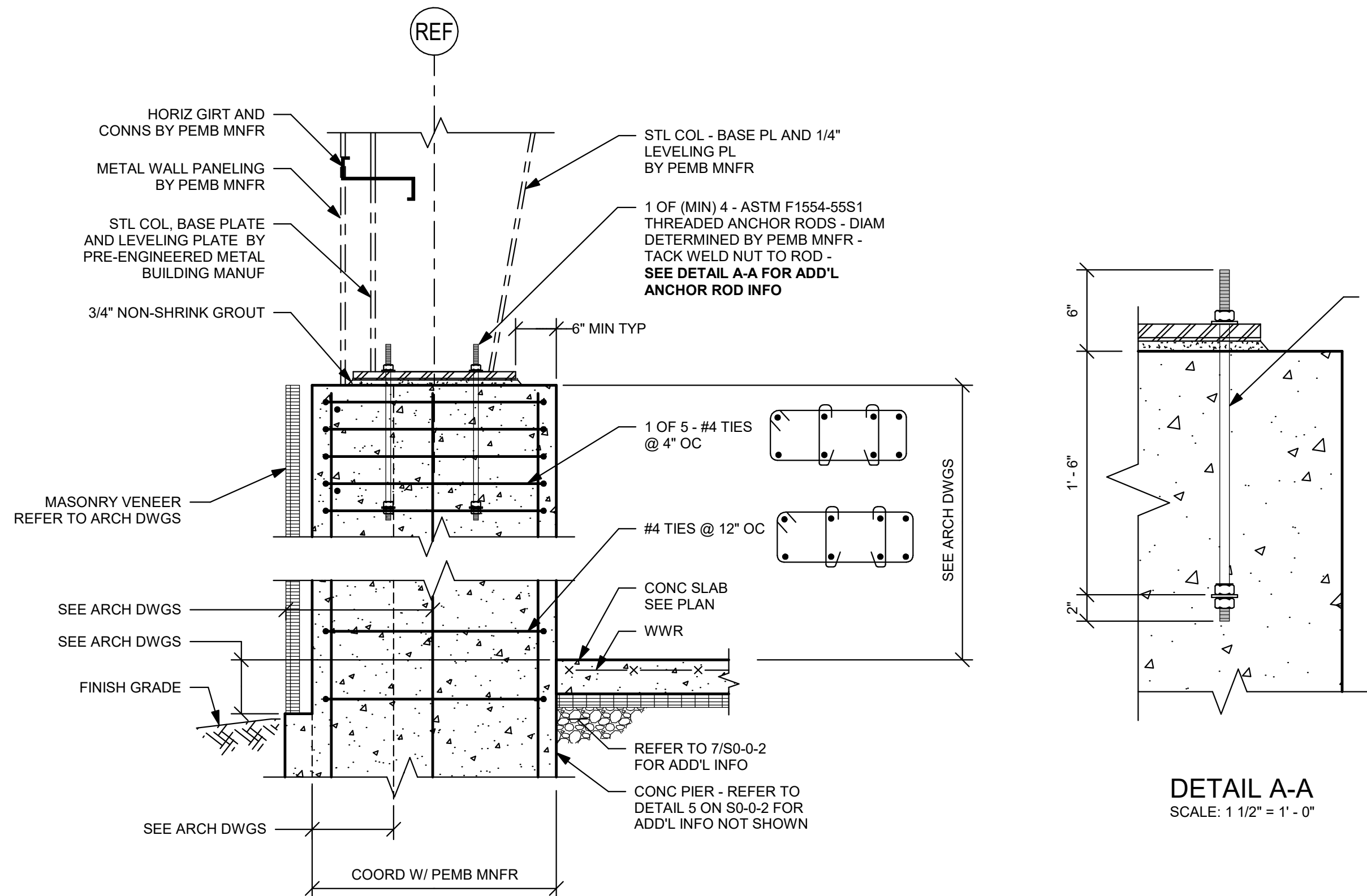
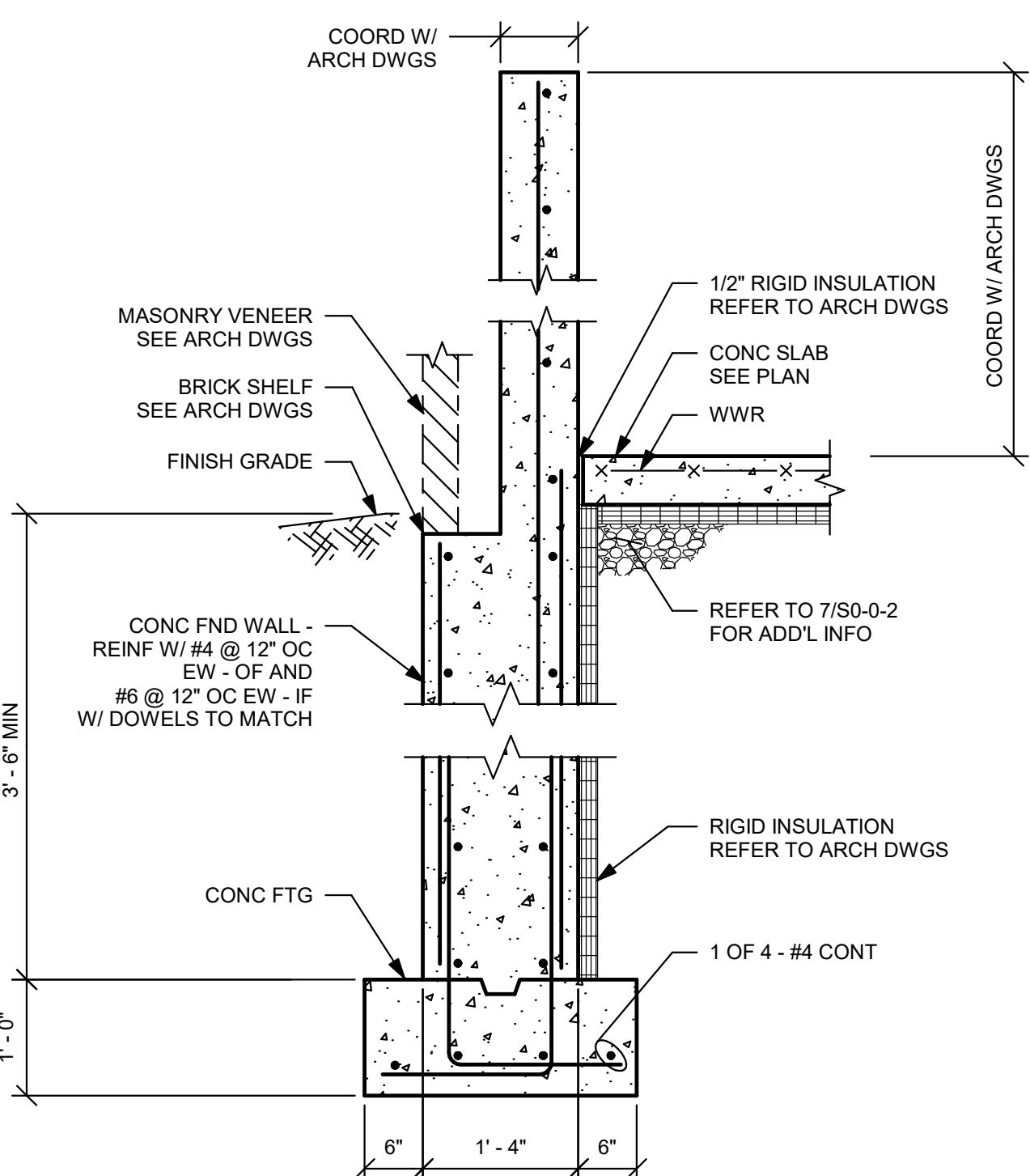
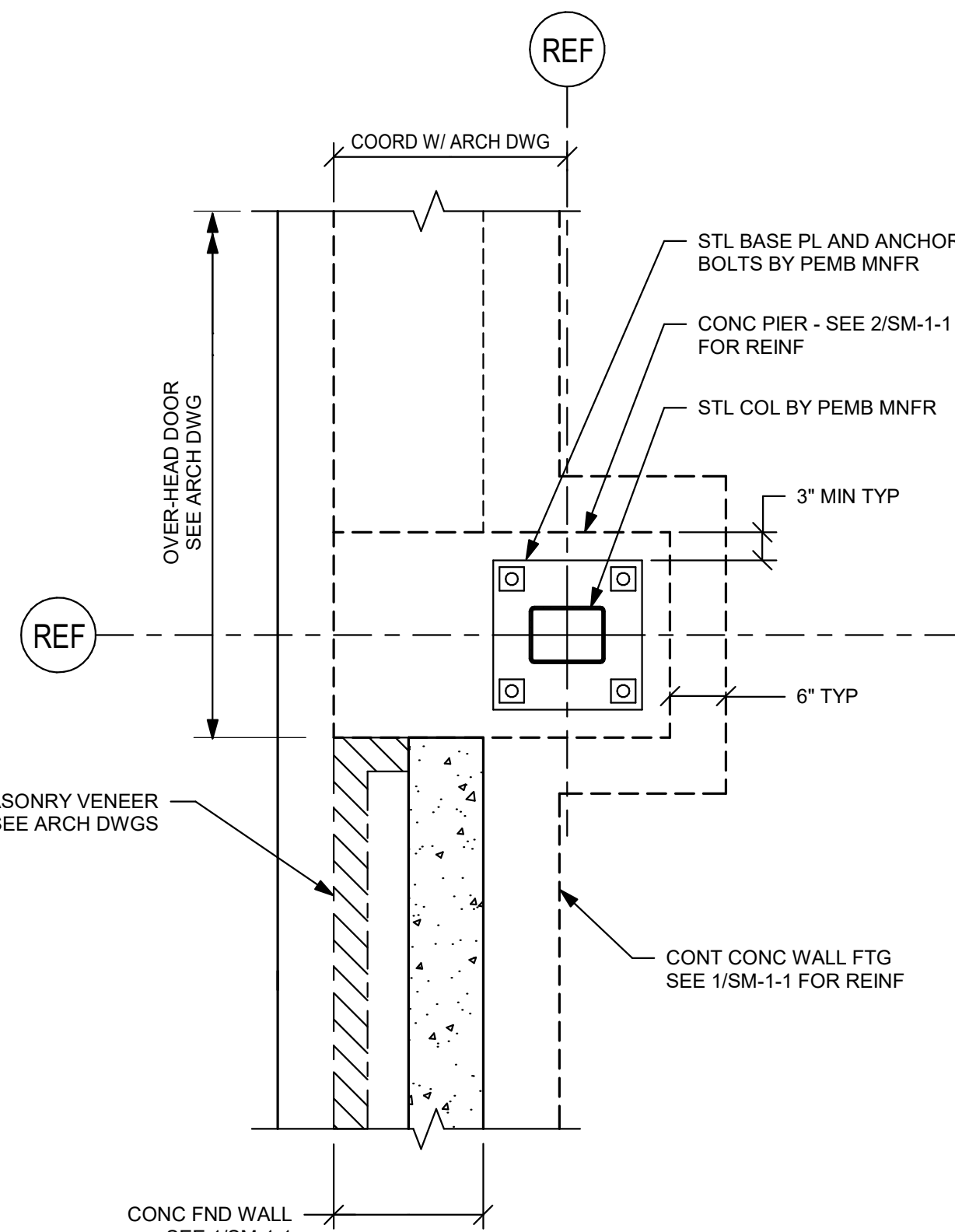
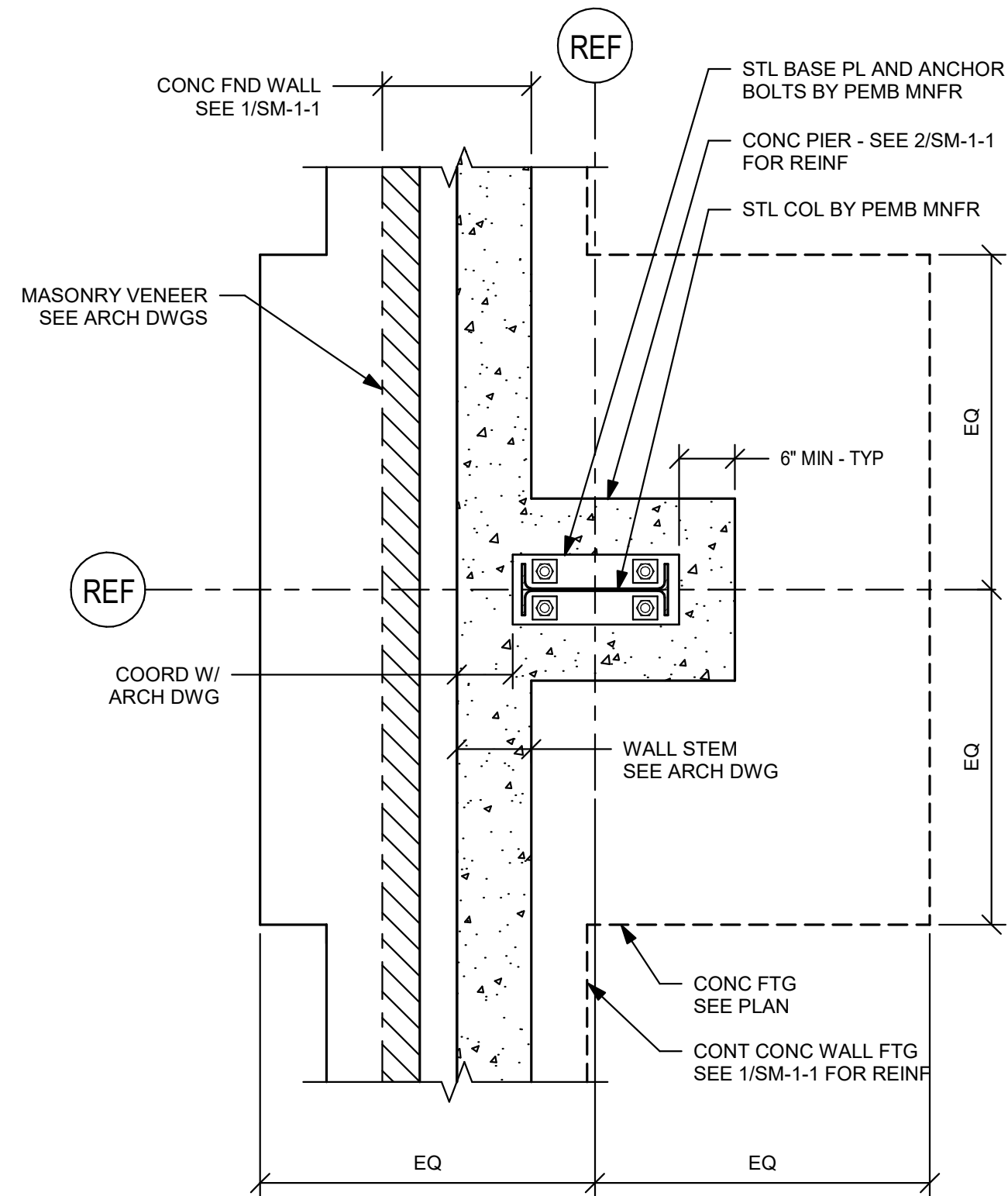


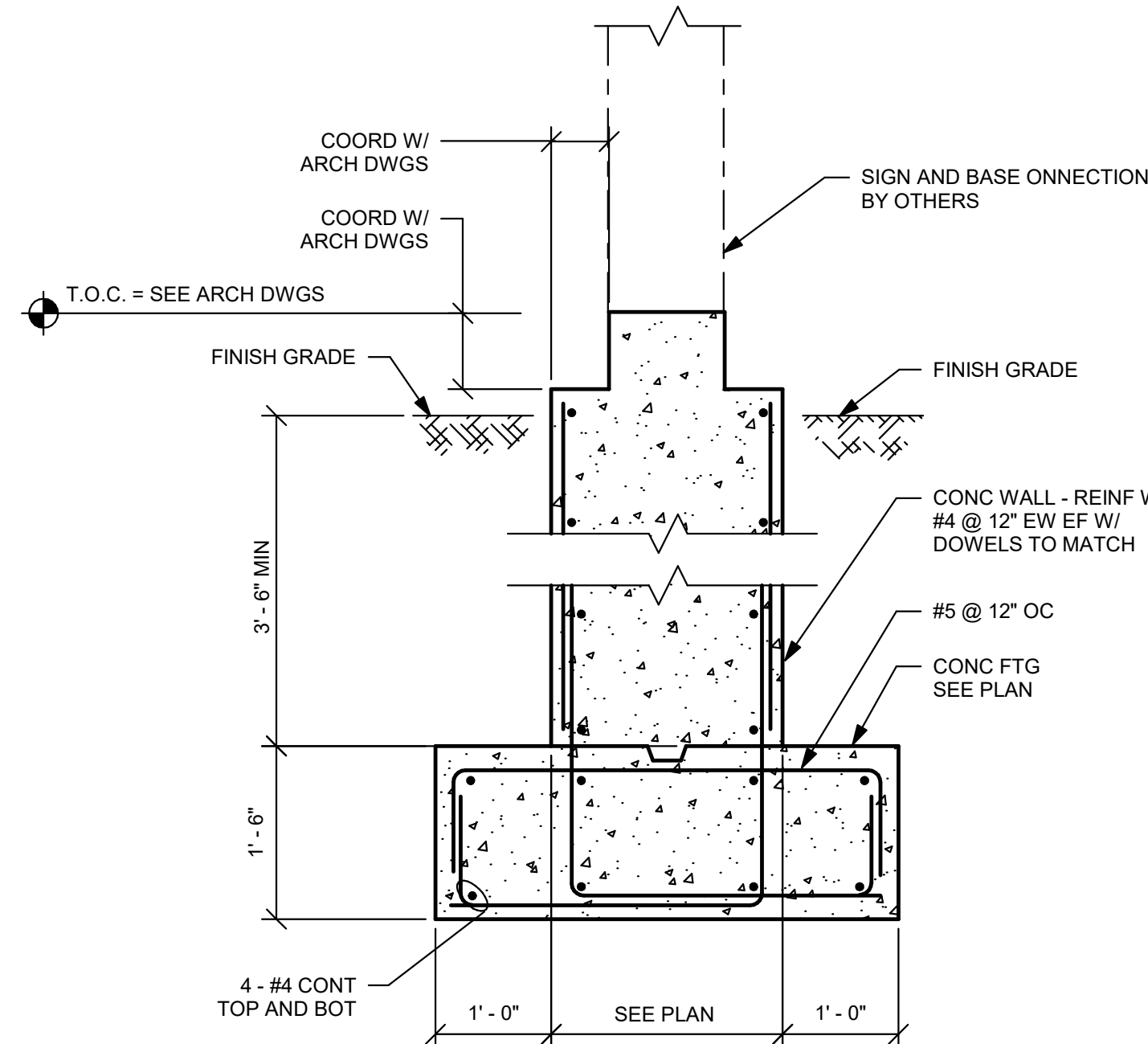
NOTE:
COORD ALL OVER-HEAD
DOOR COLS LOCATIONS
W/ ARCH DWGS

MAINTENANCE BUILDING GROUND FLOOR PLAN

FOOTING SCHEDULE		
DESIGN SOIL BEARING CAPACITY = 2 TSF		
MARK	SIZE	REINFORCEMENT
F6T	6'-0" x 6'-0" x 2'-0"	7 - #6 BOT EA WAY
F7T	7'-0" x 7'-0" x 2'-0"	8 - #6 BOT EA WAY
T INDICATES TOP REINFORCING TO MATCH BOTTOM REINFORCING		

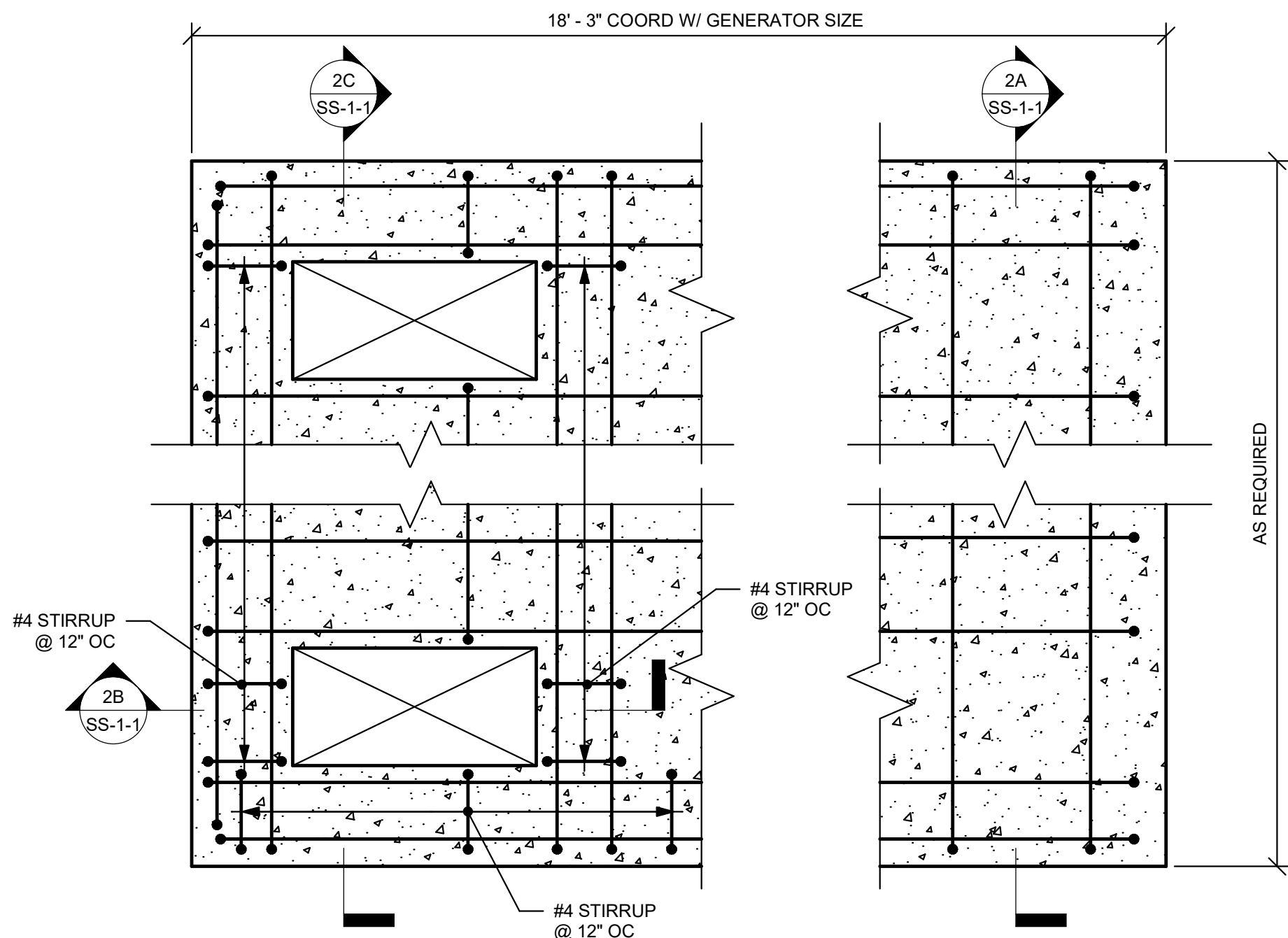
BRACE FRAME KEY		
0' TYP	WF	BF-X
0' TYP	BF-X	WF
0' TYP	WF	BF-X
0' TYP	BF-X	WF
0' TYP	WF	BF-X
0' TYP	BF-X	WF

DETAIL A-A
SCALE: 1 1/2" = 1'-0"



SITE SIGNAGE BASE

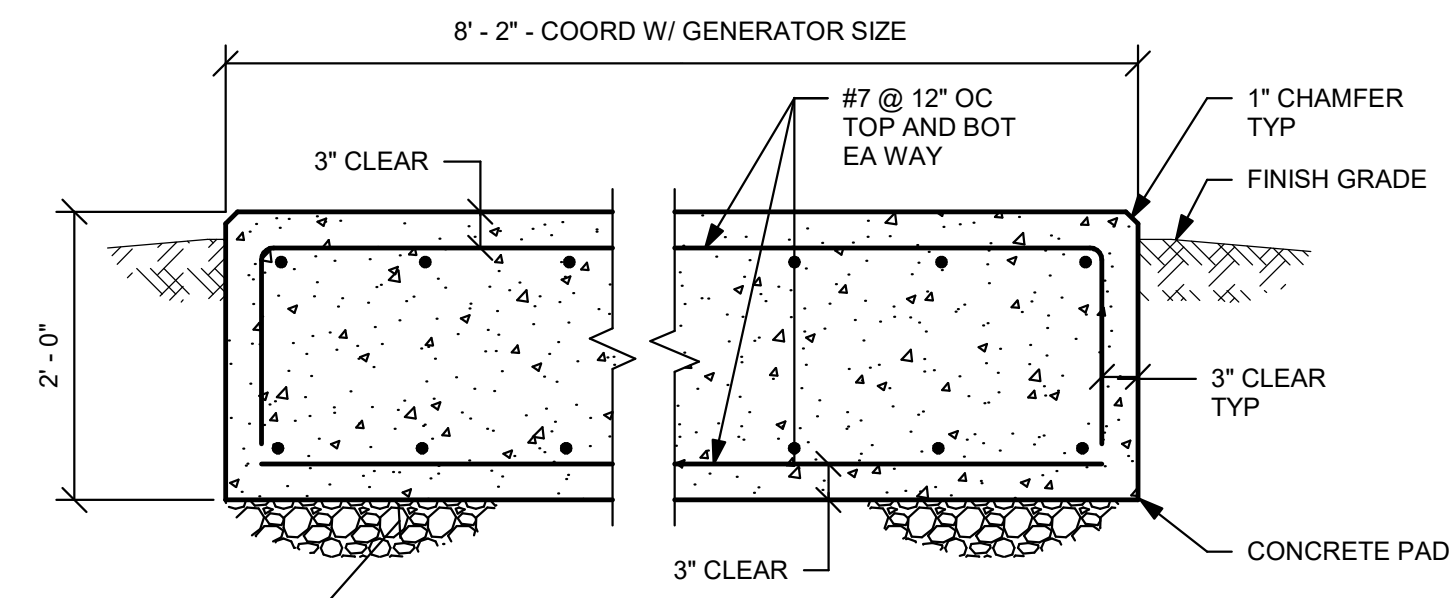
NOTE:
LOCATION AND DIMENSIONS OF SIGN TO BE
COORDINATED WITH ARCHITECTURAL AND
LANDSCAPE DRAWINGS



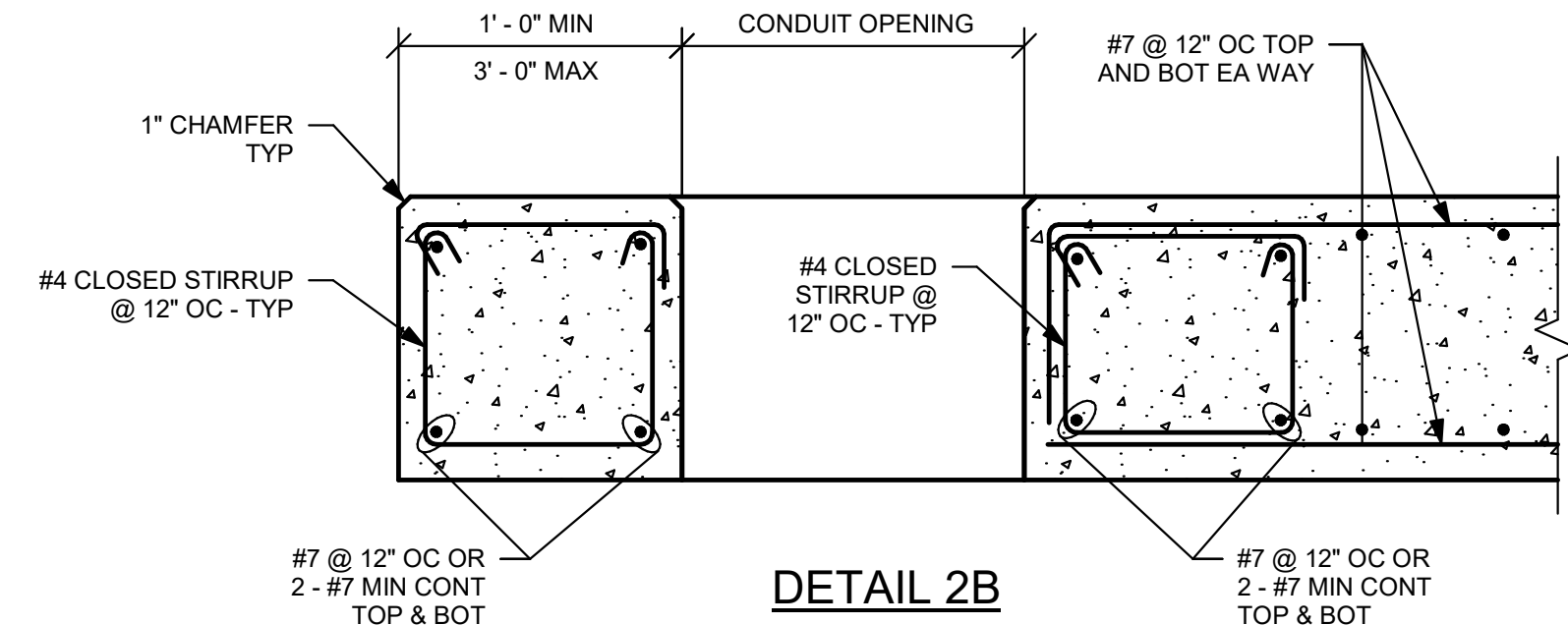
EXTERIOR PAD PLAN AND DETAILS

NOTES

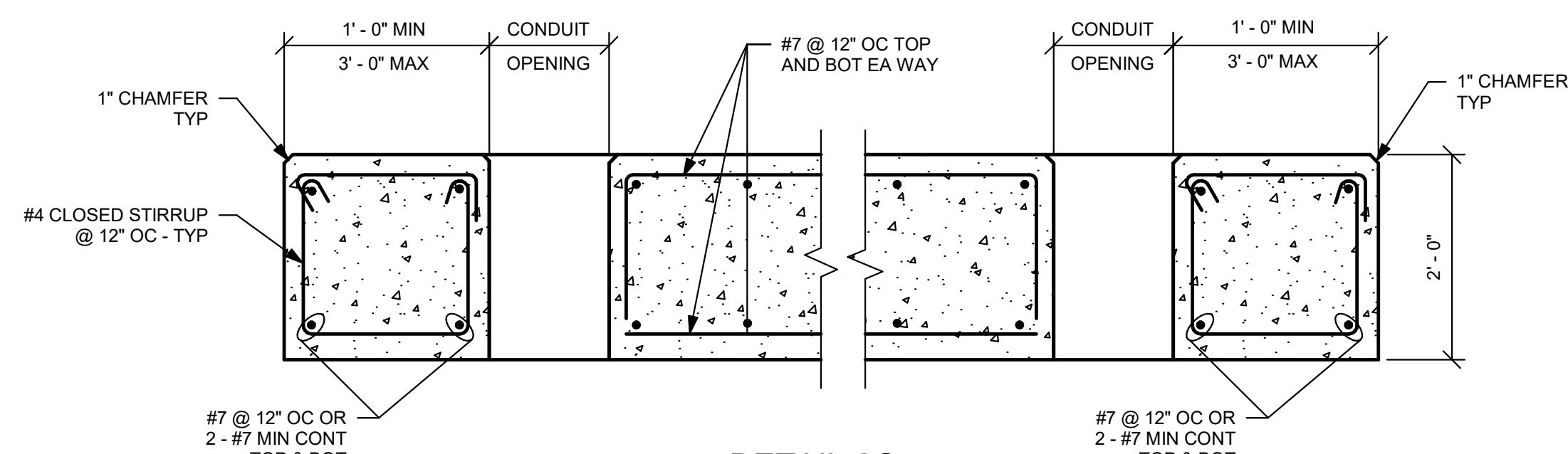
- 1.) REFER TO MECHANICAL AND ELECTRICAL DRAWINGS FOR ADDITIONAL INFORMATION.
- 2.) PAD DIMENSIONS ARE APPROXIMATE. COORDINATE WITH MECHANICAL, ELECTRICAL, AND LANDSCAPE DRAWINGS.
- 3.) REFER TO SITE, MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATION AND ADDITIONAL INFORMATION.
- 4.) TOP AND BOTTOM BARS SHALL BE CONTINUOUS LAP SPLICE LENGTH = 59".
- 5.) 28 DAY CONCRETE COMPRESSIVE STRENGTH SHALL BE 5,000 PSI.



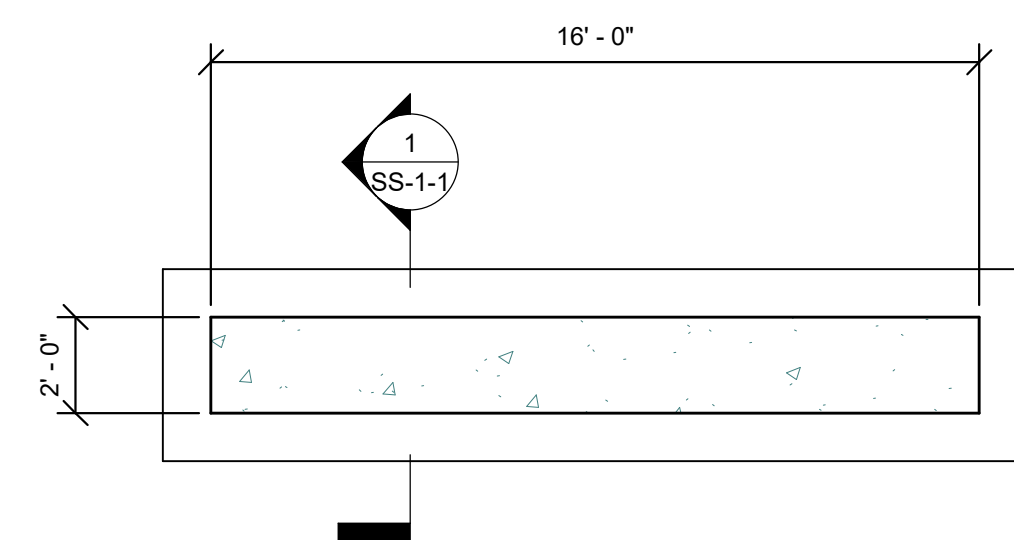
DETAIL 2A



DETAIL 2B

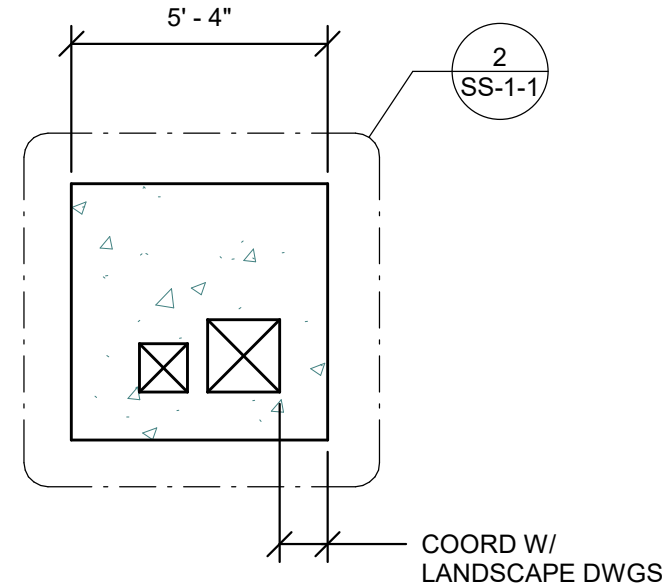


DETAIL 2C



SIGNAGE FOUNDATION PLAN

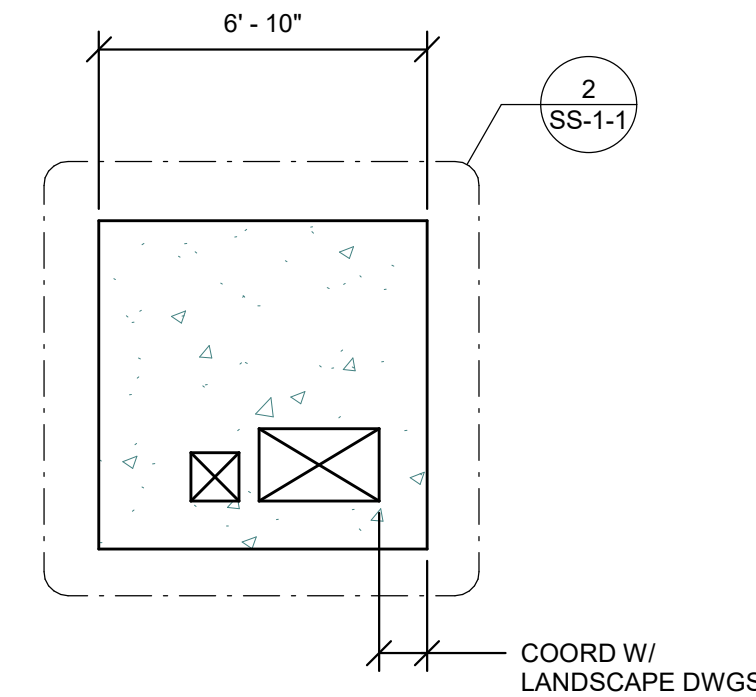
SCALE: 1/4" = 1'-0"



TRANSFORMER PAD 1 PLAN

SCALE: 1/4" = 1'-0"

NOTE:
3 REQUIRED. COORDINATE LOCATIONS
WITH ARCHITECTURAL AND LANDSCAPE
DRAWINGS.



TRANSFORMER PAD 2 PLAN

SCALE: 1/4" = 1'-0"

NOTE:
2 REQUIRED. COORDINATE LOCATIONS
WITH ARCHITECTURAL AND LANDSCAPE
DRAWINGS.

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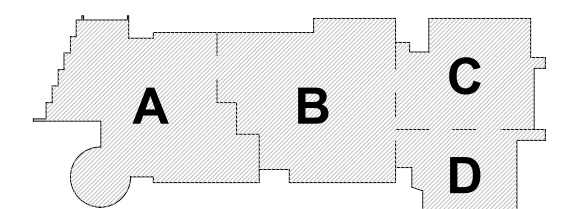
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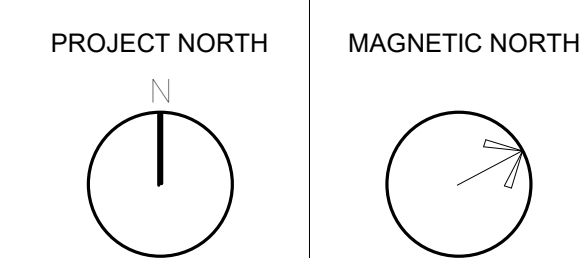
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BID SET

August 28th, 2023



KEY PLAN



SITE FOUNDATION PLANS

Scale: As indicated
Job No.: 20202
Drawn By: EDG
Date: August 28th, 2023

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